



CITY OF CHANDLER, ARIZONA

101/202 REDUNDANT 66-INCH SEWER LINE CITY PROJECT NO.: WW2402.401

CONSTRUCTION SERVICES AGREEMENT (CONSTRUCTION MANAGER AT RISK)

Daniel Haskins

**Daniel Haskins, P.E.
CIP City Engineer**

CITY OF CHANDLER, ARIZONA

101/202 REDUNDANT 66-INCH SEWER LINE
CITY PROJECT NO.: WW2402.401

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**CONSTRUCTION MANAGER AT RISK
CONSTRUCTION SERVICES AGREEMENT
PROJECT NO.: WW2402.401**

This Agreement (the "AGREEMENT") is made and entered into on the ____ day of _____, 2025, ("Effective Date") by and between City of Chandler, an Arizona municipal corporation, hereinafter called "City" and B&F Contracting, Inc. the "Construction Manager at Risk" or "CM@Risk" designated below (City and CM@Risk may individually be referred to as "Party" and collectively referred to as "Parties").

City and CM@Risk agree as follows:

ARTICLE 1 - PARTICIPANTS AND PROJECT

CITY: **CIP City Engineer: Daniel Haskins, P.E.**
 Public Works & Utilities Department
 P.O. Box 4008, Mail Stop 407 Chandler, AZ 85244-4008
 Phone: 480-782-3335 Email: Daniel.haskins@chandleraz.gov

CITY: **Construction Project Manager: Raymond Potts**
 Public Works & Utilities Department
 P.O. Box 4008, Mail Stop 407, Chandler, AZ 85244-4008
 Phone: 480-782-3326 Email: Raymond.Potts@chandleraz.gov

CM@RISK:	Legal Company Name:	B&F Contracting, Inc.
	Mailing Address:	11011 North 23rd Avenue Phoenix, AZ 85029
	Physical Address:	11011 North 23rd Avenue Phoenix, AZ 85029
	Arizona Roc No.:	089744 A
	Federal Tax Id No.:	86-0677300
	State Where Organized:	Arizona
	Business Organization:	Corporation
	Statutory Agent Name:	Bruce W. Balls
	Statutory Agent Mailing Address:	1717 West Mandalay Lane Phoenix, AZ 85023
	Statutory Agent Physical Address:	1717 West Mandalay Lane Phoenix, AZ 85023

CM@Risk's Authorized Project Representative:

Name: Josh Onstott
Title: Executive Director
Phone: 623-238-3792
Email: jonstott@bfcontracting.com

PROJECT DESCRIPTION:

This project includes a redundant 66-inch diameter gravity sewer line that crosses the Loop 202 Santan Freeway, rerouting several 48-inch diameter sewers, new junction structures for connecting the existing and proposed sewer, and cured-in-place pipe (CIPP) lining of existing sewers.

PROJECT LOCATION:

Loop 101/202 Santan Freeway interchange.

ARTICLE 2 - AGREEMENT DOCUMENTS

2.1 AGREEMENT DOCUMENTS

The Agreement between City and CM@Risk will consist of the following Agreement Documents:

1. This Construction Services Agreement and all of its Exhibits, including Project Plans and Technical Specifications.
2. General Conditions and General Conditions Appendices, incorporated by reference.
3. Project Specific Special Provisions as set forth in **Exhibit A**, incorporated by reference.
4. Accepted GMP/Price Proposal as set forth in **Exhibit D**, incorporated by reference.

2.2 In the event of any inconsistency, conflict, or ambiguity between or among the Agreement Documents, the Agreement Documents will take precedence as described in Section 14.1.4 of the General Conditions.

2.3 DEFINITIONS

The definitions in Sections 2 and 15 of the General Conditions apply to all the Agreement Documents, including this Agreement.

ARTICLE 3 - PRE-CONSTRUCTION SERVICES

Although CM@Risk has performed Pre-Construction Services pursuant to a separate Agreement between City and CM@Risk, the completion, quality and accuracy of those services and the deliverables provided by City thereunder directly impact CM@Risk's performance of its obligations under this Agreement. Therefore, all of CM@Risk's obligations, duties, and warranties in relation to Pre-Construction Services and deliverables survive completion of the Pre-Construction Services Agreement and are incorporated herein. Any breach of any of CM@Risk's duties, obligations, or warranties under the Pre-Construction Services Agreement will likewise be considered a breach of this Agreement.

ARTICLE 4 - CONSTRUCTION SERVICES

4.1 GENERAL

4.1.1 CM@Risk agrees at its own cost and expense, to do all work necessary and required to fully, timely and properly complete the construction of the Project in strict accordance with the Agreement Documents in a good and workmanlike manner, free and clear of all claims, liens, and charges whatsoever, in the manner and under the conditions specified, and within the schedule, stated in attached **Exhibit A**.

- 4.1.2 CM@Risk must provide all of the labor and materials, and perform the Work in accordance with Section 4 of the General Conditions. Some, but not all, of the major components of the Construction Services and the corresponding subsections of Section 4 of the General Conditions are set forth below.
- 4.1.3 This is an Agreement for complete construction services in accordance with the Construction Manager at Risk method of delivery of construction services. CM@RISK has participated in the design process and been an active member of the Project Design Team and is fully aware of any issues and constraints involved in this Construction Project.
- 4.1.4 CM@RISK is the CITY's fiduciary responsible for undertaking all necessary action contemplated under the Agreement documents to construct the Project and ensure timely and quality completion of the Project at a cost within the Guaranteed Maximum Price (GMP).
- 4.1.5 At all times relevant to this Agreement and performance of the Work, the CM@Risk must fully comply with all Laws, Regulations, or Legal Requirements applicable to City, the Project and the Agreement, including, without limitation, those set forth on attached **Exhibit A**.
- 4.1.6 CM@Risk must perform the Work under this Agreement using only those firms, team members and individuals designated by CM@Risk consistent with the Statement of Qualifications dated June 26, 2024, the GMP Proposal, or otherwise approved by City pursuant to the General Conditions. No other entities or individuals may be used without prior approval of the Project Manager
- 4.1.7 CM@Risk will comply with all terms and conditions of the General Conditions.
- 4.1.8 In the event of a conflict between this Agreement and the General Conditions or an exhibit hereto or appendix thereto, the terms of this Agreement will control.
- 4.1.9 Ownership of Work Product. Notwithstanding anything to the contrary in this Agreement, all Work Product prepared or otherwise created in connection with the performance of this Agreement, including the Work, are to be and remain the property of City. For purposes of this provision, "Work Product" will include all designs, drawings, plans, specifications, ideas, renderings and other information or matter, in whatever form created (e.g., electronic or printed) and in all media now known or hereinafter created. All Work Product will be considered Work Made for Hire as defined in the United States Copyright Act 17 U.S.C. § 101 (Copyright Act). If for any reason, any such Work is found not to be a Work Made for Hire, CM@Risk hereby transfers and assigns ownership of the copyright in such Work to City. The rights in this Section are exclusive to City in perpetuity.

4.2 **CM@RISK'S PRE-AGREEMENT AND PRE-WORK DELIVERABLES**

4.2.1 The CM@Risk must provide the Deliverables in accordance with Section 4.2 of the General Conditions.

4.3 **PRE-CONSTRUCTION CONFERENCE**

Refer to Section 4.3 of the General Conditions.

4.4 **PERFORMANCE OF THE WORK (INCLUDING FIELD MEASUREMENTS, SUBCONTRACTORS, AND SUPPLIERS)**

Refer to Section 4.4 of the General Conditions.

4.5 **CONTROL OF THE PROJECT SITE**

Refer to Section 4.5 of the General Conditions.

4.6 **PROJECT SAFETY**

Refer to Section 4.6 of the General Conditions.

4.7 **MATERIALS QUALITY, SUBSTITUTIONS AND SHOP DRAWINGS**

Refer to Section 4.7 of the General Conditions.

4.8 **PROJECT RECORD DOCUMENTS**

Refer to Section 4.8 of the General Conditions.

4.9 **WARRANTY AND CORRECTION OF DEFECTIVE WORK**

Refer to Section 4.9 of the General Conditions.

ARTICLE 5 - CITY RESPONSIBILITIES

5.1 City will have the responsibilities, and provide the information specified in, and subject to the conditions set forth in, Section 5 of the General Conditions.

ARTICLE 6 - AGREEMENT TIME

6.1 **GENERAL**

6.1.1 The Agreement Duration is **550** Calendar Days.

6.1.2 The Agreement Time will start with the Notice to Proceed (NTP) and end with Final Acceptance, as set forth in Article 6.4 below. The Notice to Proceed cannot be issued until approval and acceptance by City of the GMP or Fixed Price.

6.1.3 The Agreement Time will be as set forth in the Project Schedule. CM@Risk agrees that it will commence performance of the Work and complete the Project through Final Acceptance within the Agreement Time.

6.1.4 Time is of the essence of this Agreement for the Project, and for each phase and designated Milestone thereof.

6.2 **PROJECT SCHEDULE**

6.2.1 The Project Schedule approved as part of the GMP Proposal and incorporated herein as part of the attached **Exhibit D** must be updated and maintained throughout CM@Risk's performance under this Agreement in accordance with Section 6.2 of the General Conditions.

6.2.2 Failure on the part of CM@Risk to adhere to the approved Project Schedule will be deemed a material breach and sufficient grounds for termination of this Agreement by City.

6.3 **SUBSTANTIAL COMPLETION**

Substantial Completion must be achieved no later than the Substantial Completion Date set forth in the Project Schedule. Substantial Completion will be determined in accordance with Section 6.3 of the General Conditions.

6.4 **FINAL ACCEPTANCE**

6.4.1 Final Acceptance will be obtained within the time period set forth in the Project Schedule.

6.4.2 Final Acceptance will be issued pursuant to Section 6.5 of the General Conditions.

6.5 **LIQUIDATED DAMAGES**

6.5.1 Substantial Completion Liquidated Damages. CM@Risk acknowledges and agrees that if CM@Risk fails to obtain Substantial Completion of the Work within the Agreement Time, City will sustain extensive damages and serious loss as a result of such failure. The exact amount of such damages will be extremely difficult to ascertain. Therefore, City and CM@Risk agree that if CM@Risk fails to achieve Substantial Completion of the Work within the Agreement Time, City will be entitled to retain or recover from CM@Risk, as liquidated damages and not as a penalty, the sum per calendar day as indicated in MAG § 108.9.

6.5.2 Final Acceptance Liquidated Damages. For the same reasons set forth in Article 6.5.1 above, City and CM@Risk further agree that if CM@Risk fails to achieve Final Acceptance of the Work within the Agreement Time, City will be entitled to retain or recover from CM@Risk, as liquidated damages and not as a penalty, the sum per calendar day as

indicated in MAG § 108.9 commencing from the actual date of Substantial Completion or Final Acceptance as required under the Agreement.

6.5.3 MAG Liquidated Damages. If no liquidated damages are specified in Articles 6.5.1 or 6.5.2 above, then the liquidated damages provisions in MAG § 108.9 will apply.

6.5.4 City may deduct liquidated damages described in this Article 6.5 from any unpaid amounts then or thereafter due CM@Risk under this Agreement. Any liquidated damages not so deducted from any unpaid amounts due CM@Risk will be payable to City at the demand of City, together with interest from the date of the demand at the highest lawful rate of interest payable by CM@Risk.

6.6 **MUTUAL WAIVER OF CONSEQUENTIAL DAMAGES ONLY**

6.6.1 CM@Risk and City waive claims against each other for consequential damages arising out of or relating to this Agreement. This mutual waiver includes:

6.6.1.1 Damages incurred by City for rental expenses, for losses of use, income, profit, financing, business and reputation, and for loss of management or employee productivity or of the services of such persons; and

6.6.1.2 Damages incurred by CM@Risk for principal office expenses including the compensation of personnel stationed there, for losses of financing, business and reputation, and for loss of profit except anticipated profit arising directly from the Work.

6.6.2 This mutual waiver is applicable, without limitation, to all consequential damages due to either Party's termination of this Agreement. Nothing contained in this Article 6.6 will be deemed to preclude an award of liquidated damages, when applicable, in accordance with Article 6.5 above.

6.6.3 Nothing herein will be deemed to constitute a waiver of any other remedy available to City in the event of CM@Risk's default under this Agreement prior to full performance of the Work including, as applicable, specific performance or completion of the Work on behalf of CM@Risk, the cost and expense of which will be offset against any monies then or thereafter due to CM@Risk (if any) and otherwise immediately reimbursed to City by CM@Risk.

ARTICLE 7 - AGREEMENT PRICE

7.1 **AGREEMENT PRICE**

7.1.1 In exchange for CM@Risk's full, timely, and acceptable performances and construction of the Work under this Agreement, and subject to all of the terms of this Agreement, City will pay CM@Risk the "Agreement Price," which:

The sum of the CM@Risk's Fee and reimbursable Cost of the Work, as defined in Section 15 of the General Conditions, which the CM@Risk guarantees will not exceed the GMP set forth in **Exhibit D** in the amount of **\$34,649,463.94**. Costs which would cause the GMP to be exceeded must be paid by CM@Risk without reimbursement from City.

7.1.2 The Agreement Price is all-inclusive and specifically includes all fees, cost, insurance and bond premiums, allowances, construction contingency, owner's contingency, and taxes of any type necessary to fully, properly and timely perform and construct Work.

7.2 **CHANGES TO AGREEMENT PRICE**

Shall be determined under Section 9 of the General Conditions.

ARTICLE 8 - FORCED LABOR OF ETHNIC UYGHURS PROHIBITED

8.1 **FORCED LABOR OF ETHNIC UYGHURS PROHIBITED.** By entering into this Agreement, Contractor certifies and agrees Contractor does not currently use and will not use for the term of this Agreement: (i) the forced labor of ethnic Uyghurs in the People's Republic of China; or (ii) any goods or services produced by the forced labor of ethnic Uyghurs in the People's Republic of China; or (iii) any contractors, subcontractors or suppliers that use the forced labor or any goods or services produced by the forced labor of ethnic Uyghurs in the People's Republic of China.

SIGNATURE PAGE TO FOLLOW

IN WITNESS WHEREOF, the Parties hereto have executed this Agreement through their duly authorized representatives and bind their respective entitles as of the effective date.

"CITY" CITY OF CHANDLER

**"CM@Risk"
B&F CONTRACTING, INC.**

MAYOR

Joshua Onstott August 21, 2025
Signature Date

RECOMMENDED BY:

Joshua Onstott
Print Name

Daniel Haskins August 21, 2025
Daniel Haskins, P.E.
CIP City Engineer

President
Title

Jonstott@bfcontracting.com
Signer Email Address

APPROVED AS TO FORM:

City Attorney 

ATTEST:

City Clerk Seal

EXHIBIT A

PROJECT SPECIFIC SPECIAL PROVISIONS

4.2.7 Aerial Construction Photography

General Conditions Section 4 Subsections 4.2.7.1 & 4.2.7.2 are not applicable to this project.

4.2.8 Government Approvals and Permits

General Conditions Section 4 Subsection 4.2.8.1, City permit fees will be paid internally by the City and all other fees will be the responsibility of the Contractor.

Subletting of Agreement

Contractor must perform, with his own organization, work amounting to not less than 50 percent of the total Agreement cost.

Failure to submit Subcontractor's List Form, demonstrating self-performance not less than 50 percent of the total Agreement cost, will cause the bid to be deemed non-responsive.

Contractors should contact the Arizona Registrar of Contractors for information on license requirements.

EXHIBIT B

GENERAL CONDITIONS



GENERAL CONDITIONS

Approved date: July 20, 2022

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Appendix 8 – Construction Sign Detail

Appendix 9 – Cost of the Work (Applicable solely to Construction Manager at Risk and Job Order Contracting)

Appendix 10 Landscape Establishment Period

SECTION 1 - SCOPE OF THESE GENERAL CONDITIONS

These General Conditions encompass provisions that apply, and are incorporated into all construction Agreements entered into by the City of Chandler, unless otherwise specifically excluded in the executed Agreement.

SECTION 2 - GENERAL DEFINITIONS

Allowance: A specific amount for a specific item of Work, if any, that City agrees has not been sufficiently designed, detailed, or selected (including design changes from 90% to 100% as authorized by and at the discretion of the City) at the time the Agreement Price is agreed to for Contractor to provide a definitive price.

Alternate Systems Evaluations or Alternative Analysis: Alternatives for design, means and methods or other scope considerations that are evaluated using value analysis principles and have the potential to reduce construction costs while still delivering a quality and functional Project that meets City requirements.

Change Order: A written instrument issued after execution of the Agreement Documents signed by City and Contractor, stating their agreement upon all of the following: the addition, deletion or revision in the scope of services or deliverables; the amount of the adjustment to the Agreement Price, the extent of the adjustment to the Agreement Time, or modifications of other agreement terms. The Agreement Price and the Agreement Time may be changed only by Change Order.

Consultant: Person or firm that provides professional services.

City (Owner or OWNER): City of Chandler, a municipal corporation, with whom Contractor has entered into the Agreement and for whom the Work or Services are to be provided pursuant to the Agreement(s).

Contingent Bid Items: This is a minor bid item which is likely, but not certain, to occur during the course of work. If the Engineer determines that this work is required, the Contractor will accomplish the work and payment will be made based on the contingent unit bid price included in the proposal. Since the quantity listed in the proposal is primarily for bid comparison, the amount of work required by the Engineer may vary materially from this.

Agreement: The written agreement executed between City and Contractor, including all of the Agreement Documents.

Agreement Documents: The documents which together form the Agreement between City and Contractor, as identified in Article 2 of the Agreement, or are otherwise incorporated into the Agreement, including the Agreement, the exhibits thereto, these General Conditions, any Notice to Proceed, and any Job Order (if applicable), the Plans and Specifications, Project Schedule, written and properly executed Change Orders, MAG Specifications and City's amendments thereto, and any other documents so designated in the Agreement.

Agreement Price: The agreed-upon price to be paid to Contractor for full, timely, and acceptable completion of the Work or Services under the terms of the Agreement.

Agreement Time(s): The number of calendar days or the dates related to the applicable phase, Substantial Completion, or Final Acceptance as stated in Agreement Documents. Agreement Time starts

with the Notice to Proceed (NTP) and ends with Final Acceptance. The Agreement Time is set forth in the Agreement and is based upon the Project Schedule agreed to by City in writing.

Contractor: The person or business association with whom City has entered into an agreement for construction related Work or Services in relation to the Project at issue.

Contractor Payment Request: The form that is accepted by City and used by Contractor in requesting progress payments or final payment and which must include such supporting documentation as is required by the Agreement Documents or City.

Construction Budget: The City's budget for construction of the Project.

Construction Documents: The Plans, Specifications, and Drawings prepared and issued by the Design Professional and approved by City for construction, meaning the documents are sealed by the Design Professional (as required), acceptable for permitting and incorporated into the Agreement by this reference. All amendments and modifications to the Construction Documents must be approved in writing by City prior to incorporation into the Agreement.

Cost of the Work: The term Cost of the Work will mean costs necessarily incurred by Contractor in the proper performance of the Work. Such costs will be at rates not higher than the standard paid at the place of the Project except with prior consent of City.

Critical Path Method (CPM): A scheduling technique which identifies the logical sequence of the activities occurring in a Construction Project, the anticipated time required to complete each activity in the Project, and the activities that must be completed on schedule to finish the Project within the anticipated time. Typically, activities are arranged in a network that shows both activities and their dependencies. CPM is also used as a management technique which enables contracting parties to predict when activities may occur so that resources can be effectively used and limitations can be identified.

Critical Path: Critical Path is the sequence of project network activities which add up to the longest overall duration. Once established in the Project Schedule, the Critical Path for the Project must not be changed without prior written approval of City.

Day: Calendar day(s) unless otherwise specifically stated in the Agreement Documents.

Design Professional: The qualified, licensed person, firm or corporation who furnishes design and construction administration services required under the Agreement Documents. These services may include, but are not limited to: development of Construction Drawings and Documents, review of Contractor Submittal(s), review of and response to Requests for Information, approval and certification of progress payment applications, construction administration, and construction agreement close out.

Differing Site Conditions: Concealed or latent physical conditions or subsurface conditions at the Site that, (i) materially differ from the conditions indicated in the Agreement Documents, or (ii) are of an unusual nature, differing materially from the conditions ordinarily encountered and generally recognized as inherent in the Work at the general area of the Site. Caliche, rock, hard-digging or sandy/silty soil encountered on a project is not considered a "Differing Site Condition."

Drawings (Plans): Documents, which visually represent the scope, extent and character of the Work to be furnished and performed by Contractor during the construction phase and which have been prepared

or approved by the Design Professional and City. These documents include Drawings that have reached a sufficient state of completion and released by Design Professional solely for the purposes of review and use in performing constructability or bid-ability reviews by Contractor and in preparing cost estimates (e.g. Master Planning and Programming, Schematic Design, Design Development, and Construction Drawings), but *“not for construction.”* Shop Drawings are not Drawings as so defined.

Final Acceptance: The City's acceptance of the facility or project from the Contractor after all Work is completed, tested, and inspected in accordance with the Agreement requirements. Final Acceptance results in a Letter of Acceptance (LOA).

Fixed Price: A fixed price or amount for an Agreement Price, Scope of Work, materials, or other item under an Agreement, Change Order, or other agreement, which City agrees, in writing, to pay instead of the actual cost.

Float: The number of Days by which an activity can be delayed without lengthening the Critical Path and extending the Agreement Time. Unless otherwise expressly agreed in writing, all Float belongs to City.

Laws, Regulations, or Legal Requirements: Any and all applicable laws, rules, regulations, ordinances, codes and orders applicable to the Project of any and all governmental bodies, agencies, authorities and courts having jurisdiction and any applicable provisions of the Development Agreement for the Project (if any), including, without limitation, those provisions relating to the design and construction of the Project.

Line Item: The individual elements of Work identified on a bid or other schedule and associated with a price or a unit price and quantity particular to that individual element of the Work. Also refers to individual items of work within the Schedule of Values.

Liquidated Damages: Designated damages for the City to collect as compensation upon a specific breach (example: late delivery).

Long-Lead Item: Long-lead item refers to the equipment, product, or system that is identified at the earliest stage of a project to have a delivery time long enough to affect directly the Critical Path/the overall lead time of the project.

MAG: The Maricopa Association of Governments.

MAG Specifications: The most current version of the Uniform Standard Specifications for Public Works Construction published by MAG.

MAG Standard Details: The most current version of the Uniform Standard Details as published by MAG.

Minor change: A change in the Work having no impact on cost or time or the City-approved design intent, as determined by City.

Notice to Proceed (NTP): A written notice given by City to Contractor fixing the date on which Contractor will start to perform Contractor's obligations under the Agreement.

Project: The Project specified in the Agreement (including a Job Order).

Project Manager: The Project Manager designated in Article 1 of the Agreement, or any successor thereto

designated by City. The Project Manager has the authority to act on behalf of City, as delineated and limited by the Agreement Documents and applicable law. And City will communicate with Contractor through the Project Manager. However, the Project Manager has no authority to bind City or City Council in contravention of any City code, State or Federal statute or regulation, or these General Conditions.

Project Schedule: The schedule for the completion of the Project agreed to and required by City.

Project Specific Conditions: Additional conditions which apply to the specific Project and Scope of Work which are set forth in Exhibit D of the Agreement.

Project Team: The Project Team consisting of the Design Professional, Contractor, Project Manager, and such others as City may designate.

Punch List: The list initially prepared by Contractor pursuant to the Agreement Documents, reviewed and supplemented by the Project Manager (and at the sole option of the Project Manager, the Design Professional) and approved by City containing items of incomplete work not impacting Substantial Completion, if allowed for under the Agreement, and to be completed or corrected by Contractor after Substantial Completion and before Final Acceptance in accordance with the Agreement Documents.

Quality Assurance (QA) Testing: Testing performed to verify the accuracy and applicability of the QC testing results and to ascertain that the materials installed meet the specified levels of quality in accordance with the Agreement Documents.

Quality Control (QC) Testing: Testing performed to assure that the materials installed comply with the requirements in the Agreement Documents.

Requests for Information (RFIs): Formal written request from Contractor to City or Design Professional for the Project seeking clarification or additional information needed for Contractor to properly complete the Work or Services under the Agreement. City may require RFI's to be submitted on a specific form or in a specified format.

Schedule of Values (SOV): The specified document prepared by Contractor, and approved and accepted by City, which divides the Agreement Price into pay items, such that the sum of all pay items equals the Agreement Price for the construction phase Work, or for any portion of the Work having a separate specified Agreement Price.

Scope of Work: The scope of work agreed to or required by City and incorporated into the Agreement as Exhibit A.

Shop Drawings: All drawings, diagrams, schedules and other data specifically prepared for the Work by Contractor or a Subcontractor, Sub-subcontractor, manufacturer, supplier or distributor to illustrate some portion of the Work.

Site: The land or premises on which the Project is located.

Specifications: The part(s) of the Agreement Documents for the construction phase consisting of written technical descriptions of materials, equipment, construction systems, standards and workmanship as applied to the Work and certain administrative details applicable thereto. Where specified, the Project must be constructed using the current Uniform Standard Specifications and Details for Public Works

Construction as furnished by the Maricopa Association of Governments, as amended by City.

Subconsultant: A person, firm or corporation having an Agreement with Consultant/Contractor to furnish services required as its independent professional associate or consultant with respect to the Project.

Subcontractor: An individual or firm having a direct Agreement with Contractor or any other individual or firm having an Agreement with the aforesaid contractors at any tier, who undertakes to perform a part of pre-construction services or construction phase Work at the Site for which Contractor is responsible. Subcontractors must be selected through the Subcontractor selection process described in the Agreement Documents, if any.

Substantial Completion: The date when the City determines that the Work (or separable units of Phases as provided in the Agreement Documents) is essentially and satisfactorily complete in accordance with the Agreement Documents such that the Project is ready for use by the City for its intended purpose, opening to the general public, full occupancy or use by City (including, without limitation, all separate units, or rooms, facilities, access, income-generating areas, and all areas serving the general public, as applicable, must be ready for full operation without material inconvenience or discomfort), including, to the extent applicable to the Work, the following: all materials, equipment, systems, controls, features, facilities, accessories, and similar elements are installed in the proper manner and in operating condition, inspected, and approved; surfaces have been painted; masonry and concrete cleaned with any sealer or other finish applied; utilities and systems connected and functioning; site work complete; permanent heating, ventilation, air conditioning, vertical transportation, and other systems properly operating with proper controls; lighting and electrical systems installed, operable and controlled; paving completed, signage installed, and other work as applicable, has been performed to a similar state of essential and satisfactory completion.

Supplier: A manufacturer, fabricator, distributor, or vendor having a direct Agreement with Contractor or with any Subcontractor to furnish materials or equipment to be incorporated in the construction phase Work by Contractor or any Subcontractor.

Total Float: Number of Days by which pre-construction services or construction phase Work or any part of the same may be delayed without necessarily extending a pertinent Agreement Time or schedule milestone in the Project Schedule.

Work: The entire completion of construction or the various separately identifiable parts thereof, required to be furnished during the construction phase. Work includes and is the result of performing or furnishing labor and furnishing and incorporating materials, resources and equipment into the construction, and performing or furnishing services and documents as required by the Agreement Documents for the construction phase.

Writing: Typing, printing, photography and other modes of representing or reproducing words in a visible form, including email, and expressions.

SECTION 3 - STANDARD SPECIFICATIONS AND DETAILS

- 3.1 City operates under the latest revision of the MAG Specifications and MAG Standard Details as amended by City. City's current amendment to the MAG Specifications, part of the City's Unified Development Manual, may be found and downloaded from City's website at <http://www.chandleraz.gov>.

- 3.2 Copies of the MAG Specifications and MAG Standard Details are available at the Maricopa Association of Governments office, 302 N. 1st Avenue, Suite 300, Phoenix, Arizona. They may also be downloaded from their website at: <http://www.azmag.gov/Newsroom/Publications>
- 3.3 The MAG Specifications and Standard Details and City's amendments thereto are incorporated into the Agreement by this reference.

SECTION 4 - CONTRACTOR'S RESPONSIBILITIES FOR CONSTRUCTION SERVICES

4.1 GENERAL

- 4.1.1 Contractor must construct the Work in accordance with the Agreement Documents and as outlined in Exhibit A of the Agreement to the satisfaction of City, exercising the degree of professional care, skill, diligence, quality and judgment that a professional construction manager engaged, experienced and specializing in the construction management of construction and facilities of similar scope, function, size, quality, complexity and detail in urban areas throughout the United States comparable to Chandler, Arizona would exercise at such time, under similar conditions. Contractor must, at all times, perform the Work in conformance with sound and generally accepted engineering principles and construction management and construction contracting practices.
- 4.1.2 If Contractor observes errors, discrepancies or omissions in the Agreement Documents, Contractor must promptly notify the Design Professional and City and request clarification. If Contractor, with the exercise of reasonable care, should have recognized such error, inconsistency, omission, or difference and fails to report it to City, and if Contractor proceeds with the Work affected by such observed errors, discrepancies or omissions, without receiving such clarifications, Contractor does so at its own risk and will be liable to City for damages resulting from proceeding without clarification.
- 4.1.3 Project Team and agents of each of them, testing agencies and governmental agencies with jurisdictional interests will be provided access to the Work at reasonable times for their observation, inspection, and testing. Contractor must provide proper and safe conditions for such access.
- 4.1.4 Contractor must comply with, and require all Subcontractors to comply with, the Arizona Contractors' license laws, including all requirements with respect to being duly registered and licensed.
- 4.1.5 Contractor must ensure that all employees performing any Work for which Contractor is responsible have a legal right to live and work in the United States. In addition, all compensation of any such employee must meet all applicable requirements of the Fair Labor Standards Act (FLSA) and Federal Minimum Wage laws.
- 4.1.6 Contractor must comply with the Immigration Reform and Control Act of 1986 (IRCA). Contractor understands and acknowledges the applicability of the IRCA activities. Contractor agrees to comply with the IRCA while performing their work and to permit City inspection of Contractor personnel records to verify such compliance.
- 4.1.7 Pursuant to MAG Specifications §§ 107.4, Contractor must report immediately any discovery of archeological ruins or artifacts. Excavation must stop immediately so that City can decide on the pertinent steps to follow such discovery.

- 4.1.8 All property owners that may be affected by the proposed construction activities must be notified of the scope, duration of the construction activities and possible interference with their day-to-day activities by Contractor prior to start of construction. In addition, individual residential or commercial interferences, such as driveway restrictions, water outages, and all other Work adjacent to residence/business, require 48-hour notification in advance of specific adjoining Work. Notification may be through door hangers or other procedures approved by the City.
- 4.1.9 Access must be maintained to adjacent properties at all times during construction. Where property has more than one point of access, no more than one access will be restricted or closed at any one time. Access to adjacent private driveways will be maintained during all non-working hours.
- 4.1.10 Contractor must furnish and erect construction signs in accordance with Project Specifications. The signs must be professionally prepared and subject to approval by City, must be maintained by Contractor for the duration of the project, and must be removed by Contractor during the final project clean up.
- 4.1.11 The number of signs required, the size, shape, installation requirements and information to be included for construction signs is established on the detail sheet, provided, however, signs must be a minimum of 4 foot by 8 foot and must be installed so that the bottom of the sign is at least 4 foot above grade. No direct payment will be made for furnishing and erecting construction signs. The cost thereof must be included in other items for which direct payment is made. Sign locations will be determined by City.
- 4.1.12 All required construction signs must be installed by Contractor within 7 Days of Notice to Proceed.
- 4.1.13 The Work to be accomplished under these Agreement Documents has been designed for City by a Design Professional retained by City for this purpose. It is understood that normal construction Administration for the purpose of interpretation of the Agreement Documents is provided by City. Should any services of the Design Professional be required to assist in the corrections of errors or omissions by Contractor, or services of the Design Professional be required because of changes in structure or equipment where Contractor has requested approval of substitute methods or material, or any other items detailed herein below, those services will be provided by the Design Professional at the standard hourly rates previously negotiated with City and must be paid for by the Contractor.
- 4.1.14 Contractor must reimburse City for costs incurred by the Design Professional for additional services to the Project through no fault of City or the Design Professional including, but not limited to, the following conditions:
- a. Additional Site visits, investigations, inspections, design work or reports by the Design Professional which are required due to damages to existing facilities or completed Work caused by the Contractor in his performance, Contractor's negligence, or Contractor's Work which is rejected as defective or as failing to conform to the Agreement Documents;
 - b. Design Professional construction phase services rendered on the project during the time the project remains incomplete after the Agreement date of final completion will be charged to Contractor at a rate previously negotiated City; and

- c. All retesting required due to the failure of Contractor's Work to meet the requirements of the Agreement Documents will be at Contractor's expense. All standby and travel time by the City's testing lab, the Design Professional or City due to Contractor's inability to be prepared for testing at the agreed upon time will be at the Contractor's expense.

4.1.15 City may withhold from any payment otherwise due to Contractor any amounts necessary to pay the Design Professional for such additional services as provided herein above.

4.1.16 Contractor will not be required to bear additional costs incurred by City due to errors by the Design Professional.

4.2 **CONTRACTOR'S PRE-AGREEMENT AND PRE-WORK DELIVERABLES**

4.2.1 Prior to award of the Agreement, Contractor must execute Agreement and deliver to City. Failure to do so may delay Agreement award. Contractor must also provide to City its Contractor's License classification and number and its Federal Tax I.D. number.

4.2.2 Before beginning any Work under the Agreement, Agreement must be fully executed by City.

4.2.3 After Agreement award, City will issue to Contractor an award letter. At that time Contractor must deliver to City such bonds and certificates of insurance with endorsements in such amounts (and other evidence of insurance requested by City) required under Section 11 of these General Conditions, and as the Agreement requires.

4.2.4 As evidence of Workmen's Compensation Insurance, Contractor must, upon request, provide a letter of certification from the Industrial Commission of Arizona that Contractor is insured by the State Compensation Fund or is an authorized self-insurer or a certificate of insurance issued by an insurance company authorized by the Insurance Department of Arizona to write Workmen's Compensation and Occupational Disease Insurance in the State of Arizona.

4.2.5 Within 10 Days of the date of the executed Agreement letter issued by City, Contractor must submit to City for review and acceptance the following items:

4.2.5.1 Comprehensive construction Project Schedule including a Critical Path Method (CPM) diagram schedule as described in Section 6.2. Project Schedule must be in Microsoft Project standard file format. Within 10 Days of receipt of City's comments, Contractor must make all required corrections, adjustments, and additions to complete the Project Schedule and resubmit to City for review.

4.2.5.2 Preliminary schedule of submittals and Shop Drawings. Within 10 Days of receipt of City's comments, Contractor must submit the corrected and completed schedule of Shop Drawings submissions for approval. Contractor's schedule of Shop Drawings and sample submittals will be acceptable to City if it provides a workable arrangement for reviewing and processing the required submittals.

4.2.5.3 Schedule of Values in a form specified by City reflecting the subcontracts and other categories that will be used to submit Pay Applications for the Work. The total amount of the Schedule of Values must not be greater than the Agreement Price. The Schedule of Values will be reviewed at the Pre-Construction Conference and revised by Contractor within 10 Days after Pre-Construction Conference in response to comments and questions from

City. Once accepted by City in writing, the Schedule of Values for the Project must not be changed without the prior written approval of City.

4.2.6 Video Recording Requirement. Prior to performing any Work, Contractor must document the existing conditions of the Site, all other areas where Work will occur and all adjacent areas that may be impacted by the Work via digital video format. Contractor must video record and index all areas, features, buildings and other public and private improvements that could potentially be impacted by the Work. Video recording must be coordinated with City. When video recording private property, Contractor must also coordinate the video recording with the private property owner, if possible. Contractor must provide City with a copy of said digital video format prior to performing any Work.

4.2.7 Aerial Drone Construction Photography.

4.2.7.1 If Agreement duration is greater than 90 calendar days, Contractor must engage a professional unmanned aerial vehicle (UAV) aerial pilot to photograph the Site prior to construction mobilization, at three-month intervals during construction, and following final inspection. Drone camera specifications must meet the following minimum requirements:

- a. 1-inch CMOS
- b. Pixels: 20M
- c. FOV 84 8.8 mm/24 mm (35 mm format equivalent) f/2.8-f/11 auto focus at 1 m-
- d. For photographing: 16.9 Aspect Ratio: 5472x3078
- e. For video shooting: MP4/MOV/H.264
- f. FHD: 1920x1080 120p @100Mbps
- g. File format: High Definition (HD) JPEG for digital photos and HD MPEG 4 for digital video.
- h. All metadata to be recorded including GPS data and preserved with photographs provided.

Interval	JPEG
3 month intervals	At an altitude (AGL) between 70-90 ft.
3 month intervals	Images to be taken every 50-100 ft. to be determined based on project scope.

4.2.7.2 Drone photos to be taken in sequential geographical order and then organized and provided in the same manner unless otherwise specified.

4.2.7.3 Photos to be provided digitally via an online file share service and/or by a USB drive to contractor.

4.2.7.4 Drone pilots to obey ALL local (city, county, state) UAV regulations as well as FAA UAV guidelines including, but not limited to, conducting all flights during daylight hours, not exceeding maximum altitude ceilings (depending on area), not flying over people, yielding to other aircraft.

4.2.7.5 Drone pilots must fly drone within visual line of sight (VSOL) and have visual spotter when needed. Drone pilots only to operate in favorable weather conditions when minimum visibility is 3 miles or greater.

4.2.7.6 Drone pilots to conduct a preflight checklist and visually inspect the entire flight path prior to flying to ensure a safe flight.

- 4.2.7.7 **Airspace Authorizations.** Operations in Class G airspace are allowed without air traffic control (ATC) permission. Operations in Class B, C, D and E airspace need ATC authorization. Drone pilots to schedule each flight in advance and based on airspace if required will notify nearby airports/control towers, etc.
- 4.2.8 **Government Approvals and Permits.**
- 4.2.8.1 Contractor must obtain all necessary permits for the Work and pay all applicable fees, unless otherwise noted on the Plans and in the Specifications. City permit fees will be paid internally by City. For bidding purposes, an allowance for all permit fees is included in the bid schedule under the item "allowance for permit fees." The Contractor will be paid for the actual cost of the permit fees upon submitting a receipt showing the fee Contractor has paid. Excluded from the above allowance are items such as all costs incurred by the Contractor in securing the permit except for the actual permit fee established by the agency, cost for all shutdowns or outages, cost for pole bracing, cost of permits for construction water, cost of construction water, cost for any additional insurance requirements, cost for any licenses, and other similar type costs. Contractor is specifically notified of the need to obtain the necessary environmental permits or file the necessary environmental and regulatory permit notices.
- 4.2.8.2 Copies of all permits and the associated notices must be provided to City prior to starting the permitted activity.
- 4.3 **PRE-CONSTRUCTION CONFERENCE**
- 4.3.1 Prior to the commencement of any Work, City will schedule a Pre-Construction Conference.
- 4.3.2 The purpose of this Conference is to establish a working relationship between Contractor, the utility firms, and various City agencies. The agenda will include critical elements of the Work schedule, submittal schedule, cost breakdown of major lump sum items, Payment Requests and processing, coordination with the involved utility firms, and emergency telephone numbers for all representatives involved in the course of construction.
- 4.3.3 Minimum attendance by Contractor at any mandatory meeting with City must be (1) Contractor's Representative, who is authorized to execute and sign documents on behalf of the firm, (2) Contractor's on-site Superintendent, and (3) Contractor's Safety Office, or other employee responsible for safety.
- 4.4 **PERFORMANCE OF THE WORK (INCLUDING FIELD MEASUREMENTS, SUBCONTRACTORS, AND SUPPLIERS)**
- 4.4.1 Unless otherwise provided in the Agreement Documents to be the responsibility of City or a separate Contractor, Contractor must provide through itself or Subcontractors the necessary supervision, labor, inspection, testing, start-up, material, equipment, machinery, temporary utilities and other temporary facilities to permit Contractor to complete the Work consistent with the Agreement Documents.
- 4.4.2 Contractor must perform all construction activities efficiently and with the requisite expertise, skill and competence to satisfy the requirements of the Agreement Documents. Contractor must at all times exercise complete and exclusive control over the means, methods, safety, sequences and techniques of construction.
- 4.4.3 Contractor's Superintendent must be present at the Site at all times that material Work

under this Agreement is taking place. Contractor's Superintendent or designee must be present at the Site at all times any other Work under this Agreement is taking place. Superintendent must not be replaced without written notice to City. Whenever the Superintendent is not present at a particular part of the Work where the City or Design Professional may desire to inform the Contractor relative to interpretation of the Drawings and Specifications or to disapproval or rejection of materials or Work performed, the City or Design Professional may provide such information in writing to the foreman or other worker in charge of the particular part of the Work in reference to which the information is given. Information so given will be as binding as if given to the Superintendent.

4.4.4 All elements of the Work must be under the direct supervision of a foreman or his designated representative on the Site who must have the authority to take actions required to properly carry out that particular element of the Work.

4.4.5 Working Hours. Except in connection with the safety or protection of persons or the Work or property at the Site or adjacent thereto, and except as otherwise indicated, all Work at the Site must be performed during regular working hours, and Contractor will not permit overtime work or the performance of Work on Saturday, Sunday or any legal holiday without City's written consent given after prior written notice to City. If it will become absolutely necessary to perform Work at night or on Saturdays, Sundays or legal holidays, the City must be informed at least 24 hours in advance of the beginning of performance of such Work. Only such Work will be done at night as can be done satisfactorily as determined by the City. Good lighting and all other necessary facilities for carrying out and inspecting the Work must be provided and maintained at all points where such Work is being done. Further, unless such non-normal work hours are performed at City's request or required by the Agreement Documents, Contractor must pay to City all additional costs incurred by City by reason of such non-normal working hours. Expenses incurred by City for overtime compensation must be reimbursed by Contractor as follows: (i) City staff at the rate set forth in current City Fee Schedule as published on City website, (ii) Design Professional and staff at the standard hourly rates previously negotiated with City, and (iii) all others at actual cost plus ten percent administrative overhead. Such costs may be deducted by City from any payments due to Contractor. Provided, however, if overtime work or work during other than normal hours is at the request of City and not due to Contractor delay, City will pay the cost of City overtime expenses.

4.4.6 Where the Agreement Documents require that a particular product be installed or applied by an applicator approved by the manufacturer, it is Contractor's responsibility to ensure the Subcontractor employed for such work is approved by the manufacturer. All materials and equipment must be stored, applied, installed, connected, erected, used, cleaned and conditioned in accordance with the instructions of the applicable manufacturer, fabricator, supplier or distributor, except as otherwise provided in the Agreement Documents; but no provisions of any such instructions will be effective to impose on City or Design Professional responsibility for the means, methods, techniques, sequences or procedures of construction or for safety precautions incident thereto.

4.4.7 Before starting the Work, Contractor must carefully study and compare the various Plans, Drawings, other Agreement Documents, and Specifications relative to that portion of the Work, as well as the information furnished by City, must take field measurements of any existing conditions related to that portion of the Work and must observe any conditions at the Site affecting it. The exactness of grades, elevations, dimensions, or locations given on any Drawings, or the Work installed by other contractors, is not guaranteed by City.

- 4.4.8 Before ordering materials or doing Work, Contractor and each Subcontractor must verify measurements at the Site and will be responsible for the correctness of such measurements. No extra charge or compensation will be allowed because of differences between actual dimensions and the dimensions indicated on the Agreement Documents, including the Drawings.
- 4.4.9 Ground Level Construction Photography.
- 4.4.9.1 The Contractor must furnish progress photographs of the project. The photographer selected by the Contractor must be approved by the City and must be either a commercial photographer or an individual experienced and equipped for such photography.
- 4.4.9.2 The Contractor must deliver to City all photographs taken during that period with each application for payment. If the current photographs do not accompany the application, the application will not be reviewed and will be returned to the Contractor as incomplete.
- 4.4.9.3 Photographs must be identified by use of typewritten labels affixed to the back of the photograph. The label must provide a description of the view, the direction from which the photograph was taken, the name of the project, City's project number, the name of Contractor and the date of the photography. The stationing must also be included for all pipeline installations.
- 4.4.9.4 Photographs must be taken during the construction period and must be of aesthetic composition and depict the progress of the Work from the beginning of construction through and including the finished product. City may vary the specified frequency so that significant progress or changes can be recorded on the photographs.
- 4.4.10 Underground Facilities.
- 4.4.10.1 The existence and number of facilities as shown on the Plans are estimated from information furnished by the particular utility. Contractor is responsible for field verification and location of all utilities prior to the start of construction. No field work will be allowed to start until Contractor has contacted Arizona 811 and all affected utilities have been located. In addition, Contractor must expose and physically locate all potentially conflicting utilities prior to construction. The actual locations of the utilities must be compared to locations shown on the Plans and any required changes in alignment and grade must be made at the time of construction in consultation with Project Manager. It is generally recognized and Contractor should anticipate that information from Arizona 811 or information from utility companies during project design, frequently fails to disclose all underground facilities. The fact that more utility lines or other underground facilities are located in the Project Site than shown on the Project Plans does not constitute an "unforeseen Condition" and such undisclosed underground facilities do not differ materially from the conditions which Contractor should expect. The provisions of Sections 105.4, 105.6, 107.11 and 109.8.1 of the MAG Uniform Standard Specifications for Public Works Construction apply and are incorporated herein by this reference.
- 4.4.10.2 Contractor is responsible for all coordination with utility companies. The provisions of Sections 105.4, 105.6, 107.11 and 109.8.1 of the MAG Uniform Standard Specifications for Public Works Construction strictly apply and no additional compensation will be paid to Contractor for delays due to utility work on the project.
- 4.4.11 Relocation of Existing Water Meters. When a service line has been extended and a line

setter installed in a meter box, City forces will re-install meter. No compression fittings will be utilized.

4.4.12 Water Turn-On or Turn-Off.

4.4.12.1 Contractor must coordinate all water line turn-ons and turn-offs through the City. Application must be made to the Municipal Utility Division and Contractor must pay the established charges. The City will close existing valves, but will not guarantee a bone-dry Shutdown.

4.4.12.2 Contractor must notify all customers affected by the turn-off not less than 48 hours in advance. Notification must be in writing, must give the reason for the turn-off and must give the estimated time and duration that water service will be interrupted. Contractor is also notified that water turn-off will not be permitted on the Day before and after Thanksgiving Day and Christmas Day.

4.4.12.3 No direct payment will be made to Contractor for turn-ons or turn-offs. Costs associated therewith will be included in other items for which direct payment is made.

4.4.13 Tests and Inspections.

4.4.13.1 Contractor must give City timely (at a minimum, twenty-four hours) notice of readiness of the Work for all required inspections, tests or approvals. Contractor must give timely notice to City in advance of backfilling or otherwise covering any part of the Work so that city representative may, if desired, observe such part of the Work before it is concealed. Whenever Contractor varies the normal period during which Work or any portion of it is carried on each Day, Contractor must give timely notice to City so that city representative may, if desired, be present to observe the Work in progress. If Contractor fails to give such timely notice, any Work done in the absence of city representative will be subject to rejection. If Contractor gives such notice to City, but then is not ready for such inspections, tests, approvals or observations at the time so noticed, Contractor must reimburse City for all costs incurred by the attendance of city representatives.

4.4.13.2 If any law, ordinance, rule, regulation, code, or orders of any public body having jurisdiction requires any Work (or part thereof) to be inspected, tested or approved, Contractor (unless another party is specified in the Agreement Documents) must assume full responsibility therefor, pay all costs in connection therewith and furnish City the required certificates of inspection, testing, or approval. Contractor must also be responsible for and must pay all costs in connection with any inspection or testing required by the Specifications in connection with City's acceptance of a manufacturer, fabricator, supplier or distributor of materials or equipment proposed to be incorporated in the Work, or of materials or equipment submitted for approval prior to Contractor's purchase thereof for incorporation in the Work. The cost of all other inspections, tests and approvals required by the Agreement Documents will be paid by City (unless otherwise specified).

4.4.13.3 All inspections, tests or approvals other than those required by law, ordinance, rule, regulation, code or order of any public body having jurisdiction must be performed by organizations acceptable to City and by the Design Professional if so specified.

4.4.13.4 Neither observations by City, the Design Professional nor inspections, tests or approvals by others will relieve Contractor from their obligations to perform the Work in accordance with the Agreement Documents.

- 4.4.14 Uncovering Work. If any Work that is to be observed, inspected, tested or approved is covered without written concurrence of City, it must, if requested by City be uncovered for observation. Unless Contractor has given City timely notice of Contractor's intention to cover such Work and City has not acted with reasonable promptness in response to such notice, Contractor must furnish all necessary labor, material and bear all the expenses of such uncovering, exposure, observation, inspection and testing and of satisfactory reconstruction, including compensation for additional professional services, and an appropriate deductive Change Order will be issued.
- 4.4.15 In all cases of interconnection of its Work with existing or other Work, Contractor must verify at the Site all dimensions relating to such existing or other Work. Any errors due to Contractor's failure to so verify all such grades, elevations, locations or dimensions must be promptly rectified by Contractor without any increase in the Agreement Price. Any design errors or omissions noted by Contractor during this review must be reported promptly to City.
- 4.4.16 Contractor must establish and maintain all construction grades, lines, levels, and benchmarks, and will be responsible for accuracy and protection of same. This Work must be performed or supervised by a licensed civil engineer or surveyor in the State of Arizona.
- 4.4.17 Contractor must photograph all buried piping of greater than four (4) inches in diameter prior to backfill.
- 4.4.18 Contractor is responsible for the proper performance of the work of Subcontractors and any acts and omissions in connection with such performance. Nothing in the Agreement Documents is intended or deemed to create any legal or contractual relationship between City and any Subcontractor or Sub-Subcontractor, including but not limited to any third-party beneficiary rights.
- 4.4.19 Contractor must coordinate the activities of all Subcontractors. Contractor must coordinate performance of the Work with City's Public Works & Utilities Department and other departments or agencies within City. The Design Professional and other contractors or parties involved in the Project. If City performs other work on the Project or at the Site with separate contractors under City's control, Contractor agrees to cooperate and coordinate its activities with those of such separate contractors so that the Project can be completed in an orderly and coordinated manner without unreasonable disruption.
- 4.4.20 Contractor will not substitute or change any Subcontractor or Supplier without the prior written approval of City. Any substitute or replacement Subcontractor or Supplier must be required to meet the same qualifications and selection criteria and process as the original Subcontractor or Supplier. If a Subcontract/Supplier selection plan has been approved by City, Contractor will follow that plan unless otherwise approved by City in writing.
- 4.4.21 Contractor must not change or replace any members of its Project team, including Contractor's Representative, Project Manager, or Superintendent, without an explanation for the change being given to City, and receiving prior written approval of the change from City, which approval will not be unreasonably withheld.
- 4.4.22 Subcontractors whose scope of work has a value greater than 15% of the total Agreement Price are required to furnish performance and payment bonds to Contractor, unless

otherwise approved in writing by City.

- 4.4.23 Contractor must comply with MAG Specification § 108.2 (E) unless otherwise specified in Agreement Documents.

4.5 **CONTROL OF THE PROJECT SITE**

- 4.5.1 Throughout all phases of construction, including suspension of Work, Contractor must keep the Site reasonably free from debris, trash and construction wastes to permit Contractor to perform its construction services efficiently, safely and without interfering with the use of adjacent land areas. Prior to Final Acceptance of the Work, or a portion of the Work, Contractor must remove all debris, trash, construction wastes, materials, equipment, machinery and tools arising from the Work or applicable portions thereof to permit City to occupy the Project or a portion of the Project for its intended use.
- 4.5.2 Contractor must take whatever steps, procedures or means necessary to prevent dust nuisance due to construction operations. The dust control measures must be maintained at all times to the satisfaction of City and in accordance with the requirements of the Maricopa County Bureau of Air Pollution Control Rules and Regulations.
- 4.5.3 Contractor must maintain Americans with Disabilities Act (ADA) and American National Standards Institute (ANSI) accessibility requirements during construction activities, including without limitation compliance with the 2010 regulations governing implementation of the ADA to the extent applicable. ADA and ANSI accessibility requirements must include, but not be limited to, parking, building access, areas of refuge, and emergency exit paths of travel. Contractor is responsible for the coordination of all Work to minimize disruption to residents and the public.
- 4.5.4 Only materials and equipment used directly in the Work will be brought to and stored on the Site by Contractor. When equipment is no longer required for Work, it must be removed promptly from the Site. Protection of construction materials and equipment stored at the Site from weather, theft, damage and all other adversity is solely the responsibility of the Contractor.
- 4.5.5 Contractor agrees all persons working on the Site must act at all times in the best interest of the Project and will comply with all applicable rules and regulations reasonably set forth by City related to the Site. Notwithstanding the foregoing or anything in this Agreement to the contrary, City may remove from the Site any individual who City deems in their reasonable discretion to be creating a disturbance or causing any problem on the Site.
- 4.5.6 Contractor will be responsible to City for the acts and omissions of Contractor's employees, Subcontractors and their agents and employees, and any other person performing any of the Work under an Agreement with Contractor, or claiming by, through or under Contractor, for all damages, losses, costs and expenses resulting from such acts or omissions.
- 4.5.7 City may conduct criminal, drive history, and all other requested background checks of Contractor and Subcontractor personnel performing Work or who have access to City's information, data, or facilities in accordance with City's current background check policies, or the provisions of the Project Specific Conditions. Any officer, employee or agent that fails to background check must be replaced immediately.
- 4.5.8 City will have a final authority, based upon security reasons: (i) to determine when

security clearance of Contractor's and Subcontractor's personnel is required; (ii) to determine the nature of the security clearance, up to and including fingerprinting personnel; and (iii) to determine whether or not any individual or entity may provide Services or perform Work under the Agreement.

- 4.5.9 If City objects to any personnel for any reasonable cause, then Contractor must, upon notice from City, remove such individual from the Project.

4.6 **PROJECT SAFETY**

- 4.6.1 The Project and all Work performed in relation thereto is governed by applicable provisions of the federal laws, including but not limited to, the latest amendments of the following:

- a. Williams-Steiger Occupational Safety & Health Act of 1970, Public Law, 91-596.
- b. Part 1910 and Part 1926 – Occupational Safety and Health Standards, Chapter XVII of Title 29, Code of Federal Regulations.
- c. Part 1518 – Safety and Health Regulations for Construction, Chapter XIII of Title 29, Code of Federal Regulations.

- 4.6.2 Contractor is responsible for safety of the job Site for employees of Contractor as well as for members of the general public and others who may drive or walk through or be at the Site.

- 4.6.3 Contractor recognizes the importance of performing the Work in a safe manner so as to prevent damage, injury or loss to: (i) all individuals at the Site, whether working or visiting; (ii) the Work, including materials and equipment incorporated into the Work and stored On-Site or Off-Site; and (iii) all other property at the Site or adjacent thereto, including trees, shrubs, lawns, walks, pavements, roadways, structures and utilities not designated for removal, relocation or replacement in the course of construction; and (iv) the owners or tenants of adjacent property and their patrons, employees and invitees.

- 4.6.4 Contractor assumes responsibility for implementing and monitoring all safety precautions and programs related to the performance of the Work.

- 4.6.5 Contractor must provide a "competent person" as required by O.S.H.A regulations. The "competent person" must be identified at the Pre-Construction Conference with City advised in writing of any changes.

- 4.6.6 The "competent person" must make routine daily inspections of the Site and must hold weekly safety meetings with Contractor's personnel, Subcontractors and others as applicable.

- 4.6.7 Contractor and Subcontractors must comply with all legal and regulatory requirements relating to safety, as well as any City specific safety requirements set forth in the Agreement Documents, provided that such City-specific requirements do not violate any applicable legal and regulatory requirements.

- 4.6.8 Contractor will immediately report in writing any safety-related injury, loss, damage or accident arising from the Work to Project Manager and, to the extent mandated by Legal Requirements, to all government or quasi-government authorities having jurisdiction over safety-related matters involving the Project or the Work.

- 4.6.9 Contractor's responsibility for safety under this Section is not intended in any way to relieve Subcontractors and Sub-Subcontractors of their own contractual and legal obligations and responsibility for (i) complying with all Legal Requirements, including those related to health and safety matters, and (ii) taking all necessary measures to implement and monitor all safety precautions and programs to guard against injury, losses, damages or accidents resulting from their performance of the Work.
- 4.6.10 As between City and Contractor, Contractor is responsible to City for any and all the safety issues relating to the Work on the Project. Contractor must administer and manage the safety program. This will include, but not necessarily be limited to review of the safety programs of each Subcontractor. Contractor must monitor the establishment and execution of compliance with all applicable regulatory and advisory agency construction safety standards. Contractor's responsibility for review, monitoring, and coordination of the Subcontractor's safety programs will not extend to direct control over execution of the Subcontractors' safety programs. Notwithstanding Contractor's safety obligations to City, it is agreed and understood that each individual Subcontractor will remain controlling employer responsible for the safety programs and precautions applicable to its own work and the activities of other's work in areas designated to be controlled by such Subcontractor for purposes of workers compensation insurance coverage.
- 4.6.11 Nothing in this agreement will relieve Contractor of his responsibility to maintain traffic, structures, etc., as noted on the Plans, Specifications, and Project Specific Conditions. Contractor is responsible to provide all necessary shoring, bracing and trench support as is necessary to maintain traffic structures, etc., as stipulated in the Plans, Specifications, and Special Provisions. If the stability of adjoining building, walls, roadways, etc., is endangered by Contractor's excavation, shoring, bracing, or under pinning must be provided as necessary to ensure project safety. Cost for shoring, bracing, underpinning, and trench support will be included in the appropriate items listed in the Agreement Price, and no additional payment will be made for this work.
- 4.7 **MATERIALS QUALITY, SUBSTITUTIONS, AND SHOP DRAWINGS**
- 4.7.1 **Quality Control and Quality Assurance Testing.**
- 4.7.1.1 All construction materials to be used or incorporated in the Project are subject to inspection, Quality Control & Quality Assurance Testing, and approval or rejection by City. Any material rejected by City must be removed immediately and replaced in an acceptable manner to City at no additional cost to City. When QC/QA tests indicate noncompliance with the Agreement Documents, retesting must be performed by the same testing laboratory that performed the tests that indicated noncompliance.
- 4.7.1.2 The Contractor must establish, provide, and maintain an effective Quality Control Testing Program (QCTP). The Contractor must develop his own program or procure the services of a consultant. In either case, the party performing the tests must be currently certified by the National Bureau of Standards in the National Voluntary Laboratory Accreditation Program (NVLAP) for construction services or the AASHTO Accreditation Plan (AAP) for Soils, Asphalt and Concrete. The Contractor must provide all support necessary to perform QC and QA testing and sampling (i.e. shoring for testing trench backfill, backhoes, motor graders, loaders, etc. to facilitate testing and sampling). The City will perform the QA testing.
- 4.7.1.3 The Contractor must submit a written QCTP to the City as a required submittal. The Contractor must not begin Work until the Quality Control Program has been reviewed and

- accepted by the City. Resumes of all personnel that will be associated directly or indirectly with the QCTP must be included.
- 4.7.1.4 The QCTP must include, but not be limited to, on-site/field and laboratory testing of all material delivered to the Site and any existing materials or conditions pertinent to the project.
- 4.7.1.5 All testing must be under the direction of a Professional Engineer registered in the State of Arizona, knowledgeable in Materials Testing. All "Test Report" forms must be stamped by said Engineer.
- 4.7.1.6 The written QCTP will set forth the responsibilities of the engineer, project manager, supervisory personnel and each technician assigned to this project. Substitutions or replacement of personnel must require prior written approval by the City. All personnel must be proficient within their assigned duties and possess certification(s) commensurate with their position and responsibilities. The minimum certification(s) for each technician must be NICET Level II, Arizona Technical Testing Institute, American Concrete Institute, or other nationally recognized program applicable to the project and approved by the City of Chandler. The written QCTP must include a description of the required field and construction materials laboratory tests, including required frequencies that meet the minimums established herein.
- 4.7.1.7 The Contractor must establish a system to record and report all material test results. The daily test reports must include, but not be limited to:
- a. Test designation;
 - b. Date of test;
 - c. Name of tester;
 - d. Location of test/sample (station and offset);
 - e. Product suppliers and product codes (as applicable);
 - f. Depth/elevation of test/sample;
 - g. Test result;
 - h. Control requirement(s);
 - i. Cause of rejection (if applicable);
 - j. Results of retests (if applicable); and
 - k. Remedial action (if applicable).
- 4.7.1.8 The Contractor must submit test results to the designated City representative.
- 4.7.1.9 The Contractor must also submit a weekly report to the City summarizing the testing and construction activities completed by emailing the report to the email addresses noted above. All weekly reports must be submitted simultaneously to the Contractor and the City of Chandler. The report must include individual summary sheets for each utility line, structure, and portion of the pavement section. Cores must be numbered sequentially throughout the Project. Re-cores must reference the original core by number and must contain the averaged values for thickness and density. Total pavement thickness must be reported. Vertical location of tests for underground utilities must indicate the depth of the

excavation at the location of the test (i.e., cut to flow line [if applicable], depth to bottom or top of pipe, etc.). Density tests must be numbered sequentially. If the minimum number of tests has not been performed per the written QCTP, this must be stated in the weekly summary report with an explanation of the circumstances.

- 4.7.1.10 The City will maintain a copy of the Project test results and weekly reports in the Project file. In cases where quality control activities do not comply with the Agreement provisions, the City may:
- a. Order the Contractor to replace ineffective or unqualified quality control personnel.
 - b. Order the Contractor to stop operations until appropriate corrective action is taken.
- 4.7.1.11 Although minimum testing requirements are specified herein, the Contractor bears full responsibility for the quality of the materials and their installation and may elect to perform additional testing beyond the requirements set forth herein to ensure compliance.
- 4.7.1.12 The Quality Control requirements contained in this Section are in addition to and separate from Quality Assurance Testing, which will be performed by the City of Chandler or its representative. If the Quality Assurance test results are not in agreement with the Quality Control test results, the Contractor will have the option to retain a third party consultant for referee tests. The third party consultant must meet the same requirements as the consultant performing the Quality Control Testing. The results of the third party will be binding. All cost incurred by the referee testing will be the Contractor's expense. If the Contractor elects not to retain a third party for referee testing, the City of Chandler test results will prevail.
- 4.7.1.13 Except as otherwise noted within this Section, Work or materials required by this Section are non-pay items. Per MAG Section 101, a non-pay item is an item of Work for which no separate payment will be made, the cost of which is to be included as an incidental cost for associated item(s) included on the Bid Schedule or Schedule of Values.
- 4.7.2 Trade Names and Substitutions.
- 4.7.2.1 Substitutions prior to bid will only be considered if in compliance with Arizona Revised Statute § 34-104.
- 4.7.2.2 Contractor, if requested by City, must submit Samples or any additional information that may be necessary to evaluate the acceptability of the substitution.
- 4.7.2.3 City will make the final decision and will notify Contractor in writing as to whether the substitution has been accepted or rejected.
- 4.7.2.4 If City does not respond within 15 working days, Contractor must continue to perform the Work in accordance with the Agreement Documents and the substitution will be considered rejected.
- 4.7.3 Shop Drawings.
- 4.7.3.1 Contractor must prepare and submit Shop Drawings which show details of all Work to insure proper installation of the Work using those materials and equipment specified under the approved Plans and Specifications.

- 4.7.3.2 Contractor must submit a schedule of Shop Drawing submissions, which avoids bulk submissions to the extent reasonably possible, with the Project Schedule for City approval. The schedule of Shop Drawing submissions must include all of the items for which Shop Drawings are required by the Agreement Documents, including the Specifications. Unless otherwise noted, Shop Drawings will not be required for items specified or detailed in the Uniform Standard Specifications and Details or the Technical Specifications.
- 4.7.3.3 Shop Drawings must be numbered consecutively for each Specification section and must accurately and distinctly present the following:
- a. All working and erection dimensions.
 - b. Arrangements and sectional views.
 - c. Necessary details, including complete information for making connections between work under this Agreement and work under other Agreements.
 - d. Kinds of materials and finishes.
 - e. Parts list and description thereof.
- 4.7.3.4 Each Drawing or page must include:
- a. Project Name, City of Chandler Project Number and descriptions.
 - b. Submittal date and space for revision dates.
 - c. Identification of equipment, product or material.
 - d. Name of Contractor and Subcontractor.
 - e. Name of Supplier and Manufacturer.
 - f. Relation to adjacent structure of material.
 - g. Physical dimensions clearly identified.
 - h. ASTM and Federal Specifications references.
 - i. Identification of and justification for deviations from the Agreement Documents.
 - j. Contractor's stamp, initialed or signed, dated and certifying the review of submittal, certification of field measurements and compliance with Agreement.
 - k. Location at which the equipment or materials are to be installed.
- 4.7.3.5 Location will mean both physical location and location relative to other connected or attached material. City will return unchecked any submittal, which does not contain complete data on the Work and full information on related matters.
- 4.7.3.6 Stock or standard drawings will not be accepted for review unless full identification and supplementary information is shown thereon in ink or typewritten form.
- 4.7.3.7 Contractor must schedule, prepare and submit all Shop Drawings in accordance with a timetable that will allow its suppliers and manufacturers sufficient time to fabricate, manufacture, inspect, test and deliver their respective products to the project Site in a timely manner so as to not delay the complete performance of the Work.

- 4.7.3.8 If the Shop Drawings show departures from the Agreement requirements, Contractor must make specific mention thereof in the Letter of Transmittal; otherwise review of such submittals by City will not constitute review of the departure. Review of the Drawings will constitute review of the specific subject matter for which the Drawings were submitted and not of any other structure, material, equipment, or apparatus shown on the Drawings.
- 4.7.3.9 The review of Shop Drawings will be general and will not relieve Contractor of responsibility for the accuracy of such Drawings, nor for the proper fitting and construction of the Work, nor for the furnishing of materials or Work required by the Agreement. No construction called for by Shop Drawings will be initiated until such Drawings have been reviewed and approved by City.
- 4.7.3.10 The procedure in seeking review of the Shop Drawings will be as follows:
- a. Contractor must submit complete sets of Shop Drawings and other descriptive data as specified in this Section.
 - b. After Contractor's submittal or resubmittal of Shop Drawings, if Contractor has submitted Shop Drawings in accordance with the City-approved submittal schedule, or upon resubmission, City will be provided with three (3) calendar weeks for review. Should City require additional review time above and beyond the three (3) calendar weeks, Contractor may ask for a time extension or monetary compensation, if they can present valid, factual evidence that actual damages were incurred by Contractor. City will determine the amount of the time extension or the monetary compensation to be awarded Contractor, if any, in accordance with City's Policy Statement for Calculating Delays and Damages, Appendix 1.
- 4.7.3.11 Contractor will be responsible for all extra costs incurred by City caused by Contractor's failure to comply with the procedure outline above.
- 4.7.4 Long Lead Time Items. Contractor must submit Shop Drawings, as required by the Engineer, on all long lead items to be furnished and installed as part of the project within 10 Days after the date of the executed Agreement letter issued by City. In addition, Contractor must order all long lead items to be furnished and installed as part of this Project within 3 Days after receiving approved Shop Drawings. For all long lead times for which Shop Drawings are not required, Contractor must order said long lead items within 15 Days after the date of the executed Agreement letter issued by City. Within 2 Days after ordering long lead items, Contractor must supply copies of all purchase orders, along with an accurate delivery schedule from the supplier.
- 4.7.5 Construction Water. If Contractor uses water from City's water system for construction water, Contractor must obtain a fire hydrant meter from City of Chandler Utility Services (480-782-2280) and all construction water must be obtained through the hydrant meter. Contractor must pay all fees related to the hydrant meter and all water bills for construction water. All cost for meters and construction water will be included in the Agreement Price.
- 4.8 **PROJECT RECORD DOCUMENTS**
- 4.8.1 During the construction period, Contractor must maintain at the jobsite a full-size set of prints of the Construction Document Drawings and Shop Drawings for Project Record Document purposes.

- 4.8.2 Contractor must mark these Drawings to indicate the actual installation where the installation varies from the original Construction Documents. Contractor must give particular attention to information on elements that will be concealed, which would be difficult to identify or measure and record later. Items required to be marked include but are not limited to:
- a. Dimensional changes to the Drawings.
 - b. Revisions to details shown on Drawings.
 - c. Locations and depths of underground utilities.
 - d. Revisions to routing of piping and conduits.
 - e. Actual equipment locations.
 - f. Changes made by Change Order or Addendum.
 - g. Details not on original Agreement Drawings.
- 4.8.3 Contractor must mark completely and accurately Project Record Drawing sets of Construction Documents.
- 4.8.4 Contractor must mark Project Record Drawings sets with red erasable colored pencil.
- 4.8.5 Contractor must note Request for Information (RFI) Numbers and Change Order numbers, etc., as required to identify the source of the change to the Construction Documents.
- 4.8.6 Contractor must submit Project Record Drawing sets and Shop Drawings to City or its representative for review and comment.
- 4.8.7 Upon receipt of the reviewed Project Record Drawings from City, Contractor must correct any deficiencies and omissions to the Drawings and submit the final original of the Project Record Drawings to City prior to Final Payment.
- 4.8.8 Project Manager will review the Project Record Drawings monthly prior to the date established for the Payment Request and will be the sole judge of acceptance of these Drawings.
- 4.9 **WARRANTY AND CORRECTION OF DEFECTIVE WORK**
- 4.9.1 Contractor warrants to City that the construction, including all materials and equipment furnished as part of the Work, will be new unless otherwise specified in the Agreement Documents, of good quality, and free of defects in materials and workmanship. Contractor's warranty obligation excludes defects caused by abuse, alterations, or unreasonable failure to maintain the construction by persons other than Contractor, subcontractors, or others under Contractor's control. Nothing in this warranty will limit any manufacturer's warranty which provides City with greater warranty rights than set forth herein or in the Agreement. Contractor will provide City with all manufacturers' warranties and operation and maintenance manuals upon substantial completion of the Work. Contractor's warranty must be for one (1) year, in accordance with MAG Specification § 108.8, and will commence for all portions of the Work upon Final Acceptance of the entire Work as determined by City under the Agreement. All statutory or other warranties, express or implied, related to latent defects will remain in force and are not limited by this provision.
- 4.9.2 City May Stop the Work. If the Work is defective, or Contractor fails to supply sufficient skilled workmen or suitable materials or equipment, City may order Contractor to stop the

Work without cost to City, or any portion thereof, until the cause for such order has been eliminated; however, this right of City to stop the Work will not give rise to any duty on the part of City to exercise this right for the benefit of Contractor or any other party.

4.9.3 Correction or Removal of Defective Work.

4.9.3.1 If required by City, Contractor must promptly, without cost to City and as specified by City, either correct any defective Work, whether or not fabricated, installed or completed, or, if the Work has been rejected by City, remove it from the Site and replace it with non-defective Work. Contractor must correct any Work which may be displaced in correcting, removing or replacing defective Work. No compensation will be allowed Contractor for such removal, replacement or remedial Work. Contractor must reimburse City for costs incurred by City due to such correction or removal including but not limited to additional expenses for inspection, testing or observation and for repeated reviews by the City or Design Professional.

4.9.3.2 Upon failure on the part of the Contractor to comply within a reasonably prompt time with any written order of City to correct or remove defective Work, City has authority to cause nonconforming materials or rejected Work to be remedied, removed, or replaced at the Contractor's expense and to deduct the costs from any moneys due or to become due the Contractor.

4.9.4 City May Correct Defective Work. If Contractor fails within a reasonable time after written notice of City to proceed to correct defective Work or to remove and replace rejected Work as required by City or if Contractor fails to perform the Work in accordance with the Agreement Documents (including any requirements of the progress schedule), City may, after 7 Days' written notice to Contractor, correct and remedy any such deficiency. To the extent necessary to complete corrective and remedial action, City may exclude Contractor from all or part of the Work, and suspend Contractor's services related thereto, take possession of Contractor's tools, appliances, construction equipment and machinery at the Site and incorporate in the Work all materials and equipment stored at the Site or for which City has paid Contractor, but which are stored elsewhere. Contractor must allow City, city representatives, agents and employees such access to the Site as may be necessary to enable City to exercise City's rights under this Section. All direct and indirect costs of City in exercising such rights will be charged against Contractor in an amount verified by City representative, and a Change Order will be issued incorporating the necessary revisions in the Agreement Documents and a reduction in the Agreement Price. Such direct and indirect costs will include, in particular but without limitation, compensation for additional professional services required and all costs of repair and replacement of Work or others destroyed or damaged by correction, removal or replacement of Contractor's defective Work. Contractor will not be allowed an extension of the Agreement Time because of any delay in Contractor's performance of the Work attributable to the exercise by City or City's rights hereunder.

4.9.5 Correction or Removal of Unauthorized Work.

4.9.5.1 Any Work done beyond the lines and grades shown on the Drawings or established by the Design Professional or any changes in, additions to, or deductions from the Work done without written authority will be considered as unauthorized and will not be paid for. Work so done may be ordered remedied, removed, or replaced at the Contractor's expense.

4.9.5.2 Upon failure on the part of the Contractor to comply promptly with any order of the City,

City will have authority to cause unauthorized Work to be remedied, removed, or replaced at the Contractor's expense and to deduct the costs from any moneys due or to become due the Contractor.

4.9.6 Correction Period - One Year Guarantee.

4.9.6.1 If, within one year after the date of Final Acceptance, or such longer period of time as may be prescribed by law or by the terms of any applicable special guarantee required by the Agreement Documents, or by any specific provision of the Agreement Documents, any Work is found to be defective, Contractor must promptly, without cost to City and in accordance with City's written instructions, either correct such defective Work, or, if it has been rejected by City, remove it from the Site and replace it with non-defective Work. If Contractor does not promptly comply with the terms of such instructions, or in an emergency where delay would cause serious risk of loss or damage, City may have the defective Work corrected or the rejected Work removed and replaced, and all direct and indirect costs of such removal and replacement, including compensation for additional professional services, must be paid by Contractor. Such action by the City will not relieve the Contractor of the guarantees required by this Section or elsewhere in the Agreement Documents.

4.9.6.2 If, in the opinion of the City, defective Work creates a dangerous condition or requires immediate correction or attention to prevent further loss to the City or to prevent interruption of operation of the City, the City will attempt to give the notice required by this Section. If the Contractor cannot be contacted or does not comply with the City's request for correction within a reasonable time as determined by the City, the City may, notwithstanding the provisions of this Section, proceed to make such correction or provide such attention; and the costs of such correction or attention will be charged against the Contractor. Such action by the City will not relieve the Contractor of the guarantees required by this Section or elsewhere in the Agreement Documents.

4.9.6.3 This Section does not in any way limit the guarantee on any items for which a longer guarantee is specified or on any items for which a manufacturer or supplier gives a guarantee for a longer period. The Contractor agrees to act as co-guarantor with such manufacturer or supplier and must furnish the City all appropriate guarantee or warranty certificates upon completion of the Project. No guarantee period, whether provided for in this Section or elsewhere, will in any way limit the liability of Contractor or their sureties or insurers under the indemnity or insurance provisions of these General Conditions and the Project Specific Special Conditions.

4.9.7 Acceptance of Defective Work.

4.9.7.1 If, instead of requiring correction or removal and replacement of defective Work, City may accept Work when in the best interest of the City to do so with appropriate monetary credit from Contractor. If any such acceptance occurs prior to final payment, a Change Order will be issued incorporating the necessary revisions in the Agreement Documents, including appropriate reduction in the Agreement Price; or, if the acceptance occurs after final payment, an appropriate amount must be paid by Contractor to City.

4.9.7.2 Alternatively, City may require Contractor to furnish at Contractor's expense, a special performance guarantee or other surety prior to acceptance of defective work.

4.9.8 The Warranty period begins on the Final Acceptance date noted in the Certificate of Completion, irrespective of early completion by some Subcontractors of their work.

- 4.9.9 Contractor's warranty obligation must be in accordance with MAG Specifications.
- 4.9.10 Nothing in the warranties contained in the Agreement Documents are intended to limit any manufacturer's warranty which provides City with greater warranty rights than set forth in this Section or the Agreement Documents. Contractor must provide City with all manufacturers' warranties prior to Substantial Completion, if applicable, or Final Acceptance.
- 4.9.11 Contractor agrees that it will be responsible to manage and administer the correction of any Work that is not in conformance with the Agreement Documents during the warranty periods set forth in this Section, or during any longer periods to the extent required by the Agreement Documents. A progress payment, or partial or entire use or occupancy of the Project by City, will not constitute acceptance of Work not in accordance with the Agreement Documents.
- 4.9.12 When notified of a warranty issue, Contractor must respond in writing within 48-hours and must perform warranty Work as soon as material for said repairs are available (as judged solely by City), and in any event Contractor must, take immediate steps to commence and complete correction of nonconforming Work no later than the time period set forth in City's written notification in accordance with the Agreement Documents. This includes the correction, removal or replacement of the nonconforming Work and any damage caused to other parts of the Work affected by the nonconforming Work. If defects develop which are determined by City to be an emergency, City will notify Contractor, via the most expeditious means regarding the nature and condition of the defects. In turn, Contractor must immediately dispatch necessary forces to correct the defect or the emergency condition in accordance with Agreement Documents.
- 4.9.13 The time periods referenced in this Section apply only to Contractor's obligation to correct nonconforming Work and is not intended to constitute a period of limitations for any other rights or remedies that City may have regarding Contractor's other obligations under the Agreement Documents.
- 4.9.14 Without limiting the foregoing or anything in these General Conditions or the Agreement to the contrary, Contractor must obtain and provide to City all warranties for any portion of the Project offered by the manufacturer, installer or provider thereof. City and the user of the facility will have the right to the full value and benefit of all such warranties. Contractor must ensure all such warranties are fully transferrable to facilitate the full value of this Section.
- 4.9.15 Contractor's warranty excludes damages or defects caused by abuse, alterations to the Work not executed by or through Contractor, improper or insufficient maintenance, improper operation, or normal wear and tear and normal usage.
- 4.9.16 In the event of any noncompliance with this entire Section 4, City may require Contractor to stop or suspend the Work in whole or in part.

SECTION 5 - CITY RESPONSIBILITIES

5.1 CITY PROJECT MANAGER AND INSPECTORS

- 5.1.1 Project Manager is responsible for providing City-supplied information and approvals in a timely manner to assist Contractor to fulfill its obligations under the Agreement Documents.

- 5.1.2 Project Manager will also provide Contractor with prompt notice when it observes any failure on the part of Contractor to fulfill its contractual obligations, including any default or defect in the Project or non-conformance with the Drawings and Specifications.
- 5.1.3 City may utilize Field Inspectors to assist Project Manager during construction in observing performance of Contractor. City's use of Inspectors is for the purpose of assisting Project Manager.
- 5.1.4 The Inspectors are authorized to inspect all Work and materials furnished. Such inspections may extend to all or part of the Work and to preparation, fabrication or manufacture of the materials to be used. The Inspectors have the authority to issue instructions contrary to the Construction Documents if approved and coordinated with the directions of Project Manager.
- 5.1.5 The Inspectors have the authority to reject work or materials until any questions at issue can be decided by Project Manager.
- 5.1.6 The use of Inspectors by City will not make City responsible for or give City control over construction means, methods, techniques, sequences or procedures or for safety precautions or programs or responsibility for Contractor's failure to perform the Work in accordance with Agreement Documents. The Inspectors are not authorized to direct any of Contractor's activities, employees or Subcontractors.

5.2 **DESIGN PROFESSIONAL SERVICES**

City may contract separately with one or more Design Professionals to provide construction administration of the Project. The Design Professional's Agreement, as well as other firms hired by City may be furnished to Contractor. Contractor does not have the right to limit or restrict or reject any Agreement modifications that are mutually acceptable to City and Design Professional.

5.3 **CITY'S SEPARATE CONTRACTORS**

City is responsible for all work performed on the Project or at the Site by separate contractors retained by City. City will contractually require its separate contractors to reasonably cooperate with, and reasonably coordinate their activities so as not to interfere with Contractor in order to enable Contractor to timely complete the Work consistent with the Agreement Documents. Contractor must immediately notify the Project manager, and address the matter in the next monthly status report, if any activities of such separate contractors are expected to interfere, or are interfering, with Contractor and such interference will or could result in any delay in Contractor's performance of the Work.

5.4 **PERMIT REVIEW AND INSPECTIONS**

- 5.4.1 If requested by Contractor, Project Manager will provide assistance and guidance in obtaining necessary reviews, permits and inspections.
- 5.4.2 The regulating agencies of City, such as Development and Sustainability, Fire and Planning Departments, enforce legal requirements. The enforcement activities of City are independent and separate from this Agreement.

5.5 **PLANS AND SPECIFICATIONS TO THE CONTRACTOR**

Contractor will be provided up to five copies of the Agreement Documents as are reasonably necessary for the execution of the Work. Additional copies will be furnished,

upon request, at the cost of reproduction.

SECTION 6 - AGREEMENT TIME

6.1 AGREEMENT TIME

- 6.1.1 The Agreement Time will start with the Notice to Proceed ("NTP") and end with Final Acceptance.
- 6.1.2 Beginning on the date of the NTP, Contractor must begin to fulfill Contractor's obligations under the Agreement. Contractor's obligations include providing City and other agencies with any submittals required by the Project Specific Special Provisions, including but not limited to, an approved Project Schedule, Traffic Control Plans, and a Stormwater Pollution Prevention Plan. Contractor must submit all such required submittals before any physical construction work commences on the Site. NTP does not authorize construction work until all agreement insurance, bonds, and schedules are submitted to and accepted by the City.
- 6.1.3 The Agreement Time will be as set forth in the Project Schedule. Contractor agrees that it will commence performance of the Work and complete the Project through both Substantial Completion and Final Acceptance within the Agreement Time.
- 6.1.4 Time is of the essence of this Agreement, for the Project, for the Work, and for each phase and designated Milestone thereof.
- 6.1.5 Failure of Contractor to perform any covenant or condition contained in the Agreement Documents within the time periods specified herein, will constitute a material breach of this Agreement entitling City to terminate the Agreement unless Contractor applies for and receives an extension of time, in accordance with the procedures set forth in the Agreement Documents.
- 6.1.6 Failure of City to insist upon the performance of any covenant or condition within the time periods specified herein, will not constitute a waiver of Contractor's duty to perform every other covenant or condition within the designated periods, unless a specific waiver is granted in writing for each such covenant or condition.
- 6.1.7 City's agreement to waive a specific time provision or to extend the time for performance will not constitute a waiver of any other time provisions contained in the Agreement Documents. Failure of Contractor to complete performance promptly within the additional time authorized in the waiver or extension of time agreement constitutes a material breach of this Agreement entitling City to all the remedies set forth herein or provided by law.

6.2 PROJECT SCHEDULE

- 6.2.1 The Project Schedule must be in Microsoft Project standard file format, must be updated and maintained throughout the Agreement Time, and must contain the following:
 - 6.2.1.1 Detailed representation of all activities for the project, both on-site construction and major procurement. All significant activities together with the resource loading requirements for each and all items appearing on the schedule of values or bid schedule for progress payments must be shown on the Project Schedule or in attached transmittal letter as described in Section 6.2.8.

- 6.2.1.2 Dependencies between activities must be indicated so that it may establish as to the effect the progress of any one activity would have on other activities and on the Schedule.
- 6.2.1.3 Activities for submission, review, and approval of all required submittals.
- 6.2.1.4 An amount of time will be established prior to the final completion date for “punch list and cleanup”. No other activities will be scheduled during this period. Punch list and cleanup must be shown on the Project Schedule and must be entirely completed prior to the expiration of the Agreement Time.
- 6.2.2 Within 10 Days of receipt of City’s comments, Contractor must make all required corrections, adjustments, and additions to complete the Project Schedule and resubmit to City for review. City’s review of and response to the Project Schedule is for the purpose of: (1) City planning and staffing for the Project as may be required from time to time; (2) ensuring Contractor’s general conformance with the scheduling requirements of the Agreement Documents and completion of the Project within the Agreement Time; and (3) monitoring and evaluating the construction status for purposes of approving monthly progress payments. Acceptance of a submitted schedule by City should in no way be construed as an affirmation or admission that the schedule is reasonable or workable by Contractor. The responsibility for completing the Work on the Project within the Agreement Time remains the obligation of Contractor. City’s review does not relieve Contractor from compliance with the requirements of the Agreement Documents or be construed as relieving Contractor of its complete and exclusive control over the means, methods, sequences and techniques for executing the work.
- 6.2.3 The Project Schedule must show milestones, including milestones for City-furnished information, and must include activities for City-furnished material and construction by other contractors when those activities are interrelated with Contractor activities.
- 6.2.4 The Project Schedule must be revised as required by conditions and progress of the Work, but such revisions do not relieve Contractor of its obligations to complete the Work within the Agreement Time, as adjusted in accordance with the Agreement Documents. No modification to the Agreement Documents or the Agreement Time will be effective unless approved in advance by City.
- 6.2.5 For all items of materials and equipment that are critical or may require long lead times to acquire, the Project Schedule must show dates for submission, review and approval of submittals, ordering, and delivery.
- 6.2.6 An updated Project Schedule must be submitted monthly to City as part of the Payment Request. The monthly submittal must include one full size plot of the entire schedule and one electronic copy containing the schedule in Microsoft Project standard file format. In addition, Contractor must, upon request by City, provide a copy of all submitted schedule data in electronic format which must be clearly labeled with the Project description, scheduling program name and version number, and schedule print/data date.
- 6.2.7 Contractor must provide City with a monthly status report with each Project Schedule detailing the progress of the Work, including: (i) if the Work is proceeding according to schedule, (ii) any discrepancies, conflicts, or ambiguities found to exist in the Agreement Documents that require resolution, and (iii) other information detailing items that require resolution so as not to jeopardize the ability to complete the Work in the Agreement Time.

- 6.2.8 With each Project Schedule submittal, Contractor must include a transmittal letter including the following:
- a. Description of problem tasks, referenced to field instructions or requests for information (RFI's), as appropriate.
 - b. Current and anticipated delays including:
 - (i) Cause of the delay.
 - (ii) Corrective action and schedule adjustments to correct the delay.
 - (iii) Known or potential impacts and their delay on other activities, milestones, and their impact on the Substantial Completion and Final Acceptance dates.
 - (iv) Changes in construction sequence.
 - c. Pending items and status thereof including but not limited to:
 - (i) Time Extension requests;
 - (ii) Substantial Completion date status;
 - (iii) Final Acceptance date status.
 - d. If ahead of schedule, the number of calendar Days ahead.
 - e. If behind schedule, the number of calendar Days behind.
 - f. Other Project or scheduling concerns.
- 6.2.9 Critical Path Method (CPM).
- 6.2.9.1 Unless otherwise specified in the Agreement, the Project Schedule must include a Critical Path Method (CPM) diagram schedule showing the sequence of activities, the interdependence of each activity and identifies the Critical Path.
- 6.2.9.2 The CPM diagram schedule must be in calendar Days and indicate duration, earliest and latest start and finish dates for all activities, and total Float Times for all activities except critical activities. The CPM diagram must be presented in a time scaled graphical format for the Project as a whole.
- 6.2.9.3 The CPM diagram schedule must indicate all relationships between activities.
- 6.2.9.4 The activities making the Project Schedule must contain sufficient detail to assure that adequate planning has been done for proper execution of the Work and such that it provides an appropriate basis for monitoring and evaluation the progress of the Work. Individual activities must not exceed 30 Days in length, in most cases.
- 6.2.9.5 The CPM diagram schedule must be based upon activities, which coincide with the Schedule of Values.
- 6.2.9.6 The CPM diagram schedule must show all submittals associated with each work activity and the review time for each submittal.
- 6.2.10 Float Time.
- 6.2.10.1 The total Float Time within the overall schedule is for the exclusive use of City, but City may approve Contractor's use of Float as needed to meet Agreement Milestones and

the Project completion date.

- 6.2.10.2 Contractor will not be allowed to sequence, hide, or reallocate Float Time through such strategies, as extending activity duration estimates to consume available Float, using preferential logic, or using extensive crew/resource sequencing, etc. No time extensions will be granted nor delay damages paid until a delay occurs which extends the Work beyond the Agreement Time.
- 6.2.11 City-Caused Delays. City-caused delays on the Project, if any, may be offset by City-caused time savings (i.e., Critical Path submittals returned in less time than allowed by the Agreement, approval of substitution requests and credit changes which result in savings of time to Contractor, etc.) In such an event, Contractor will not be entitled to receive a time extension or delay damages until all City-caused time savings are exceeded and the Agreement Time is also exceeded.
- 6.2.12 Rain-Related Delays. Contractor is required, in preparing the Project Schedule to take into account all relevant weather conditions, including normal rainfall and distribution. No additional compensation will be given for any rain-related delays or impacts on the Work or the Project Schedule. No time extension will be granted in the Project Schedule unless the rainfall during the construction of Work is unusually severe, was not reasonably anticipated, and the total rainfall was significantly in excess of the normal rainfall for the Project Site location. Normal rainfall for the Project will be determined from the 10-year average rainfall for the Site as measured by the National Oceanic and Atmospheric Administration or comparable source of reliable information for rainfall in Chandler, Arizona. In addition, the excessive rainfall must have actually impacted Work activities on the Critical Path and caused delay beyond any remaining Float at the time of the rain-caused delay. The burden of documenting normal rainfall, the excessive rainfall and the impact on Critical Path activities is on Contractor. All other provisions in the Agreement Documents relating to claims, including without limitation notice requirements, apply to any claim by Contractor for a rain delay.
- 6.2.13 City's "Policy Statement for Calculating Delays and Damages," Appendix 1 to these General Conditions, will apply to all claims of delay and delay damages.
- 6.2.14 Force Majeure. If Contractor is delayed or prevented from the performance of any Work required under this Agreement by reason of acts of God or other causes beyond the control and without fault of Contractor (financial inability excepted), performance of that Work will be excused, but only for the period of the delay. The time for performance of the Work will be extended for a period equivalent to the period of delay. In addition, the parties agree if Contractor's delayed or suspended performance directly arises out of or directly results from the COVID-19 pandemic, Contractor's delayed or suspended performance may be excused as set forth in this clause. Provided, however; Contractor must give the City written notice within 30 days of the occurrence of the event giving rise to COVID-19 pandemic related delayed or suspended performance. For COVID-19 pandemic related delay or suspended performance, the parties must agree in writing to the length of the excused delay or suspended performance. Further, Contractor must obtain the City's written approval to use any allowance established as part of the project for delays and costs related to the COVID-19 pandemic.

6.3 **SUBSTANTIAL COMPLETION**

- 6.3.1 When Contractor considers that the Work, phase or a portion thereof, which City agrees in

writing to accept separately, is substantially complete, City will prepare and submit to Contractor a comprehensive Punch List of items to be completed or corrected prior to Final Acceptance and Final Payment. Failure to include an item on such Punch List does not alter the responsibility of Contractor to complete all Work in accordance with the Agreement Documents.

6.3.2 Upon receipt of Contractor's Punch List, Project Manager will make an inspection to determine whether the Work or designated portion thereof is substantially complete. Project Manager may, at Project Manager's sole option, be assisted in such inspection by the Design Professional for the Project. If the inspection by the Project Manager discloses any item, whether or not included on Contractor's Punch List, which is not sufficiently completed in accordance with the Agreement Documents so that City can occupy or utilize the Work, phase or designated portion thereof for its intended use, Contractor must, before issuance of the Certificate of Substantial Completion, complete or correct such item upon notification by Project Manager. In such case, Contractor must then submit a request for another inspection by Project Manager to determine Substantial Completion.

6.3.3 Certificate of Substantial Completion.

6.3.3.1 The Project Manager will not issue a Certificate of Substantial Completion unless and until the Work (or separable units or Phases as provided in the Agreement Documents) is essentially and satisfactorily complete in accordance with the Agreement Documents, such that the Project is ready for use by City for its intended purpose, opening to the general public, full occupancy or use by City (including, without limitation, all separate units, or rooms, facilities, access, income-generating areas, and all areas serving the general public, as applicable, must be ready for full-operation without material inconvenience or discomfort), including, to the extent applicable to the Work, the following: all materials, equipment, systems, controls, features, facilities, accessories and similar elements are installed in the proper manner and in operating condition, inspected and approved; surfaces have been painted; masonry and concrete cleaned with any sealer or other finish applied; utilities and systems connected and functioning; site work complete; permanent heating, ventilation, air condition, vertical transportation and other systems properly operating with proper controls; lighting and electrical systems installed, operable and controlled; paving completed, signage installed, and other Work as applicable, has been performed to a similar state of essential and satisfactory completion. A minor amount of Work, as determined by and at the discretion of the Project Manager, such as installation of minor accessories or items, a minor amount of painting, minor replacement of defective work, minor adjustment of controls or sound systems, or completion or correction of minor exterior work that cannot be completed as a result of weather conditions, will not delay determination of Substantial Completion. If prior written approval is obtained from City for purposes of Substantial Completion, specified areas of the entire Work or Project may be individually certified as Substantially Complete. In no event will Substantial Completion be deemed to have occurred unless and until: (i) a temporary certificate of occupancy has been issued by the appropriate Governmental Authorities (as applicable) and (ii) all terms and Work required under this Agreement have been fulfilled by Contractor and same will have also been approved and accepted by City, subject only to the Punch List items.

6.3.3.2 If requested by City, Contractor must complete and turn-over to City the Project on a phased basis. Each phase will have a separate inspection by the Project Manager, a Punch List generated, and then an inspection by City with final approval and acceptance only after the

Project Manager's Punch List.

6.4 **PARTIAL UTILIZATION**

6.4.1 City at City's option may use and occupy any substantially completed parts of the Work which has specifically been identified in the Agreement Documents, or which City, the Design Professional and Contractor agree constitutes a separately functioning and usable part of the Work that can be used by City for its intended purpose, without significant interference with Contractor's performance of the remainder of the Work, provided, however, if the portion of the Work to be used or occupied has not been found to be substantially complete, City must do so in accordance with Section 6.3 prior to such occupancy.

6.4.2 In lieu of the issuance of a Certificate of Substantial Completion as to part of the Work, City may take over operation of a facility constituting part of the Work whether or not it is substantially complete if such facility is functionally and separately usable; provided that prior to any such takeover, City and Contractor agree in writing as to the division of responsibilities between City and Contractor for security, operation, safety, maintenance, correction period, heat, utilities and insurance with respect to such facility.

6.4.3 Substantial Completion of or City's beneficial occupancy of a part of the Project will not alter the fact that the one-year warranty for the whole Project starts at the date of Final Completion of the whole Project.

6.5 **FINAL ACCEPTANCE**

6.5.1 Unless otherwise expressly agreed to in writing by City, Final Acceptance must be obtained by no later than 30 Days (60 Days for federally funded agreements) after the date of Substantial Completion. Failure to timely obtain Final Acceptance will be a material breach of the Agreement.

6.5.2 Upon receipt of written notice that the Work is ready for final inspection and acceptance, City and Contractor will jointly inspect to verify that the remaining items of Work have been completed. There will be no partial acceptance. Final Acceptance will not occur until all items of Work, including Punch List Items, have been completed to City's satisfaction as reflected in the written Final Acceptance.

6.5.3 Final Payment will not be due, owing, or paid by City until Final Acceptance is issued.

6.5.4 Landscape Establishment Period. Unless otherwise expressly agreed to in writing by City, the Landscape Establishment Period will begin on the date of Final Acceptance of the Project and will run 90 Calendar Days thereafter. Landscape Establishment Period requirements are detailed in General Conditions Appendices, attached herein.

6.6 **CONTINUATION OF WORK**

6.6.1 Permitting Contractor to continue and finish the Work or any part of it after the time fixed for its completion (whether milestone, phase, Substantial Completion or Final Acceptance) or after the date to which the time fixed for any completion may have been extended, does not operate as a waiver by City of any rights under the Agreement Documents, law or equity.

6.6.2 Furthermore, the timely completion of the Work being of the utmost importance under this Agreement, notwithstanding the existence of one or more disputes between the parties

concerning the scope of the Work, the Project Schedule, Agreement Time, payments or any other matter, and further notwithstanding a party's invocation of the Dispute Resolution provisions specified in Appendix 6 of these General Conditions, unless City suspends the Agreement or Contractor's performance pursuant to Section 10 of these General Conditions, Contractor will continue to prosecute the Work, including any Change Order work or Extra Work Orders, in a diligent and timely manner and not stop, slow down or impede by action or inaction the progress of the Work, including commencing performance of and thereafter completing any additional work called out in any Change Order or Extra Work Order issued by Project Manager with the approval of City, so long as City makes payment to Contractor in accordance with Section 8 of these General Conditions.

SECTION 7 - AGREEMENT PRICE

7.1 UNIT PRICE AGREEMENTS

- 7.1.1 The Agreement Price for all Unit Price Agreements will be the amount set forth in the Agreement or Change Order multiplied by the verified quantity provided.
- 7.1.2 Measurements of quantities to determine the total Agreement Price must be in accordance with MAG Specification §§ 109.1 and 109.2.
- 7.1.3 The Unit Price may only be changed as set forth in Section 9 below.

7.2 CHANGE ORDERS

- 7.2.1 Unit Price Change Orders. The Change Order Price for all Unit Price Change Orders will be the amount set forth in the Change Order multiplied by the verified quantity provided.
- 7.2.2 Measurements of quantities to determine the total Change Order Price must be in accordance with MAG Specifications §§ 109.1 and 109.2.
- 7.2.3 The Unit Price may only be changed as set forth in Section 9 below.
- 7.2.4 MAG Specification § 109.4.1 is modified as follows: Before § 109.4.1, the following is added: Any deduction or increase in the Agreement Price must be supported by a signed, written Change Order fully executed by City, and supported by such backup as the Project Manager may require.

7.3 SALES TAX

Contractor is required to pay all applicable sales tax in accordance with the law of the state of Arizona and this cost must be included in all Agreement Prices. When equipment, materials or supplies generally taxable to Contractor are eligible for a tax exemption due to the nature of the Project, Contractor must assist City in applying for and obtaining such tax credits and exemptions which will be paid or credited to City.

SECTION 8 - PAYMENT

8.1 PAYMENT FOR CONSTRUCTION SERVICES

- 8.1.1 Payment for the Work will be made in accordance with MAG Standard Specification § 109 as amended below.
- 8.1.2 Contractor must submit to City for review a completed Contractor Payment Request signed

by Contractor, covering the Work completed as of the date of the Request and accompanied by such supporting documentation as is required by the Agreement Documents and also as City may reasonably require. A Contractor Payment Request will not be considered complete unless it is accompanied by an updated Project Schedule and a certification that the on-site, red lined, as built Drawings are up to date. If payment is requested on the basis of materials and equipment not incorporated in the Work but delivered and suitably, securely stored at the Site or at another location (such as a bonded warehouse) agreed to in writing, the Contractor Payment Request must also be accompanied by such data, satisfactory to City, as will establish City's title to the material and equipment and protect City's interest therein, including applicable insurance. Each subsequent Contractor Payment Request must include an affidavit of Contractor stating that all previous progress payments received on account of the Work have been applied to discharge in full all of Contractor's obligations reflected in prior Contractor Payment Requests.

- 8.1.3 Except for Work performed under a JOC Agreement, the Contracting Agency will retain 10 percent of all estimates as a guarantee for complete performance of the Agreement in accordance with Arizona Revised Statutes Section 34-221 or 34-607. The Contractor may elect to deposit securities in lieu of retention in accordance with Arizona Revised Statutes Section 34-221, Paragraph C.5. or 34-607, Paragraph B.5.
- 8.1.4 The payment process functions as follows: Prior to the payment cycle date, Contractor must send a draft Contractor Payment Request to Project Manager. The Project Team will review the Request and agree upon any necessary adjustments. Contractor must certify the final Request by signing and returning to Project Manager.
- 8.1.5 When construction of the Project is fifty percent (50%) completed, Contractor may request payment of one-half of the retention pursuant to A.R.S. § 34-609(B)(3), subject to all of City's rights to withhold or offset payments, and other rights of City, under the Agreement.
- 8.1.6 City reserves the right under A.R.S. § 34-609(B)(3) to reinstate the ten percent (10%) retention if City determines that satisfactory progress is not being made.
- 8.1.7 Contractor's Warranty of Title.
 - 8.1.7.1 Contractor warrants and guarantees that title to all Work, materials and equipment covered by any Contractor Payment Request, whether incorporated in the Project or not, will pass to City at the time of payment, free and clear of all liens, claims, security interests, and encumbrances, provided that this will not preclude the Contractor from installing metering devices or other equipment of utility companies or municipalities, the title of which is commonly retained by the utility company or municipality.
 - 8.1.7.2 No materials, supplies, or equipment for the Work under this Agreement will be purchased subject to any chattel mortgage or under a conditional sale contract or other agreement by which an interest therein, or any part thereof, is retained by the seller or supplier.
 - 8.1.7.3 Nothing contained in this Section will defeat or impair the right of such persons furnishing materials or labor under any bond given by the Contractor for their protection, or any right under any law permitting such persons to look to funds due the Contractor in the hands of the City. The provisions of this Section must be inserted in all subcontracts and material agreements, and notices of its provision must be given to all persons furnishing materials for the Work when no formal agreement is entered into for such materials.

8.2 PAYMENT UPON SUBSTANTIAL COMPLETION

8.2.1 No payment will be made upon Substantial Completion, except for a regularly-scheduled monthly progress payment, as allowed by Section 8.2.2.

8.2.2 No further payments will be made to Contractor until Final Acceptance.

8.3 FINAL PAYMENT

8.3.1 Subject to all of City's rights to withhold or offset payment, and other rights under the Agreement, Final Payment including remaining retainage will be paid only after:

- a. Work has been fully completed (including completion of all incorrect or incomplete work items) and the written Final Acceptance has been issued by City;
- b. Necessary operating manuals, any excess materials and supplies necessary for matching materials and supplies incorporated into the Work, acceptable sewer video results (if applicable), and complete "as-built" Drawings (including the Building Information Model, if required by the Agreement Documents) have been delivered to City, as specified in this Section 8.3;
- c. Full and unconditional lien waivers and releases by Contractor and any person performing labor or supplying material, machinery, fixtures, or tools for the Work have been delivered to Contractor;
- d. All conditions and requirements imposed by City or any financing entity for the corresponding disbursement have been met; and
- e. Contractor delivers to City a Contractor Payment Request requesting Final Payment.

8.3.2 Contractor must also submit a signed copy of Contractor's Affidavit Regarding Settlement of Claims, Appendix 3 to these General Conditions, and Certificate of Completion, Appendix 7 to these General Conditions, prior to Final Payment.

8.3.3 In addition, if required under the Project Specific Special Provisions, Contractor must compile a complete equipment list and maintenance manual to be submitted to City as a precondition to Final Payment. The list must include the following items for all equipment supplied under the Plumbing, Electrical, Air Conditioning, Elevator, and other Special Equipment Specifications:

- a. Name, Model and Manufacturer.
- b. Complete parts lists and Drawings.
- c. Local source of supply for replacement parts along with suppliers' telephone numbers.
- d. Local service organizations serving the equipment and their telephone numbers.
- e. All tags, inspection slips, instruction packages, etc., removed from equipment must be properly identified as to pieces of equipment from which they were taken.

8.3.4 Contractor must also deliver to City, prior to Final Payment, one (1) digital (in the format specified by City), and if requested by City, one (1) hard copy, of any applicable Maintenance manuals. Each manual must include all manufacturer's operation and maintenance instructions and "as-built" Drawings with the list herein specified. It must also include all other diagrams and instructions necessary to properly operate and maintain the equipment, the name, address and telephone number of Contractor and all Subcontractors involved.

8.4 **CITY'S RIGHT TO WITHHOLD PAYMENT**

City may withhold payment to such extent as may be necessary in City's opinion to protect City from loss for which Contractor is responsible, including, without limitation, if any of the following conditions exist:

- a. Defective Work not remedied;
- b. Third party claims filed or reasonable evidence indicating probable filing of such claims unless security acceptable to City is provided by Contractor;
- c. Failure of Contractor to make payments properly to Subcontractors or for labor, materials or equipment;
- d. Reasonable evidence that the Work cannot be completed for the unpaid balance of the Agreement Price;
- e. Damage to City or another Contractor;
- f. Reasonable evidence that the Work will not be completed within the Agreement Time, and that the unpaid balance would not be adequate to cover actual or liquidated damages for the anticipated delay;
- g. Failure to carry out the Work in accordance with the Agreement Documents;
or
- h. Contractor is in default of any of its other obligations under the Agreement Documents.

8.5 **JOINT/DIRECT CHECKS**

To promote the timely completion and progress of the Work or when appropriate and necessary, payments to Contractor may be made jointly to Contractor and its employees, agents, Subcontractors and suppliers, or any of them. For federally funded agreements, see federal provisions for additional requirements for the joint check process.

8.6 **PAYMENT NOT A WAIVER**

No payment (nor use or occupancy of the Project by City) will be deemed acceptance or approval of the Work or as a waiver of any claims, rights, or remedies of City.

8.7 **LIENS AND BOND CLAIMS**

Contractor must make all payments, in the time required, of all labor and materials furnished to Contractor in the course of the Work and must promptly furnish evidence of such payments as City may require. Contractor must pay when due all claims arising out of performance of the Work covered by this Agreement for which a lien may be filed either against the real estate or leasehold interest of City, or against payments due from City to Contractor, or for which a claim may be made against any payment or performance bond or both. To the fullest extent permitted by law, Contractor agrees that no liens or other claims in the nature of a lien against the real estate, leasehold, or other interest of City,

against payment due from City to Contractor, or against any payment or performance bond, must be filed or made in connection with the Work by any party who has supplied professional services, labor, materials, machinery, fixtures, tools, or equipment used in or in connection with the performance of this Agreement, and Contractor agrees to remove or to cause to be removed any such liens or claims in the nature of a lien or bond claim within 10 Days upon receiving notice or obtaining actual knowledge of the existence of such liens or claim. In addition, Contractor agrees to defend, indemnify, and hold harmless City from and against any and all such liens and claims. This paragraph does not apply to claims and liens of Contractor due to non-payment for work performed.

8.8 FINANCIAL RECORDKEEPING AND CITY'S AUDIT RIGHT

8.8.1 Records for all Agreements between City and Contractor must, upon reasonable notice, be open to inspection and subject to audit, scanning, and reproduction during normal business working hours. Such audits may be performed by any City's representative or any outside representative engaged by City for the purpose of examining such records. City or its designee may conduct such audits or inspections throughout the term of this Agreement and for a period of five years after Final Payment or longer if required by law. City's representatives may (without limitation) conduct verifications such as counting employees at the Site, witnessing the distribution of payroll, verifying information and amounts through interviews and written confirmations with Contractor employees, field and agency labor, Subcontractors, and vendors.

8.8.2 Contractor's "records" must include any and all information, materials and data of every kind and character, including without limitation, records, books, papers, documents, subscriptions, recordings, agreements, purchase orders, leases, contracts, commitments, arrangements, notes, daily diaries, superintendent reports, drawings, receipts, vouchers and memoranda, and any and all other agreements, sources of information and matters that may in City's judgment have any bearing on or pertain to any matters, rights, duties or obligations under or covered by any Agreement Document. Such records must include (hard copy, as well as computer readable data if it can be made available), written policies and procedures; time sheets; payroll registers; payroll records; cancelled payroll checks; subcontract files (including proposals of successful and unsuccessful bidders, bid recaps, negotiation notes, etc.); original bid estimates; estimating work sheets; correspondence; Change Order files (including documentation covering negotiated settlements); back-charge logs and supporting documentation; invoices and related payment documentation; general ledger, information detailing cash and trade discounts earned, insurance rebates and dividends; and any other Contractor records which may have a bearing on matters of interest to City or the Project in connection with Contractor's dealings with City or the Project (all foregoing hereinafter referred to as "records") to the extent necessary to adequately permit evaluation and verification of any or all of the following:

- a. Compliance with Agreement requirements for deliverables;
- b. Compliance with approved Plans and Specifications;
- c. Compliance with § 14.9 below;
- d. Compliance with Agreement provisions regarding the pricing of Change Orders;
- e. Accuracy of Contractor representations regarding the pricing of invoices; or
- f. Accuracy of Contractor representations related to claims submitted by

Contractor or any of their employees.

- 8.8.3 Contractor must require all payees (examples of payees include Subcontractors, Suppliers, Insurance Carriers, etc.) to comply with the provisions of this Section by including the requirements hereof in a written agreement between Contractor and payee. Contractor will ensure that all payees (including those entering into lump sum agreements) have the same right to audit provisions contained in this Agreement included in their agreements with Contractor.
- 8.8.4 City's authorized representative(s) (including, without limitation, Project Manager) must have reasonable access to Contractor's facilities, must be allowed to interview all current or former employees to discuss matters pertinent to the performance of this Agreement and must be provided adequate and appropriate work space, in order to conduct audits in compliance with this Section.
- 8.8.5 If an audit inspection or examination in accordance with this Section, discloses overpricing or overcharges to City (of any nature) by Contractor or Contractor's Subcontractors in excess of \$100,000 in addition to making adjustments for the overcharges, the reasonable actual cost of City's audit must be reimbursed to City by Contractor. Any adjustments or payments which must be made as a result of any such audit or inspection of Contractor's invoices or records must be made within a reasonable amount of time (not to exceed 90 Days) from presentation of City's findings to Contractor.
- 8.8.6 In addition to the normal paperwork documentation Contractor typically furnishes to City, Contractor agrees to furnish, upon written request from City, any of the documentation necessary for City to exercise its audit rights under this Section 8.8 in computer readable file formats (Word, Excel, or .pdf), as City may designate.
- 8.8.7 City, its authorized representative, and the appropriate agency, reserve the right to audit Contractor's records in compliance with local, state or federal policies, statutes or at City's discretion, within three (3) years of Final Acceptance of the Work.

SECTION 9 - CHANGES TO THE AGREEMENT

9.1 FIELD ORDERS

City may authorize minor changes in the Work not involving an adjustment in the Agreement Price or the Agreement Times, which are consistent with the overall intent of the Agreement Documents. These may be accomplished by a written Field Order on the standard form approved and executed by City. Such Field Orders must be binding and Contractor must perform the change promptly. If Contractor believes that a Field Order justifies an increase in the Agreement Price or Agreement Time, Contractor may make a claim therefor as provided in Section 7.2.

9.2 EXTRA WORK/CHANGES IN THE WORK

- 9.2.1 City reserves the right to make such changes in the Plans and Specifications for the Work, as it may deem appropriate and any such change as set forth in a written Change Order must be deemed a part of this Agreement as if originally incorporated herein.
- 9.2.2 In the event City and Contractor cannot agree on the terms of a Change Order, or when circumstances otherwise require, the Project Manager has the authority to direct the Contractor to perform extra work, if the work in question is an item not provided for in the

Agreement as awarded. The Project Manager will have the authority to determine, based upon factual evidence presented by the Contractor, whether the work in question is an item not provided for in the Agreement as awarded. If the Project Manager directs the Contractor to perform extra work, the Project Manager's instructions will include a price that the Contractor cannot exceed in charging the City for the extra work. Upon receipt of the Project Manager's directions to perform extra work, the Contractor must promptly proceed with the extra work and document the actual cost thereof. Contractor's right to payment for extra work will be determined under Subsection 9.2.4 below. The Contractor is responsible to manage the extra work to ensure that the price limits set by the Project Manager are not exceeded. Contractor must perform the extra work and submit documentation for the actual cost of the extra work to the City. A Change Order will be issued to cover this work.

9.2.3 Contractor will not be entitled to payment for extra work unless a written Change Order, in form and content prescribed by City, has been executed by City. On all requests for Change Orders, Contractor must specify the increased or decreased costs and whether it believes any extensions of time will be necessary to complete its Work as modified by the Change Order. If extra work is performed under Subsection 9.2.2 above, a corresponding Change Order will be prepared, approved and processed by City before payment can be made to Contractor.

9.2.4 In general, pricing for Change Orders will include the same mark-up percentages that were in effect when the Agreement was awarded. The cost or credit to the City resulting from a change in the Work is subject to Appendix 1 (Policy Statement for Calculating Delays and Damages) and will be determined, based on the type of pricing for the Agreement involved, as follows:

- a. By mutual acceptance of a lump sum properly itemized in a form acceptable to City;
- b. By unit prices stated in the Agreement Documents;
- c. When the City determines that a Unit Price Book Job Order associated with a Job Order Agreement requires a Change Order, by using the same Total Cost Data and CCI that are in effect when the Change Order is anticipated to be issued; or
- d. By actual cost and a percentage fee covering overhead and profit, as follows:
 - (i) Contractor will perform the extra work and be compensated for actual cost of labor, materials and equipment.
 - (ii) Contractor will have the right to add the fee percentage applicable to the Work under the Agreement, or if no such fee has been agreed to by the parties, not more than five percent (5%) to the Subcontractor's prices for authorized extra work performed solely by Subcontractors. Such percentage will include all of Contractor's charges for overhead, profit, administration and supervision.
 - (iii) Contractor or Subcontractor will have the right to add the fee percentage applicable to Work under the Agreement for self-performed extra work, or if no such fee has been agreed to by the parties, Contractor's or Subcontractor's maximum total allowable additions for overhead, profit, administration and supervision will

not exceed ten percent (10%) of actual verifiable labor, materials and equipment for such self- performed extra work.

- 9.2.5 Any agreement which modifies the terms of the Agreement (including Change Orders) will be approved in writing by the Project Manager. Once properly executed by both parties, these modifications to the Agreement will have the same effect as if they had been included in the original Agreement.

9.3 **ACCURACY OF CHANGE ORDER PRICING INFORMATION**

- 9.3.1 Subject to Sections 9.3.2 through 9.3.4, signature by the contracting parties constitutes full accord and satisfaction between City and Contractor for all costs, damages, and expenses of whatever kind of nature, including delay, impact or acceleration damages, which may be occasioned by a Change Order of other modification of the Agreement agreed to in writing.

- 9.3.2 Accurate Change Order Pricing Information: Contractor agrees that it is responsible for submitting accurate cost and pricing data to City to support its Fixed Price, Unit Price, or Cost Plus Change Order Proposals or other Agreement Price adjustments under the Agreement. Contractor further agrees to submit Change Order proposals with cost and pricing data which is accurate, complete, current, and in accordance with the terms of the Agreement with respect to pricing of change orders. Contractor agrees that any “buy-out savings” on Change Orders will accrue 100% to Owner. “Buy-out savings” are defined as any savings negotiated by the Contractor with a Subcontractor or a Material Supplier after receiving approval of a Change Order amount that was designated to be paid to a specific Subcontractor or Supplier for the Approved Change Order work.

- 9.3.3 Right to Verify Change Order Pricing Information: Contractor agrees that City, through its designated representative, will have the right to examine, copy, and scan the records of the Contractor, Subcontractor or Sub-Subcontractor’s records (during the Agreement period and up to three years after final payment is made on the Agreement) to verify the accuracy and appropriateness of the pricing data used to price all Change Order proposals or claims. Contractor agrees that if City determines the cost and pricing data submitted (whether approved or not) was inaccurate, incomplete, not current, or not in compliance with the terms of the Agreement regarding pricing of Change Orders, an appropriate Agreement Price adjustment will be made. Such post-approval Contact Price adjustments will apply to all levels of contractors and Subcontractors and to all types of Change Order proposals, specifically including Fixed Price, Unit Price, and Cost Plus Change Orders.

- 9.3.4 Requirements for Detailed Change Order Pricing Information: Contractor agrees to provide a detailed breakdown of allowable labor and labor burden cost (i.e., base wage rate of applicable classifications of workers, payroll taxes, and insurance and benefits costs). This information will be used to evaluate the potential cost of labor and labor burden related to Change Order work. It is intended that this information represent an accurate estimate of the Contractor’s actual labor and labor burden cost components. Information is not intended to establish fixed billing or Change Order pricing labor rates. However, at the time Change Orders are priced, the submitted cost data for labor rates may be used to price Change Order work. The accuracy of any such agreed upon labor rate cost components used to price Change Orders will be subject to later audit. Approved Change Order amounts may be adjusted later to correct the impact of inaccurate labor cost components if the agreed upon labor cost components are determined to be inaccurate.

9.4 **EMERGENCIES**

In any emergency affecting the safety of persons or property, Contractor will act, at its discretion, to prevent threatened damage, injury or loss. Any change in the Agreement Price or Agreement Time resulting from emergency work will be determined as provided in this Section.

9.5 **DIFFERING SITE CONDITIONS**

9.5.1 If Differing Site Conditions are encountered at the Project Site, then notice by the observing party must be given to the other party promptly before conditions are disturbed (to the extent practicable) and in no event later than 14 Days after first observance of the conditions. City will promptly investigate such conditions and, if City determines that Differing Site Conditions exist and they materially cause an increase in the cost of, or time required for, performance of any part of the Work, Contractor will be entitled to equitable adjustment in the Agreement Price or Construction Schedule (and other time requirements), or both. If it is determined by City that the conditions at the Project Site are not Differing Site Conditions and no change is justified, then City will so notify Contractor in writing, stating the reasons. Claims in opposition to such determination must be made within 14 Days after City has given notice of its decision. If City and Contractor cannot agree on an adjustment in the Agreement Price or Construction Schedule (and other time requirements), the adjustment may be submitted to dispute resolution as provided these General Conditions.

9.6 **CHANGES IN LAWS, REGULATIONS, OR LEGAL REQUIREMENTS OR TAXES**

In the event of a material change in applicable Laws, Regulations, or Legal Requirements, or taxes subsequent to the date of the Agreement by the parties, Contractor may be entitled to a Change Order, in City's discretion, to the extent Contractor can document to the satisfaction of City that such change significantly increases Contractor's actual cost of performance of the Work.

SECTION 10 -SUSPENSION AND TERMINATION

10.1 **SUSPENSION**

City may suspend the Agreement and Contractor's performance in accordance with MAG Specifications § 105.1 and 108.7.

10.2 **TERMINATION BY THE CITY FOR CAUSE**

10.2.1 MAG Specifications § 108.11 applies to the Agreement.

10.2.2 City may also terminate the Agreement if City determines, in its sole discretion that Contractor has:

- a. After prior written notice, refused or failed to supply enough properly skilled workers or proper materials;
- b. After prior written notice, failed to make payment to Subcontractors for materials or labor in accordance with the respective agreements between Contractor and the Subcontractors;
- c. After prior written notice, disregards laws, ordinances, or rules, regulations or orders of a public authority having jurisdiction;
- d. After prior written notice, repeatedly failed to comply with written directives from City;

- e. Is adjudged as bankrupt or insolvent;
- f. Made a general assignment for the benefit of creditors;
- g. Appointed a trustee or receiver for itself or any of its property;
- h. Filed a petition to take advantage of any debtor's act, or to reorganize under the bankruptcy or similar laws; or
- i. Otherwise breached a provision of the Agreement Documents or any other Agreement between City and Contractor.

10.2.3 When any of the above reasons exist, City may terminate the Agreement, without prejudice to any other rights or remedies of City, after giving Contractor and Contractors' surety, if any, 7 Days written notice of City's intent to terminate the Agreement and Contractor's failure to cure any such reasons. Upon such termination, City may: (1) take possession of the Site and of all materials thereon owned by Contractor; or (2) finish the Work by whatever reasonable method City may deem expedient. When City terminates the Agreement for one of the reasons state above, Contractor will not be entitled to receive further payment until the Work is finished. If the unpaid balance of the Agreement Price existing at the time of such termination exceeds the costs and expenses of finishing the Work and any other damages incurred by City, such excess will be paid to Contractor. If such costs, expenses and damages exceed such unpaid balance, Contractor must pay the difference to City. This obligation for payment will survive termination of the Agreement.

10.3 **TERMINATION BY CITY FOR CONVENIENCE**

City may also terminate the Agreement at any time for its convenience upon 7 Days written notice to Contractor specifying the termination date. In the event of termination which is not the fault, in whole or in part, of Contractor, City will pay to Contractor only such compensation, including reimbursable expenses, due for Work properly performed on the Project prior to the termination date. Upon any termination of the Agreement, no further payments will be due from City to Contractor.

10.4 **A.R.S. § 38-511**

The Agreement is subject to, and may be terminated by City in accordance with, the provisions of A.R.S. § 38-511.

SECTION 11 -INSURANCE AND BONDS

11.1 **INSURANCE REQUIREMENTS**

11.1.1 After Agreement award, the Contractor must furnish the City a certificate of insurance on a standard insurance industry ACORD form. The ACORD form must be issued by an insurance company authorized to transact business in the State of Arizona possessing a current A.M. Best, Inc. rating of A-7, or better and legally authorized to do business in the State of Arizona with policies and forms satisfactory to City. Provided, however, the A.M. Best rating requirement may not be deemed to apply to required Worker's Compensation coverage.

11.1.2 The Contractor and any of its Subcontractors must procure and maintain, until all of their obligations have been discharged, including any warranty periods under this Agreement are satisfied, the insurances set forth below.

11.1.3 The insurance requirements set forth below are minimum requirements for this Agreement and in no way limit the indemnity covenants contained in this Agreement.

- 11.1.4 The City in no way warrants that the minimum insurance limits contained in this Agreement are sufficient to protect Contractor from liabilities that might arise out of the performance of the Agreement services under this Agreement by Contractor, its agents, representatives, employees, or Subcontractors and the Contractor is free to purchase any additional insurance as may be determined necessary.
- 11.1.5 Failure to demand evidence of full compliance with the insurance requirements in this Agreement or failure to identify any insurance deficiency will not relieve the Contractor from, nor will it be considered a waiver of its obligation to maintain the required insurance at all times during the performance of this Agreement.
- 11.1.6 Use of Subcontractors: If any Work is subcontracted in any way, the Contractor must execute a written agreement with Subcontractor containing the same Indemnification Clause and Insurance Requirements as the City requires of the Contractor in this Agreement. The Contractor is responsible for executing the Agreement with the Subcontractor and obtaining Certificates of Insurance and verifying the insurance requirements.
- 11.2 **MINIMUM SCOPE AND LIMITS OF INSURANCE**
- 11.2.1 The Contractor must provide coverage with limits of liability not less than those stated below.
- 11.2.1.1 Commercial General Liability-Occurrence Form. Contractor must maintain "occurrence" form Commercial General Liability insurance with a limit of not less than \$2,000,000 for each occurrence, \$4,000,000 aggregate. Said insurance must also include coverage for products and completed operations, independent contractors, personal injury and advertising injury. If any Excess insurance is utilized to fulfill the requirements of this paragraph, the Excess insurance must be "follow form" equal or broader in coverage scope than underlying insurance.
- 11.2.1.2 Automobile Liability-Any Automobile or Owned, Hired and Non-Owned Vehicles. Contractor must maintain Business/Automobile Liability insurance with a limit of \$1,000,000 each accident on Contractor owned, hired, and non-owned vehicles assigned to or used in the performance under this Agreement. If any Excess or Umbrella insurance is utilized to fulfill the requirements of this paragraph, the Excess or Umbrella insurance must be "follow form" equal or broader in coverage scope than underlying insurance.
- 11.2.1.3 Workers Compensation and Employers Liability Insurance. Contractor must maintain Workers Compensation insurance to cover obligations imposed by federal and state statutes having jurisdiction of Contractor employees engaged in the performance of Work under this Agreement and must also maintain Employers' Liability insurance of not less than \$1,000,000 for each accident and \$1,000,000 disease for each employee.
- 11.2.1.4 Builders' Risk/Installation Floater Insurance. The Contractor bears all responsibility for loss to all equipment or Work under construction. Unless waived in writing by the City the Contractor will purchase and maintain in force Builders' Risk/Installation Floater insurance on the entire Work until completed and accepted by the City. This insurance will be Special Causes of Loss policy form, (minimally including perils of fire, flood, lightning, explosion, windstorm and hail, smoke, aircraft and vehicles, riot and civil commotion, theft, vandalism, malicious mischief, and collapse), completed value, replacement cost policy form equal to the Agreement Price and all subsequent modifications. The Contractor's Builders'

Risk/Installation Floater insurance must be primary and not contributory.

- a. Builders' Risk/Installation Floater insurance must cover the entire Work including reasonable compensation for architects and engineers' services and expenses and other "soft costs" made necessary by an insured loss. Builders' Risk/Installation Floater insurance must provide coverage from the time any covered property comes under the Contractor's control and or responsibility, and continue without interruption during course of construction, renovation and or installation, including any time during which any Project property or equipment is in transit, off Site, or while on Site for future use or installation. Insured property must include, but not be limited to, scaffolding, false work, and temporary buildings at the Site. This insurance must also cover the cost of removing debris, including demolition as may be legally required by operation of any law, ordinance, regulation or code.
- b. The Contractor must also purchase and maintain Boiler and Machinery insurance with the same requirements as Builders' Risk/Installation Floater insurance cited above if the Work to be performed involves any exposures or insurable property normally covered under a Boiler and Machinery insurance policy or made necessary as required by law or testing requirements in the performance of this Agreement. The Contractor will be responsible for any and all deductibles under these policies and the Contractor waives all rights of recovery and subrogation against the City under the Contractor- Builders' Risk/Installation Floater insurance described herein.
- c. Builders' Risk/Installation Floater Insurance must be maintained until whichever of the following first occurs: (i) final payment has been made; or, (ii) until no person or entity, other than the City, has an insurable interest in the property required to be covered.
- d. The Builders' Risk/Installation Floater insurance must be endorsed so that the insurance will not be canceled or lapse because of any partial use or occupancy by the City.
- e. The Builders Risk/Installation Floater insurance must include as named insureds, the City, the Contractor, and all tiers of Subcontractors and others with an insurable interest in the Work who will be named as additional insureds unless they are able to provide the same level of coverage with the City and Contractor named as additional insureds. Certificates must contain a provision that the insurance will not be canceled or materially altered without at least 30 Days advance notice to the City. The City must also be named as a Loss Payee under the Builders' Risk/Installation Floater coverage.
- f. The Builders Risk/Installation Floater insurance must be written using the Special Causes of Loss policy form, replacement cost basis.
- g. All rights of subrogation under the Builders Risk/Installation Floater insurance are, by this Agreement, waived against the City, its officers, officials, agents and employees.
- h. The Contractor is responsible for payment of all deductibles under the Builders' Risk/Installation Floater insurance policy.

- 11.2.1.5 Pollution Liability Insurance (Including Errors and Omissions). For Job Orders, Pollution Liability Insurance is only required if applicable and determined on a project specific basis. Contractor must maintain Pollution Liability Insurance with a limit of not less than \$5,000,000 per loss, \$5,000,000 aggregate for losses caused by pollution conditions including coverage for bodily injury, property damage, defense costs, clean-up costs, and completed operations that arise from the operations of Contractor as described in this Agreement.
- a. The policy must provide for complete professional service coverage, including coverage for pollution liability that is a result of a breach of professional duties.
 - b. The policy must provide for protection again claims for third-party bodily injury, property damage, or environmental damage caused for pollution conditions resulting from general contracting activities for which Contractor is legally liable.
 - c. The policy must provide for cleanup costs when mandated by governmental entities, when required by law, or as a result of third-party claims.
 - d. Completed Operations Coverage must be kept in place for up to the statute of repose.
 - e. The policy must be endorsed to include the following additional insured language: "City, its elected officials, trustees, employees, agents, and volunteers must be named as additional insureds with respect to liability arising out of the activities performed by, or on behalf of Contractor".
 - f. If Work under this Agreement requires the transportation of any hazardous material or regulated substances, Contractor must carry Auto Liability with a CA 9948 endorsement or equivalent.
 - g. If Work under this Agreement requires the disposal of any hazardous materials from the job site, Contractor must obtain a certificate of insurance for Pollution Legal Liability from the disposal site operator with a limit of not less than \$5,000,000 per loss, \$5,000,000 aggregate.

11.3 **ADDITIONAL POLICY PROVISIONS REQUIRED**

- 11.3.1 Self-Insured Retentions or Deductibles. Any self-insured retentions and deductibles must be declared and approved by the City. If not approved, the City may require that the insurer reduce or eliminate any deductible or self-insured retentions with respect to the City, its officers, officials, agents, employees, and volunteers.
- 11.3.2 The Contractor's insurance must contain broad form contractual liability coverage.
- 11.3.3 The Contractor's insurance coverage must be primary insurance with respect to the City, its officers, officials, agents, and employees. Any insurance or self-insurance maintained by the City, its officers, officials, agents, and employees will be in excess of the coverage provided by the Contractor and must not contribute to it.
- 11.3.4 The Contractor's insurance must apply separately to each insured against whom claim is made or suit is brought, except with respect to the limits of the insurer's liability.
- 11.3.5 Coverage provided by the Contractor must not be limited to the liability assumed under the

indemnification provisions of this Agreement.

11.3.6 The policies must contain a severability of interest clause and waiver of subrogation against the City, its officers, officials, agents, and employees, for losses arising from Work performed by the Contractor for the City.

11.3.7 The Contractor, its successors and or assigns, are required to maintain Commercial General Liability insurance as specified in this Agreement for a minimum period of 3 years following completion and acceptance of the Work. The Contractor must submit a Certificate of Insurance evidencing Commercial General Liability insurance during this 3 year period containing all the Agreement insurance requirements, including naming the required Additional Insureds set forth herein.

11.3.8 If a Certificate of Insurance is submitted as verification of coverage, the City will reasonably rely upon the Certificate of Insurance as evidence of coverage but this acceptance and reliance will not waive or alter in any way the insurance requirements or obligations of this Agreement.

11.3.9 Insurance Cancellation During Agreement Term.

11.3.9.1 If any of the required policies expire during the life of this Agreement, the Contractor must forward renewal or replacement Certificates to the City within 10 Days after the renewal date containing all the required insurance provisions.

11.3.9.2 Each insurance policy required by the insurance provisions of this Agreement must provide the required coverage and must not be suspended, voided or canceled except after 30 Days prior written notice has been given to the City, except when cancellation is for non-payment of premium, then 10 Days prior notice may be given. Such notice must be sent directly to Chandler Law-Risk Management Department, Post Office Box 4008, Mailstop 628, Chandler, Arizona 85225. If any insurance company refuses to provide the require notice, the Contractor or its insurance broker must notify the City of any cancellation, suspension, non-renewal of any insurance within 7 Days of receipt of insurers' notification to that effect.

11.3.10 City as Additional Insured. The above-referenced policies are to contain, or be endorsed to contain, the following provisions:

11.3.10.1 The Commercial General Liability and Automobile Liability policies are to contain, or be endorsed to contain, the following provisions: The City, its officers, officials, agents, and employees are additional insureds with respect to liability arising out of activities performed by, or on behalf of, the Contractor including the City's general supervision of the Contractor; Products and Completed Operations of the Contractor; and automobiles owned, leased, hired, or borrowed by the Contractor.

11.3.10.2 The City, its officers, officials, agents, and employees must be additional insureds to the full limits of liability purchased by the Contractor even if those limits of liability are in excess of those required by this Agreement.

11.4 **BONDS AND OTHER PERFORMANCE SECURITY**

11.4.1 After Agreement award, Contractor must provide a Performance Bond and a Payment Bond, each in an amount equal to the full amount of the Agreement Price.

11.4.2 Each such bond must be executed by a surety company or companies holding a Certificate

of Authority to transact surety business in the State of Arizona, issued by the Director of the Arizona Department of Insurance and must be named in the current list of "Companies Holding Certificates of Authority as Acceptable Sureties on Federal Bonds and as Acceptable Reinsuring Companies" as published in Circular 570 (amended) by the Audit Staff Bureau of Accounts, U.S. Treasury Department. A copy of the Certificate of Authority must accompany the bonds. The Certificate must have been issued or updated within two (2) years prior to the execution of this Agreement. The bonds must be written or countersigned by an authorized representative of the surety who is either a resident of the state of Arizona or whose principal office is maintained in this state, as by law required.

- 11.4.3 The bonds must be made payable and be acceptable to City. The bond forms for the performance and payment bonds must be in the forms required under A.R.S. § 34-221, *et. Seq.*, as in Appendices 4 and 5 of these General Conditions.
- 11.4.4 Upon the request of any person or entity appearing to be a potential beneficiary of bonds covering payment of obligations arising under the Agreement Documents, Contractor must promptly furnish a copy of the bonds or must permit a copy to be made.
- 11.4.5 All bonds submitted for this Project must be provided by a company which has been rated AM Best rating of A- or better for the prior four quarters by the latest edition of the 'Results Best's Key Rating Guide (Property/Casualty)' published by the A.M. Best Company.
- 11.4.6 Personal or individual bonds are not acceptable.
- 11.4.7 If the surety on any Bond furnished by Contractor is declared a bankrupt or becomes insolvent, or Contractor's right to do business is terminated in any state where any part of the Project is located, or it ceases to meet the requirements of this Section 11.4, Contractor must within 5 Days thereafter substitute another Bond and surety, both of which must be acceptable to City.

SECTION 12 - INDEMNIFICATION

- 12.1 To the extent permitted by law, the Contractor and its owners, officers, directors, agents, employees, and subconsultants (collectively "Indemnitor") must indemnify, save, and hold harmless the City and its officers, officials, agents, and employees (collectively "Indemnitee") from any and all claims, actions, liabilities, damages, losses, or expenses (including court costs, attorneys' fees, and costs of claim processing, investigation, and litigation) (collectively "Claims") caused or alleged to be caused, in whole or in part, by the negligent, reckless, wrongful, or willful acts, errors, or omissions of Indemnitor in connection with this Agreement. This indemnity includes any Claim or amount arising out of or recovered under workers' compensation laws or on account of Indemnitor's failure to conform to any federal, state, or local law, statute, ordinance, rule, regulation, or court decree. Indemnitor must indemnify Indemnitee from and against any and all Claims, except those arising solely from Indemnitee's own negligent, reckless, wrongful, or willful acts, errors, or omissions. Indemnitor is responsible for primary loss investigation, defense, and judgment costs where this indemnification applies. In consideration of the award of this Agreement, Indemnitor agrees to waive all rights of subrogation against Indemnitee for losses arising from or related to this Agreement. Indemnitor's obligations under this provision survive the termination or expiration of this Agreement.

SECTION 13 -DISPUTE RESOLUTION

- 13.1 All disputes arising out of or relating to the Agreement, the Work or the Project, other than termination under Section 10, will be resolved pursuant to the Dispute Resolution process set forth in Appendix 6 of these General Conditions, and not pursuant to MAG Specifications § 110.
- 13.2 Contractor agrees that during any dispute between the parties, Contractor will continue to perform its obligations under the Agreement until such dispute is resolved.
- 13.3 Notwithstanding any other provision in this Agreement, City has the right to immediately file in court and pursue an action for a temporary restraining order and injunctive relief against Contractor if City determines that such action is necessary to protect its interests under the Agreement, to obtain specific performance of any provision of the Agreement, to advance the completion of the Project, or to protect health, welfare and safety.

SECTION 14 - MISCELLANEOUS PROVISIONS

14.1 AGREEMENT DOCUMENTS

- 14.1.1 The Agreement Documents are intended to permit the parties to complete the Work and all obligations required by the Agreement Documents within the Agreement Times for the Agreement Price. The Agreement Documents are intended to be complementary and interpreted in harmony so as to avoid conflict, with words and phrases interpreted in a manner consistent with construction and design industry standards.
- 14.1.2 It is the intent of the Agreement Documents to describe a functionally complete Project (or part thereof) to be constructed in accordance with the Agreement Documents. Any labor, documentation, services, materials, or equipment that may reasonably be inferred from the Agreement Documents or from prevailing custom or trade usage as being required to produce the intended result must be provided by Contractor whether or not specifically called for at no additional cost to City.
- 14.1.3 The Agreement Documents establish the rights and obligations of the parties and include the Agreement, Addenda (which pertain to the Agreement Documents), Contractor's Bid or Proposal (including documentation accompanying the Bid and any post-Bid documentation submitted prior to Agreement award) when attached as an exhibit to the Agreement, the accepted Project Schedule, the Notice to Proceed, the Performance Bond, the Payment Bond, Project Design, Engineering and Specifications, these General Conditions, the Project Specific Special Provisions, Technical Specifications, Agreement Drawings, as the same may be more specifically identified in the Agreement, Change Orders, Work Change Directives, Field Orders and the written interpretations and clarifications of the Design Professional or City representative and Modifications issued after execution of the Agreement. Approved Shop Drawings and the reports and drawings of subsurface and physical conditions are not Agreement Documents.
- 14.1.4 In the event of any inconsistency, conflict, or ambiguity between or among the Agreement Documents, the Agreement Documents will take precedence as follows from highest to lowest: Change Orders, Addenda, Agreement, Project Specific Special Provisions, General Conditions, Technical Specifications, Drawings/Plans, Chandler Amendments to MAG Standard Specifications and Chandler Standard Details, and MAG Uniform Standard Specifications and Details for Public Works Construction. If applicable to this Agreement, Federal Provisions prevail.

- 14.1.5 On the Drawings, given dimensions will take precedence over scaled measurements and large scale drawings over small-scale drawings.
- 14.1.6 Clarifications and interpretations of the Agreement Documents will be issued by the Design Professional through the City.
- 14.1.7 The headings used in this Agreement or any other Agreement Documents, are for ease of reference only and must not in any way be construed to limit or alter the meaning of any provision.
- 14.1.8 The Agreement Documents form the entire agreement between City and Contractor. No oral representations or other agreements have been made by the parties except as specifically stated in the Agreement Documents.
- 14.1.9 The Agreement Documents may not be changed, altered, or amended in any way except in writing signed by a duly authorized representative of each party in the form of a Change Order.
- 14.1.10 Re-Use of Documents. Neither Contractor nor any Subcontractor, manufacturer, fabricator, supplier or distributor will have or acquire any title to or ownership rights in any of the Drawings, Specifications or other documents (or copies of any thereof) prepared by or bearing the seal of the Design Professional; and they must not re-use any of them on extensions of the Project or any other project without written consent of City and the Design Professional and specific written verification or adaptation by the Design Professional.
- 14.2 **REFERENCE STANDARDS**
 - 14.2.1 Reference to standards, specifications, manuals, or codes of any technical society, organization, or association, or to Laws or Regulations, whether such reference be specific or by implication, will mean the standard, specification, manual, code, or Laws or Regulations in effect at the time of opening of Bids (or on the Effective Date of the Agreement if there were no Bids), except as may be otherwise specifically stated in the Agreement Documents.
 - 14.2.2 The provisions of any such standard, specification, manual or code, or any instruction of a Supplier will not change the duties or responsibilities of City, Contractor, Design Professional or Project Manager, or any of their Subcontractors, consultants, agents, or employees from those set forth in the Agreement Documents, nor will any such provision or instruction assign to City, Contractor, Design Professional, or any of their agents, or employees any duty or authority to supervise or direct the performance of the Work or any duty or authority to undertake responsibility inconsistent with the provisions of the Agreement Documents.
- 14.3 **COMPLIANCE WITH ARIZONA LAW AND FEDERAL LAW**
 - 14.3.1 Compliance with A.R.S. § 41-4401. Pursuant to the provisions of A.R.S. § 41-4401, the Contractor hereby warrants to the City that the Contractor and each of its subcontractors ("Subcontractors") must comply with all Federal Immigration laws and regulations that relate to the immigration status of their employees and the requirement to use E-Verify set forth in A.R.S. §23-214(A).
 - 14.3.2 A breach of the Contractor Immigration Warranty constitutes a material breach of this Agreement and subject to penalties up to and including termination of this Agreement.

14.3.3 The City retains the legal right to inspect the papers of any Contractor or Subcontractor employee who works on this Agreement to ensure that the Contractor or Subcontractor is complying with the Contractor Immigration Warranty. The Contractor agrees to assist the City in the conduct of any such inspections.

14.3.4 The City may, at its sole discretion, conduct random verifications of the employment records of the Contractor and any Subcontractors to ensure compliance with Contractors Immigration Warranty. The Contractor agrees to assist the City in performing any such random verifications.

14.3.5 The provisions of this Article must be included in any agreement the Contractor enters into with any and all of its subcontractors who provide services under this Agreement or any subcontract. "Services" are defined as furnishing labor, time or effort in the State of Arizona by a contractor or subcontractor. Services include construction or maintenance of any structure, building or transportation facility or improvement to real property.

14.4 **HAZARDOUS ENVIRONMENTAL CONDITIONS**

14.4.1 Contractor will not be responsible for any Hazardous Environmental Condition uncovered or revealed at the Site which was not shown or indicated in Drawings or Specifications or identified in the Agreement Document to be within the scope of the Work. Contractor will be responsible for a Hazardous Environmental Condition created with any materials brought to the Site by Contractor, Subcontractors, Suppliers, or anyone else for whom Contractor is responsible.

14.4.2 If Contractor encounters a Hazardous Environmental Condition or if Contractor or anyone for whom Contractor is responsible creates a Hazardous Environmental Condition. Contractor must immediately:

- a. Secure or otherwise isolate such condition;
- b. Stop all Work in connection with such condition and in any area affected thereby; and
- c. Notify City and promptly thereafter confirm such notice in writing.

14.4.3 City will promptly retain a qualified expert to evaluate such condition or take corrective action, if any.

14.4.4 Contractor will be responsible for any and all civil or criminal penalties, fines, damages, or other charges imposed by any regulatory agency or court for sewage discharges that are in violation of applicable statutes and laws and that are a result, direct or indirect, of work performed under this Agreement. Contractor will also be responsible for reimbursement to City for administration, reporting, and tracking expenses required as a result of any spill event. In the event the regulatory agency or court imposes a probationary period, Contractor must post bond for the probationary period to ensure that all such costs are reimbursed to City. This responsibility will apply whether penalties are imposed directly on Contractor or any of its Subcontractors, or the City of Chandler. Contractor must defend and indemnify City against such penalties. Regulatory agencies may include, but are not limited to, the Arizona Department of Environmental Quality (ADEQ) and the United States Environmental Protection Agency (USEPA).

14.5 **COOPERATION AND FURTHER DOCUMENTATION**

Contractor agrees to provide City such other duly executed documents as may be reasonably requested by City to implement the intent of the Agreement Documents.

14.6 **ASSIGNMENT**

Neither Contractor nor City will, without the written consent of the other assign, transfer or sublet any portion of this Agreement or part of the Work or the obligations required by the Agreement Documents, any such assignment will be void, will transfer no rights to the purported assignee, and would be a material breach of the Agreement.

14.7 **SUCCESSORS**

Contractor and City intend that the provisions of the Agreement Documents are binding upon the parties, their employees, agents, heirs, successors and assigns.

14.8 **LAWFUL PRESENCE**

Pursuant to A.R.S. §§ 1-501 and 1-502, City is prohibited from awarding an agreement to any natural person who cannot establish that such person is lawfully present in the United States. To establish lawful presence, a person must produce qualifying identification and sign a City-provided affidavit affirming the identification provided is genuine. This requirement will be imposed at the time of agreement award. This requirement does not apply to business organizations such as corporations, partnerships, or limited liability companies.

14.9 **NO BOYCOTT OF ISRAEL CERTIFICATION**

By Contractor's signature on this Agreement, Contractor certifies under A.R.S. § 35-393.01 that Contractor is not currently engaged in and for the duration of this Agreement will not engage in a boycott of Israel.

14.10 **NO THIRD PARTY BENEFICIARY**

Nothing under the Agreement Documents will be construed to give any rights or benefits in the Agreement Documents to anyone other than City and Contractor, and all duties and responsibilities undertaken pursuant to the Agreement Documents will be for the sole and exclusive benefit of City and Contractor and not for the benefit of any other party, unless otherwise expressly set forth in the Agreement Documents.

14.11 **GOVERNING LAW AND VENUE**

The Agreement and all Agreement Documents will be deemed to be made under, and will be construed in accordance with and governed by the laws of the State of Arizona without regard to the conflicts or choice of law provisions thereof. Any court action to enforce any provision of the Agreement or to obtain any remedy with respect hereto must be brought in the Superior Court, Maricopa County, Arizona, and for this purpose, each party hereby expressly and irrevocably consents to the jurisdiction and venue of such Court.

14.12 **SEVERABILITY**

If any provision of the Agreement Documents or the application thereof to any person or circumstance will be invalid, illegal or unenforceable to any extent, the remainder of the affected provision, the remainder of the Agreement Documents, and the application thereof will not be affected and will be enforceable to the fullest extent permitted by law.

14.13 **LEGAL REQUIREMENTS**

At all times relevant to its entry into this Agreement and performance of the Services and the Work, Contractor must fully comply with all Laws, Regulations, or Legal Requirements applicable to City, the Project, and the Agreement, including, without limitation, those set

forth on Exhibit C of the Agreement.

14.14 **PARTIAL INVALIDITY**

If any provision of the Agreement is held by a court of competent jurisdiction to be invalid, void or unenforceable, the remaining provisions will nevertheless continue in full force without being impaired or invalidated in any way.

14.15 **ATTORNEYS' FEES**

Should either party to the Agreement bring an action to enforce any provision of the Agreement, the prevailing party will be entitled to recover reasonable attorneys' fees and costs in connection therewith.

14.16 **CONFLICT OF INTEREST**

14.16.1 Contractor agrees to disclose any financial or economic interest with the Project property, or any property affected by the Project, existing prior to the execution of the Agreement. Further, Contractor agrees to disclose any financial or economic interest with the Project property, or any property affected by the Project, if Contractor gains such interest during the course of this Agreement. If Contractor gains financial or economic interest in the Project during the course of this Agreement, this may be grounds for terminating this Agreement. Any decision to terminate the Agreement must be at the sole discretion of City.

14.16.2 Contractor will not engage the services on this Agreement of any present City employee who was involved as a decision maker in the selection or approval processes, or who negotiated or approved billings or Agreement Modifications for this Agreement.

14.16.3 Contractor agrees that it will not perform services on this Project for a contractor, subcontractor, or any supplier, not covered under this Agreement.

14.17 **INDEPENDENT CONTRACTOR**

Contractor is and must be an independent contractor. Any provisions in the Agreement Documents that may appear to give City the right to direct Contractor as to the details of accomplishing the Work or to exercise a measure of control over the Work means that Contractor must follow the wishes of City as the results of the Work only. These results must comply with all applicable laws and ordinances.

14.18 **NOTICE OF INJURY**

Should City or Contractor suffer injury or damage to their person or property because of any error, omission or act of the other party or of any of the other party's employees or agents or others for whose acts the other party is legally liable, claim must be made in writing to the other party within 7 Days of the first observance of such injury or damage.

14.19 **CONFIDENTIALITY**

Contractor, for the benefit of City, hereby agrees it will not release or cause or permit to be released to the public any press notices, publicity (oral or written) or advertising promotion relating to, any statement regarding, or any other public announcement or disclosure or cause or permit to be publicly announced or disclosed, in any manner whatsoever, the specific terms and conditions of this Agreement or any comment relating to the Project or the Site. Notwithstanding the foregoing, Contractor will be entitled to disclose the terms of the Agreement to the extent required by law or in the course of enforcing or defending a claim or action hereunder. Contractor must give City reasonably prompt notice of any disclosure or statement made pursuant to this provision.

14.20 **DATA CONFIDENTIALITY**

- 14.20.1 As used in the Agreement, “data” means all information, whether written or verbal, including plans, specifications, renderings, photographs, studies, investigations, audits, analyses, samples, reports, calculations, internal memos, meeting minutes, data field notes, work product, proposals, correspondence and any other similar documents or information prepared by, obtained by, or transmitted to the City in the performance of this Agreement.
- 14.20.2 Contractor agrees that all data, regardless of form, including originals, images, and reproductions, prepared by, obtained by, or transmitted to City in connection with the Contractor’s performance of this Agreement is confidential and proprietary information belonging to City.
- 14.20.3 Except as specifically provided in this Agreement, Contractor or its Subcontractors must not divulge data to any third party without prior written consent of City. Contractor or its Subcontractors must not use data for any purposes except to perform Work required under this Agreement. These prohibitions will not apply to the following data provided Contractor has first given the required notice to City:
- a. Data which was known to Contractor or its Subcontractors prior to its performance under this Agreement unless such data was acquired in connection with Work performed for City;
 - b. Data which was acquired by Contractor or its Subcontractors in its performance under this Agreement and which was disclosed to Contractor or its Subcontractors by a third party, who to the best of Contractor’s or its Subcontractor’s knowledge and belief, had the legal right to make such disclosure and Contractor or its Subcontractors are not otherwise required to hold such data in confidence; or
 - c. Data which is required to be disclosed by virtue of law, regulation, or court order, to which the Contractor or its Subcontractors are subject.
- 14.20.4 In the event the Contractor or its Subcontractors are required or requested to disclose data to a third party, or any other information to which the Contractor or its Subcontractors became privy as a result of any other agreement with City, the Contractor must first notify City as set forth in this Section of the request or demand for the data. The Contractor or its Subcontractors must give City sufficient facts so that City can be given an opportunity to first give its consent or take such action that City may deem appropriate to protect such data or other information from disclosure.
- 14.20.5 Unless prohibited by law, within 10 Days after completion of services for a third party on real or personal property owned or leased by City, the Contractor must promptly deliver, as set forth in this Section, a copy of all data to City. All data will continue to be subject to the confidentiality agreements of this Agreement.
- 14.20.6 Contractor or its Subcontractors assume all liability for maintaining the confidentiality of the data in its possession and agree to compensate City if any of the provisions of this Section are violated by Contractor, its employees, agents, or Subcontractors. Solely for the purposes of seeking injunctive relief, it is agreed that a breach of this Section will be deemed to cause irreparable harm that justified injunctive relief in court. Contractor agrees that the requirements of this Section will be incorporated into all subcontracts entered into by Contractor. A violation of this Section may result in immediate termination of this

Agreement without notice.

14.21 **SURVIVAL**

All warranties, representations and indemnifications by Contractor must survive the completion or termination of this Agreement.

14.22 **COVENANTS AGAINST CONTINGENT FEES**

Contractor warrants that no person has been employed or retained to solicit or secure this Agreement upon an agreement or understanding for a commission, percentage, brokerage, or contingent fee, and that no member of City Council, or any employee of City has any interest, financially, or otherwise, in the firm. For breach or violation of this warrant, City will have the right to annul the Agreement without liability or at its discretion to deduct from the Agreement Price or consideration, the full amount of such commission, percentage, brokerage, or contingent fee.

14.23 **NO WAIVER**

The failure of either party to enforce any of the provisions of the Agreement Documents or to require performance of the other party of any of the provisions hereof must not be construed to be a waiver of such provisions, nor will it affect the validity of the Agreement Documents or any part thereof, or the right of either party to thereafter enforce each and every provision.

14.24 **NONEXCLUSIVE REMEDIES**

The remedies set forth in this Agreement are cumulative and not exclusive, and failure to exercise any remedy (including, without limitation, any right to terminate) will not preclude any party from exercising any other right in seeking any other remedy available to it at law or in equity.

14.25 **PROJECT COMMUNICATIONS**

14.25.1 All communications concerning the performance of the Work or the Project will be provided to the designated Project Manager and Contractor's Representative set forth in Article 1 of the Agreement. City may change the designated Project Manager and, subject to Section 4.4.20 of these General Conditions, Contractor may change Contractor Representative, by written notice to the other.

14.25.2 Project communications may be exchanged by e-mail, but email communications cannot change the terms of the Agreement or the Scope of Work, or effectuate any change that requires a written Change Order.

14.25.3 When any provisions of the Agreement Documents requires Contractor or the Design Professional to give written notice to City, it will be deemed to have been validly given if delivered in person or if delivered at or sent by registered or certified mail, postage prepaid, to the Parties indicated in Agreement Article 1, incorporated by reference.

14.25.4 When any provisions of the Agreement Documents requires City, Project Manager, or the Design Professional to give written notice to Contractor, it will be deemed to have been validly given if delivered in person to the person designated in the Agreement Documents as Contractor's Resident Superintendent, or if delivered at or sent by registered or certified mail, postage prepaid, to Contractor at the last address in the Agreement Documents or such substitute address which Contractor designates in writing, or to the business address known to the giver of notice.

14.26 **DRUG FREE WORKPLACE PROGRAM**

14.26.1 City has adopted a policy establishing a drug free workplace for itself and as a requirement for Contractors doing business with City, to ensure the safety and health of employees working on City projects.

14.26.2 Contractor must require a drug free workplace for all employees working under the Agreement. Specifically, all employees of Contractor who are working under an agreement with City must be notified, in writing, by Contractor that they are prohibited from the manufacture, distribution, dispensation, possession or unlawful use of a controlled substance in the workplace.

14.27 Failure to require a drug free workplace in accordance with the City's policy may result in termination of the Agreement and possible debarment from bidding on future City projects.

SECTION 15 - PROVISIONS APPLICABLE SOLELY TO GMP AND COST-BASED AGREEMENTS, CHANGE ORDERS, AND JOB ORDERS

15.1 **ADDITIONAL DEFINITIONS**

The definitions set forth in Section 2 apply to GMP and Cost-Based Agreements, Change Orders, and Job Orders, together with the additional definitions set forth below.

Baseline Cost Model –

A breakdown and estimate of the scope of the Project developed by CM@Risk pursuant to Section 17.5 of these General Conditions.

CM@Risk or Construction Manager at Risk –

The person or firm selected by City to provide pre-construction and/or construction services as detailed in a Construction Manager at Risk Agreement with City. In these General Conditions, the term "Contractor" includes CM@Risk under both Pre-Construction and Construction Services Agreements.

CM@Risk Fee or Contractor's Fee –

An agreed to percentage in an accepted GMP that represents the Contractor's fee for performance of the Work.

Agreement Documents –

Where compensation under the Agreement is based upon a GMP accepted by City, the term "Agreement Documents" also includes the accepted GMP Proposal.

Agreement Price –

Where compensation under the Agreement based upon a GMP accepted by City, the term "Agreement Price" refers to the GMP.

Cost-Based Agreement, Change Order, or Job Order –

A Agreement, Change Order, or Job Order where the Agreement Price is based upon the actual cost of performing the Work, subject to the terms of the Agreement Documents, including this Section 15. These would include those generally referred to as "Cost of the Work plus a Fee with a GMP," "Time and Materials," or "Cost Plus a Fee."

Cost of the Work –

The direct costs necessarily incurred by Contractor in the proper, timely, and complete performance on the Work. The Cost of the Work will include only those costs set forth in Section 15.2 of these

General Conditions.

Deliverables –

The work products prepared by Contractor in performing the scope of work described in the Agreement. Some of the major deliverables to be prepared and provided by Contractor during pre-construction may include but are not limited to: the Baseline Cost Model and Schedule that validate City's plan and budget, Construction Management Plan, Detailed Project Schedule, Schedule of Values, alternative system evaluations, procurement strategies and plans, Detailed Cost Estimates, construction market surveys, cash flow projections, GMP Proposals, Subcontractor procurement plan, Subcontractor agreements, Subcontractor bid packages, Supplier agreements, Constructability Review, Cost Control Log, Traffic control and phasing plans and others as indicated in this Agreement or required by the Project Team.

Pre-Construction Services Agreement –

The Agreement entered into between City and the CM@Risk for Pre-Construction Services to be provided by the CM@Risk, including, without limitation, the generation of a GMP Proposal. If the GMP Proposal is accepted by City and a Construction Agreement is entered into between City and CM@Risk, the duties, obligations and warranties of the CM@Risk under the Pre-Construction Services Agreement survive and are incorporated into the resulting Construction Agreement.

Pre-Construction Services –

The services to be provided under the Pre-Construction Services Agreement, including Section 17 of these General Conditions.

Detailed Project Schedule –

The Detailed Project Schedule developed by the CM@Risk for the review and approval of the Project Manager in accordance with Section 17.3 of these General Conditions, if applicable.

General Conditions Costs –

Those costs set forth in Section 4 of Appendix 9 to these General Conditions.

GMP Plans and Specifications –

The plan and specifications upon which the Guaranteed Maximum price Proposal is based.

GMP Proposal –

The proposal of Contractor submitted pursuant to Section 17.7 of these General Conditions for the entire Work and/or portion (phases) of the Work.

Guaranteed Maximum Price or GMP –

The Guaranteed Maximum Price set forth in the Agreement, Change Order, or Job Order if applicable.

15.2 AGREEMENT PRICE

15.2.1 The Agreement Price for all Agreements, Change Orders, and Job Orders based upon payment of the Cost of the Work plus a Fee with a GMP, time and materials, or cost-plus a fee will be the Cost of the Work incurred plus the Fee agreed to in writing by City, limited to the amount of the GMP, if agreed to. Unless otherwise expressly provided in the Agreement, Change Order, or Job Order, all Cost Based pricing will be subject to and limited to GMP.

15.2.2 The Agreement Price may only be changed as set forth in Section 9 above.

15.2.3 Only costs specifically designated as reimbursable costs are eligible for payment by City or may be charged against the Agreement Price. All other costs will not be paid by City and will not be chargeable against the Agreement Price.

15.2.4 Cost-Based Agreements. For Agreements, Change Orders, or Job Orders, reimbursable costs must be determined pursuant to Appendix 9 to these General Conditions, Cost of the Work, and not by MAG Specifications §109.5.

15.3 **ALLOWANCES**

15.3.1 Contractor must include in the Agreement Price all Allowances stated in the Agreement Documents and agreed to in writing by City. Items covered by these Allowances must be supplied for such amounts and by such persons as City may direct, provided Contractor will not be required to employ persons against whom Contractor makes a reasonable objection. Materials, labor, and equipment under an Allowance will be selected by City in accordance with a schedule to be mutually agreed upon by City, Design Professional and Contractor or otherwise in reasonably sufficient time to avoid delay in the Work.

15.3.2 Unless otherwise provided in the Agreement Documents:

15.3.2.1 These Allowances must cover the cost to Contractor, less any applicable trade discount, of the materials, labor, and equipment required by the Allowances, delivered at the Site, and all applicable taxes;

15.3.2.2 Contractor's costs for unloading and handling on the Site, labor, installation costs, overhead, profit and other expenses relating to materials, labor, and equipment required by the Allowance must be included in the Agreement Sum and not in the Allowance; and

15.3.2.3 Whenever the cost is more or less than the Allowance, the Agreement Sum must be adjusted accordingly by Change Order, the amount of which will recognize the difference between actual costs for an Allowance item and the amount of the Allowance item and changes, if any, in handling costs on the Site, labor, installation costs, overhead, profit and other expenses.

15.4 **CONTINGENCY**

An agreed to amount in the GMP that may only be used in accordance with the terms set forth in these General Conditions and with prior written approval by the City.

15.4.1 Construction Contingency. This GMP includes a dollar amount listed as a Construction Contingency which will be readily available for increased costs for subcontractors, material, and equipment subject to prior approval of City, which approval will not be withheld unreasonably. The Construction Contingency may also be used, at the discretion of City, to reimburse CM@Risk for unexpected costs due to (a) scope gaps between trade subcontractors; (b) agreement default by trade subcontractors; (c) unforeseen field conditions, but only as defined in Section 9.5 above; (d) work completed to meet the intent of the design, but which was not indicated on the plans; (e) costs overruns not covered by allowances; (f) costs of corrective work not provided for elsewhere; and (g) implementation of any Recovery Plan. Cost for which CM@Risk desires to be paid from the Construction Contingency must be documented by CM@Risk on a time and materials basis and are subject to verification by City. If agreed to by City, a "Use of Contingency" form will be executed by both parties authorizing the actual cost of the work to be paid and included in the Work Item Direct Costs. The Construction Contingency is not allocated to any particular

item of the Project but may be used for any portion of the work as determined above. Any amount not used in the Construction Contingency will belong to City and will reduce the GMP.

- 15.4.2 **Owner's Contingency.** This GMP also includes a dollar amount listed as an Owner's Contingency which may be used only by the City (owner department) for upgrades and changes in scope or other changes not already included within the intent of the Project Program. City will provide CM@Risk with a Work Change Directive authorizing CM@Risk to perform the additional work and to transfer funds from the Owner's Contingency to the Work Item Direct Costs category to be paid with such direct costs. These additional costs will be in an amount mutually agreed upon by CM@Risk and City or will be documented by CM@Risk on a time and materials basis and are subject to verification by City. Any amount not used in the Owner's Contingency will belong to the City and will reduce the GMP.

15.5 **REDUCTION IN RETENTION**

If the Agreement Price is based upon a GMP, in order to receive payment of one-half of the retention as set forth in Section 8.1.5 above, Contractor must also submit to the Project Manager a complete accounting of the Actual Reimbursable Cost of the Work to date, including all such documentation (including, without limitation, invoices, subcontract, subcontractor change orders, purchase orders, records of payment, etc.) as City may require, to establish whether the payments made to Contractor equal, exceed, or are less than the actual reimbursable Cost of the Work to date. Any excess payments by City, as determined by the Project Manager, will be deducted from the one-half retention payment to be made to Contractor, and any additional excess amounts paid to Contractor will be refunded by Contractor to City. The Project Manager's determinations as to Actual Reimbursable Cost of the Work will be the basis of payment until final Project Closeout and Final Payment under the Agreement.

15.6 **FINAL PAYMENT**

If the Agreement Price is based upon a GMP, as a further condition precedent to Final Payment by City, Contractor must submit to the Project Manager a complete final accounting of the Actual Reimbursable Cost of the Work, including all such documentation (including, without limitation, invoices, subcontracts, subcontractor change orders, purchase orders, records of payment, etc.) as City may require, to establish whether the payments made to Contractor equal, exceed, or are less than the Actual Reimbursable Cost of the Work to date. Any excess payments by City, as determined by the Project Manager, will be deducted from the one-half retention payment to be made to Contractor, and any additional excess amounts paid to Contractor will be refunded by Contractor to City. Disputes relating to the Final Cost of the Work will be subject to City's audit rights under Sections 8.8 above and 15.7 below, and the dispute resolution process under Section 13 above.

15.7 **OPEN BOOK**

On any GMP-based or Cost-Based Agreement, Job Order, or Change Order, City may attend any and all meetings or discussions pertaining to the Project, including bid openings, and must have access to all books, invoices, accounts, memoranda, correspondence, and written communications or records of any kind pertaining to the Project, including without limitation, those stored in electronic format.

15.8 **DIFFERING SITE CONDITIONS AND/OR CHANGE IN LAWS**

A Change Order for increased costs under Section 9.5 or 9.6 above will only be considered

or granted by City to the extent such actual, documented costs are justified.

SECTION 16 - PROVISIONS APPLICABLE SOLELY TO JOB ORDER AGREEMENTS (JOC)

16.1 ADDITIONAL DEFINITIONS

The definitions set forth in Sections 2 and below will apply to all Job Order Agreements and Job Orders. In addition, the definitions set forth in Section 15.1 above will apply to all Cost-Based Job Orders.

Agreement –

Includes the Job Order Master Agreement and Job Order Project Agreements issued and agreed to by City and JOC Contractor.

JOC Contractor's Coefficient –

A numerical factor that represents JOC Contractor total costs (indirect and direct costs, sales tax, profit, etc.) and other adjustments for market conditions.

Job Order Request for Proposal (RFP) –

The Request for Proposals issued by City for each Job Order Project Agreement relating to a specific Project.

Job Order Cost Proposal –

The Proposal submitted by JOC Contractor in response to a Job Order Request for Proposal (RFP) issued by City to develop a Job Order Project Agreement for a specific Project.

Guaranteed Maximum Price (GMP) Job Order –

Job Order under which JOC Contractor is compensated for actual costs incurred.

Job Order Project Agreement (Job Order) –

The agreement for a specific project, as it may be modified by all Change Orders, executed by the Parties, which incorporates the terms and conditions of the Job Order Master Agreement.

16.2 ORDERING AND PROCESSING PROCEDURES FOR JOB ORDERS

16.2.1 The process for developing and issuing a Job Order for a particular Project consists of three (3) procedures: (1) issuance of a RFP by City; (2) JOC Contractor's response to the RFP in the form of JOC Contractor's Job Order Cost Proposal; and (3) Issuance of a Job Order by City, as set for below.

16.2.2 RFP's For Job Orders

16.2.2.1 City will provide to JOC Contractor RFP with a Scope of Work (SOW) describing the Work to be performed, which may include special instructions and conditions, material submittal requirements, and, if applicable, a complete set of sketches, construction drawings and specifications for the Job Order.

16.2.2.2 Some Job Order RFP's will be issued by City without detailed sketches, drawing and specifications and will rely on JOC Contractor to produce them for City review and approval and is considered to be Pre-Construction and incidental design services included in JOC Contractor's overhead for GMP Job Orders. In addition JOC Contractor will not be reimbursed for any Pre-Job Order costs, including proposal preparation, attendance during negotiations, or site visits.

16.2.3 JOC Contractor's Job Order Cost Proposal

16.2.3.1 JOC Contractor must respond within ten (10) calendar days of the RFP date or site visit, whichever is later or as otherwise indicated on a case-by-case basis, by submitting JOC Contractor's Job Order Cost Proposal to the City representative.

16.2.3.2 Unless otherwise required under the terms of the RFP, JOC Contractor's Job Order Cost Proposal must include the following.

- a. JOC Contractor's Job Order Cost Proposal in PDF and electronic format;
- b. A Project Schedule and schedule of values that reflects the costs of each work element on the schedule. The schedule must show all milestones (e.g., permits, submittals, ordering materials, demolition, work phases, closeout and completion date); and
- c. Necessary documentation will be required to indicate that adequate scoping, layout, setup and planning to accomplish the Work has been done. Examples of documentation that might reasonably be expected include sketches, drawings, calculations, catalog cuts and specifications produced to a level of detail and skill that could be expected of experienced, competent Project Managers with five or more periods experience in their respective trade.

16.2.3.3 JOC Contractor must select Subcontractors based on qualifications alone or on a combination of qualifications and price and must not select Subcontractors based on price alone. A qualifications and price selection may be a one-step selection based on a combination of qualifications and price or two-step selection. In a two-step selection, the first step must be based on qualifications alone and the second step may be based on a combination of qualifications and price or on price alone.

16.2.3.4 Upon request, JOC Contractor must provide City with copies of Subcontractor quotes and the basis for selection of each Subcontractor.

16.2.3.5 If City objects to a selected Subcontractor, City will make the objection and the reasons for the objection known to the JOC Contractor. JOC Contractor must then present an acceptable Subcontractor for the applicable discipline. City will not unreasonably object to or withhold approval of a Subcontractor.

16.2.3.6 For self-performed work, the City retains the right to have the JOC Contractor establish JOC Contractor's costs by bidding their costs against at least three (3) other interested trade Contractors. No self-performed work will be allowed to be performed on a lump sum basis.

16.2.4 Issuance of Job Order

16.2.4.1 City Representative will compare the JOC Contractor's Job Order Cost Proposal with the City's estimate, schedules and other requirements, and then, if the City Representative determines it is in the best interest of City, arrange a meeting with JOC Contractor, at which time the JOC Contractor's Job Order Cost Proposal will be discussed and negotiated.

16.2.4.2 If the City Representative determines that it is in the best interest of the City, City will then issue a Job Order to JOC Contractor for execution.

16.2.4.3 Specific Job Orders may vary, but unless agreed to by City in writing otherwise, the content

of Job Orders under the Agreement will generally be as follow, all of which will be signed and/or initialed by JOC Contractor's designated representative:

- a. The description of the Scope of the Work and Project Schedule (attached as Exhibit A to the Job Order);
- b. The address or location of the Work;
- c. The Agreement Price for Work (Construction) included in the Project (attached as Exhibit B to the Job Order);
- d. The name of the JOC Contractor representative for the Project;
- e. The Drawings and Specifications for the Project;
- f. If any shop drawings, project date and/or samples are required for the Job Order, the date for delivery of each required item (included in the Project Schedule, Exhibit A to the Job order); and
- g. Project Specific Provisions, if any, in Exhibit A to the Job Order, including, without limitation, whether any of the following are required: Pre-Construction Conference, weekly progress meetings, field office, storage enclosure, materials and equipment handling facility, submittals, shop drawings, product data, equipment list, samples, project manual, schedule of values, Construction progress schedule, narrative reports, progress report, progress charts, progress photographs, materials status report, Construction diagram, Construction status report, operation and maintenance data, operating maintenance instructions and parts list, and as-build drawings.

16.2.5 Job Order Intent. Each Job Order will be interpreted to include all items reasonably necessary to complete the Project under that Job Order as described in the scope of the Work in that Job Order. All Work must be performed in a professional manner and all materials used must be new and of the highest quality and of the type best adapted to their purpose, unless otherwise specified. The Notice to Proceed date, and the award date established therein, will be deemed an integral part of the Job Order the same as if set forth therein.

16.3 **INCIDENTAL DESIGN SERVICES**

16.3.1 This effort includes all "extensions of design" for systems that are typically specified in a performance oriented manner by consultants and designers. Examples include: fire sprinkler systems, fire alarm and sprinkler systems, DDC controls, control systems, prefabricated metal building and similar situations. These designs are normally provided under submittals as a shop drawing with engineering backup and as appropriate, seals of registered engineers specializing in the particular system.

16.3.2 Incidental design includes all documents, sketches, schematic diagrams, floor plan layouts, equipment schedules and other documents produced by the JOC Contractor to define the work required for projects that the City does not develop formal or abbreviated designs requiring a seal by a registered engineer. Incidental design does not include preparation of designs requiring an architect or engineer seal.

16.3.3 JOC Contractor represents, covenants, and agrees, and contractually assumes the obligation to furnish, all of the required Design Services through properly licensed and

experienced Design Professionals in complete accordance with all of the duties imposed on a Design Professional under the Agreement Documents, Laws, Regulations, or Legal Requirements, and the common law.

- 16.3.4 All Design Documents (and all other Project-related documents, models, computer drawings and other electronic expression, photographs and other expressions CADD, and BIM files and images included) that JOC Contractor and/or JOC Contractor's Design Professional(s) prepare in connection with a Job Order and the copyrights therein (collectively, the "Instruments of Service") will be the property of City. JOC Contractor covenants and agrees to execute any additional document reasonably requested by City to confirm such assignment without any additional compensation.

16.4 **CONSTRUCTION SERVICES**

- 16.4.1 The following subsections of this Section 16.4 set forth requirements beyond those set forth in Section 4 above which apply to Construction Services performed under a Job Order.

- 16.4.2 JOC Contractor must perform the Work using only those firms, team members and individuals designated by JOC Contractor consistent with each Job Order or otherwise approved by City pursuant to the General Conditions. No other entities or individuals may be used without the prior written approval of the Project Manager.

16.4.3 Construction Phasing

- 16.4.3.1 City use of the facilities is anticipated while the Work is being performed. The Work must be planned and accomplished so that there will be a minimum of interference and inconvenience to occupants in the building and agencies in the vicinity and to other craftsmen who may have to do work in the affected facilities. Any blockage of building exits or driveways must be coordinated in advance.

- 16.4.3.2 If applicable, furniture, portable office equipment and wall appurtenances not rigidly fastened to the walls must be moved by JOC Contractor, protected from damage and replaced to the original position upon completion of the work. If the work required by the Job Order will not allow furniture and portable office equipment to be replaced to its original position, new locations will be designated by City. Incidental costs associated with moving one-piece furnishings up to approximately 150 pounds to perform such tasks as painting, carpet or tile replacement, etc., are considered a general cost of building renovation and must be included in the JOC Contractor's Coefficient. Costs for large scale or wholesale removal and replacement of large quantities of desks or modular workstations, copiers, multiple full file cabinets, etc. to another location or storage outside the work space, or disassembly and reassembly of modular furniture is not considered part of the JOC Contractor's Coefficient and will be priced separately.

- 16.4.3.3 The work must, so far as practicable, be done in definite sections or divisions and confined to limited areas which must be completed before work in other sections or divisions are begun.

- 16.4.4 Work Site Conflicts. In the event of a conflict between JOC Contractor and others in an occupied facility or where other Contractors are performing work on the same facility under other Agreements, City will decide to dispute and that decision will be final.

- 16.4.5 Ownership of Work Product. Work Product prepared or otherwise created in connection

with the performance of this Agreement, including the Work, are to be and remain the property of City. For purposes of this provision, "Work Product" will include all designs, drawings, plans, specifications, ideas, renderings and other information or matter, in whatever form created (e.g., electronic or printed) and in all media now known or hereinafter created. All Work Product will be considered Work Made for Hire as defined in the United States Copyright Act 17 U.S.C. §101 (Copyright Act). If for any reason any such Work is found not to be a work for hire, JOC Contractor hereby transfers and assigns ownership of the copyright in such Work to City. The rights in this Section are exclusive to City in perpetuity.

16.5 **OPTIONAL LIQUIDATED DAMAGES**

16.5.1 Specific Job Orders. City will have the right to assess liquidated damages in relation to any specific JOC Project Agreement as set forth below.

16.5.2 Substantial Completion Liquidated Damages. JOC Contractor acknowledges and agrees that if JOC Contractor fails to obtain Substantial Completion of the Work within the Agreement Time, City will sustain extensive damages and serious loss as a result of such failure. The exact amount of such damages will be extremely difficult to ascertain. Therefore, City and JOC Contractor agree that if JOC Contractor fails to achieve Substantial Completion of the Work within the Agreement Time, City will be entitled to retain or recover from JOC Contractor, as liquidated damages and not as a penalty, the sum per calendar day as indicated in MAG § 108.9.

16.5.3 Final Acceptance Liquidated Damages. For the same reasons set forth in Section 16.5.2 above, City and JOC Contractor further agree that if JOC Contractor fails to achieve Final Acceptance of the Work within the Agreement Time, City will be entitled to retain or recover from JOC Contractor, as liquidated damages and not as a penalty, the sum per calendar day as indicated in MAG § 108.9 commencing from the actual date of Substantial Completion or Final Acceptance as required under any specific JOC Project Agreement.

16.5.4 MAG Liquidated Damages. Liquidated damages provisions in MAG § 108.9 may apply to any specific JOC Project Agreement.

16.5.5 City may deduct liquidated damages assessed pursuant to this Section 16.5 from any unpaid amounts then or thereafter due JOC Contractor under the Agreement or any specific JOC Project Agreement between JOC Contractor and City. Any liquidated damages not so deducted from any unpaid amounts due JOC Contractor must be payable to City at the demand of City, together with interest from the date of the demand at the highest lawful rate of interest payable to JOC Contractor.

16.6 **PERFORMANCE MEASUREMENT**

16.6.1 Performance Assessment. After the Final Acceptance of Work under each Job Order, City will complete a written evaluation of the performance of JOC Contractor on the Job Order.

16.6.2 Consideration in Assignment of Work. JOC Contractor's record of cost, schedule and quality performance and comparative assessments will be significant considerations in City's determination whether to award future Job Orders. JOC Contractor agrees that any determination by City not to award future Job Orders or Option periods based on performance will be at the sole discretion of City.

**SECTION 17 - PROVISIONS APPLICABLE SOLELY TO PRE-CONSTRUCTION SERVICES FOR
CONSTRUCTION MANAGER AT RISK**

17.1 ADDITIONAL DEFINITIONS

The definitions set forth in Section 2 and 15.1 above will apply to all Pre-Construction Services Agreements.

17.2 GENERAL

17.2.1 CM@Risk must perform the Services required by, and in accordance with the Agreement Documents and as outlined in Exhibit A of the Agreement to the satisfaction of the Project Manager, exercising the degree of care, skill, diligence and judgment a professional construction manager experienced in the performance of such services for construction and/or facilities of similar scope, function, size, quality, complexity and detail to the Project in urban areas throughout the United States, would exercise at such time, under similar conditions. CM@Risk must, at all times, perform the required services consistent with sound and generally accepted engineering principles and construction management and construction contracting practices.

17.2.2 As a participating member of the Project Team, CM@Risk must provide to City and Design Professional a written evaluation of City's Project Program and budget, each in terms of the other, with recommendations as to the appropriateness of each. CM@Risk must prepare a Baseline Cost Model that validates City's budget. The Baseline Cost Model must include all assumptions and basis of estimates in enough detail so that the Project Team can compare future detail estimates to the Baseline Cost model for variances. City and Design Professional will provide all the reasonably required data that is available in order to reach agreement between the team members that the Baseline Cost Model is an accurate projection of the costs of the Project.

17.2.3 CM@Risk must attend Project Team meetings, which may include, but are not limited to, bi- weekly Project management meetings, Project workshops, special Project meetings, construction document rolling reviews, public meetings and partnering sessions. CM@Risk attendance at design or other meetings in which CM@Risk is provided the opportunity but does not actively participate and/or is not properly prepared is not acceptable. Repeated instances of non-participation and/or lack of preparedness will be grounds for termination of CM@Risk Agreement for default.

17.2.4 CM@Risk must provide Pre-Construction Services, described herein, in a timely manner and consistent with the intent of the most current Drawings and Specifications. CM@Risk must promptly notify City in writing whenever CM@Risk determines any Drawings or Specifications are inappropriate for the Project and/or cause changes in the scope of Work that deviates more than the allowed contingencies within the Baseline Cost Model or requires an adjustment in the Baseline Cost Model, Detailed Cost Estimate, Detailed Project Schedule, GMP Proposals and/or in the Agreement Time for the Work, to the extent such as established.

17.2.5 CM@Risk when requested by City, must attend, make presentations and participate as may be appropriate in public agency and or community meetings, relevant to the Project. CM@Risk must provide drawings, schedule diagrams, budget charges and other materials describing the Project when their use is required or appropriate in any such public agency meetings.

- 17.2.6 **Ownership of Work Product.** All Work Product prepared or otherwise created in connection with the performance of this Agreement, including the Work, are to be and remain the property of City. For purposes of this provision, "Work Product" will include all designs, drawings, plans, specifications, ideas, renderings and other information or material, in whatever form created (e.g., electronic or printed) and in all media now know or hereinafter created. All Work Product will be considered Work Made for Hire as defined in the United States Copyright Act 17 U.S.C.

§101 (Copyright Act). If for any reason any such Work is found not to be a work for hire, Contractor hereby transfers and assigns ownership of the copyright in such Work to City. The rights in this Section are exclusive to City in perpetuity.

- 17.2.7 CM@Risk represents to City in completing Pre-Construction Services and providing the reports and analysis required thereunder, that Work can be properly and timely constructed within the GMP Proposal, if accepted. CM@Risk does not assume any design responsibilities unless specifically called for in the scope of work, but CM@Risk will be responsible for their errors, omissions or inconsistencies included in the Work.

17.3 **DETAILED PROJECT SCHEDULE**

- 17.3.1 The fundamental purpose of the Detailed Project Schedule is to identify, coordinate and record the tasks and activities to be performed by all of the Project Team members and then for the Project Team to utilize that Deliverable as a basis for managing and monitoring all member's compliance with the schedule requirements of the Project. Each Project Team member is responsible for its compliance with the Detailed Project Schedule requirements. CM@Risk must, however, develop and maintain the Detailed Project Schedule on behalf of and to be used by the Project Team based on input from the other Project Team members. The Baseline Project Schedule must be developed as part of the Baseline Cost Model. The Detailed Project Schedule must use the Critical Path method ("CPM") technique, unless required otherwise, in writing by City. CM@Risk must use scheduling software acceptable to City to develop the Detailed Project Schedule. The Detailed Project Schedule must be presented in graphical and tabular reports as agreed upon by the Project Team. If Project phasing as described below is required, the Detailed Project Schedule must indicate milestone dates for the phases once determined. As part of construction phase, City may require CM@Risk to prepare a "resource loaded" schedule for all work, including work performed by Subcontractors, detailing each of the project tasks and the required/anticipated number of personnel per day for each task. CM@Risk must also indicate on the schedule its ability to meet said required/anticipated personnel requirements.

- 17.3.2 CM@Risk must include and integrate in the Detailed Project Schedule the services and activities required of City, Design Professional and CM@Risk including all construction phase activities based on the input received from City and the Design Professional. The Detailed Project Schedule must define activities as determined by City to the extent required to show: (a) the coordination between preliminary design and various pre-construction documents, (b) any separate long-lead procurements, (c) any permitting issues, (d) any land, right-of-way, or easement acquisition, (e) bid packaging strategy and awards to Subcontractors and Suppliers, (f) major stages of construction, (g) start-up and commissioning, and (h) occupancy of the completed Work by City. The Detailed Project Schedule must include by example and not limitation, proposed activity sequences and durations for design, procurement, construction and testing activities, milestone dates for actions and decisions by the Project Team, preparation and processing of shop drawings

and samples, delivery of materials or equipment requiring long-lead time procurement (if any), milestone dates for various construction phases, Total Float for all activities to the extent authorized by City, relationships between the activities, City's occupancy requirements showing portions of the Project having occupancy priority, and proposed dates for Final Acceptance.

- 17.3.3 A Baseline Project Schedule must be initiated with the project Baseline Cost Model and agreed to by the project team at the same time. CM@Risk must update and maintain a detailed Project Schedule throughout pre-construction such that it will not require major changes at the start of the construction phase to incorporate CM@Risk's plan for the performance of the construction phase Work. CM@Risk must provide updates and/or revisions to the Detailed Project Schedule for use by the Project Team, whenever required, but no less often than at the Project Team meetings. CM@Risk must include with such submittals a narrative describing its analysis of the progress achieved to-date vs. the Baseline Project Schedule, including any concerns regarding delays or potential delays, and any recommendations regarding mitigating actions.
- 17.3.4 If phased construction is deemed appropriate at the time of developing the Baseline Cost Model or during the development of the Detailed Project Schedule, and City approves, CM@Risk must review the design and make recommendations regarding the phased issuance of Construction Documents to facilitate phased construction of the Work, with the objective of reducing the Project Schedule and/or Cost of the Work. CM@Risk must take into consideration such factors as natural and practical lines of work severability, sequencing effectiveness, access and availability constraints, total time for completion, construction market conditions, labor and materials availability, and any other factors pertinent to saving time and cost.
- 17.3.5 Long Lead Time Items. As part of developing the Detailed Project Schedule, CM@Risk must identify all long lead time materials, fabrications, equipment, or other items which may impact the Project Schedule and may require early action on the part of the Project Team. Dates for selecting and ordering long lead time items will be included and highlighted in the Detailed Project Schedule
- 17.3.6 Equipment Plan. Contractor must develop an Equipment Plan that addresses all rental and owned equipment, regardless of whether such equipment will be provided by CM@Risk or subcontractor(s), that will be necessary to construct the Project and the cost of which will be included as a Cost of the Work in the GMP Proposal. The Equipment Plan will seek to minimize the cost of the equipment to City and maximize the efficient and coordinated use of the equipment for completion of the Project. The Equipment Plan will not only include the costs and allowable lease rates for the equipment, but will also include an equipment schedule that will be incorporated into the Detailed Project Schedule and the Schedule of Values submitted with the GMP Proposal.
- 17.4 **DESIGN DOCUMENT REVIEWS**
- 17.4.1 CM@Risk must evaluate periodically the availability of labor, materials/equipment, cost-sensitive aspects of the design; and other factors that may create an unacceptable variance to the Baseline Cost Model and/or Baseline Project Schedule.
- 17.4.2 CM@Risk must recommend, in conjunction with the Project Team, those additional surface and subsurface investigations that, in its professional opinion, are required to provide the necessary information for CM@Risk to construct the Project. These additional

- investigations, if agreed to be necessary by the Project Manager and the Design Professional, will be acquired by City and copies of the reports will be provided to CM@Risk.
- 17.4.3 CM@Risk must meet with the Project Team as required to review designs during their development. CM@Risk must familiarize itself with the evolving documents through pre- construction. CM@Risk must proactively advise the Project Team and make recommendations on factors related to construction costs, and concerns pertaining to the feasibility and practicality of any proposed means and methods, selected materials, equipment and building systems, and, labor and material availability. CM@Risk must furthermore advise the Project Team on proposed site improvements, excavation and foundation considerations, as well as, concerns that exist with respect to coordination of the Drawings and Specifications. CM@Risk must use established value analysis principles in recommending cost effective alternatives.
- 17.4.4 CM@Risk must routinely conduct constructability and bid-ability reviews of the Drawings and Specifications as necessary to satisfy the needs of the Project Team. The reviews must attempt to identify all discrepancies and inconsistencies in the Construction Documents especially those related to clarity, consistency, completeness and coordination of Work of Subcontractors and Suppliers.
- 17.4.4.1 CM@Risk must evaluate whether: (a) the Drawings and Specifications are configured to enable efficient construction; (b) design elements are standardized; (c) construction efficiency is properly considered in the Drawings and Specifications; (d) module/preassembly design is prepared to facilitate fabrication, transport and installation; (e) sequences of Work required by or inferable from the Drawings and Specifications are practicable; (f) the design has taken into consideration efficiency issues concerning access and entrance to the site, laydown and storage of materials, staging of site facilities, construction parking, and other similar pertinent issues; and (g) the design maintains continued operation of the existing City systems and maintains traffic on adjacent roadways. CM@Risk must also review the Drawings and Specifications to ensure that what is depicted therein can be constructed as designed and must promptly inform the Project Team of any issues.
- 17.4.4.2 CM@Risk must check cross-reference and complementary Drawings and sections within the Specifications and in general evaluate whether: (a) the Drawings and Specifications are sufficiently clear and detailed to minimize ambiguity and to reduce scope interpretation discrepancies; (b) named materials and equipment are commercially available and are performing well, or otherwise, in similar installations; (c) Specifications include alternatives in the event a requirement cannot be met in the field; and (d) in its professional opinion, the Project is likely to be subject to Differing Site Conditions.
- 17.4.4.3 The results of the reviews must be provided to Project Team in formal, written reports clearly identifying all reviewed documents and the discovered discrepancies and inconsistencies in the Drawings and Specifications with notations and recommendations made on the Drawings, Specifications and other documents. CM@Risk must meet with Project Team to discuss any findings and review reports.
- 17.4.4.4 CM@Risk's reviews must be from a Contractor's perspective, and though it will serve to eliminate/reduce the number of RFIs) and changes during the construction phase, responsibility for the Drawings and Specifications will remain with the Design Professional and not CM@Risk.

- 17.4.5 It is CM@Risk's responsibility to assist the Design Professional in ascertaining that, in CM@Risk's professional opinion, the Construction Documents are in accordance with applicable Laws, Regulations, or Legal Requirements, building codes, sound engineering principle's rules and regulations. If CM@Risk recognizes that portions of the Construction Documents are at variance with applicable laws, statutes, ordinances, building codes, sound engineering principle's rules and regulations, it must promptly notify the Project Team in writing, describing the apparent variance of deficiency. However, the Design Professional is ultimately responsible for the compliance of the Drawings and Specifications with those laws, statutes, ordinances, building codes, rules and regulations.
- 17.4.6 The Project Team will routinely identify and evaluate using value analysis principles and alternate systems, approaches, design changes that have the potential to reduce Project costs while still delivering a high quality and fully functional Project consistent with the Project Program. If the Project Team agrees, CM@Risk in cooperation with the Design Professional, will perform a cost/benefit analysis of the alternatives and submit such in writing to the Project Team. City, through the Project Manager, will direct which alternatives will be incorporated into the Project. The Design Professional will have full design responsibility for the review and incorporation of CM@Risk suggested alternatives into the Drawings and Specifications. CM@Risk must analyze the costs and schedule impacts of the alternatives against the Baseline Cost Model and Schedule and provide a recommendation for the Project Team's consideration and City's approval prior to the establishment of the GMP.
- 17.5 **BASELINE COSTS MODEL, DETAILED COST ESTIMATES, AND SCHEDULE OF VALUES**
- 17.5.1 At the conclusion of the Master Planning and Programming, if required, CM@Risk will review all available information regarding the design and scope of the Project using CM@Risk's experience in performing similar work, knowledge of similar projects and current and projected construction costs and, based upon that review, must develop a Baseline Cost Model for review by the Project Team and approval by City. Once approved by City, the Baseline Cost Model will be continually referenced as detailed estimates are created as the design progresses throughout Pre-Construction until the final GMP for the entire Project is established. A final GMP for the entire Project must be established and approved by City prior to the start of construction. It is the responsibility of CM@Risk to ensure City has sufficient information to evaluate and approve a final GMP prior to the time necessary to start construction so construction can be completed within the Agreement Time. The Project Detailed Cost Estimate will be the best representation from CM@Risk of what the complete functional Project's construction costs will be as indicated by the most current available documents and will be constantly checked against the Baseline Cost Model. CM@Risk must communicate to the Project Team and assumptions made in preparing the Baseline Cost Model. The Baseline Cost Model must support CM@Risk's Detailed Cost Estimates and may be broken down initially as dictated by the available information, as required by City.
- 17.5.2 After receipt of the Design Professional's most current documents from certain specified pre- construction milestones, CM@Risk must provide a draft Detailed Cost Estimate including a detailed written report detailing any variances to the Baseline Cost Model and Baseline Project Schedule. The Design Professional and CM@Risk will reconcile any disagreements on the estimate to arrive at an agreed upon Detailed Cost Estimate for the construction costs based on the scope of the Project through that specified pre-construction milestone. Pre-Construction milestones applicable to this paragraph are:

Master Planning and Programming, Schematic Design, 50% Design Development, 100% Design Development, and 50% Construction Drawings, If no consensus is reached, City will make the final determination. If the Project Team requires additional updates of the Detailed Cost Estimate beyond that specified in this paragraph, CM@Risk must provide the requested information in a timely manner.

- 17.5.3 If at any point the Detailed Cost Estimate submitted to City exceeds the previously accepted Baseline Cost Model or previously approved Detailed Cost Estimate agreed to as set forth in Section 17.5.2 above, CM@Risk must make appropriate recommendations to project Team on means/methods, materials, and or other design elements that it believes will reduce the estimated construction costs, such that it is equal to or less than the established Project Team's Baseline Cost Model.
- 17.5.4 Unless other levels of completion are agreed to in writing in the Construction Documents, at 50% Construction Drawings and included with the associated report, CM@Risk must also submit to the Project Team for review and approval a Schedule of Values that complies with the following requirements. The Schedule of Values must be based on City standard bid schedule and highlight significant variances from any previously submitted Schedule of Values. The Schedule of Values must be directly related to the breakdowns reflected in the Detailed Project Schedule and CM@Risk's Detailed Cost Estimate. In addition, the Schedule of Values must: (a) detail unit prices and quantity take-offs, (b) detail all other contingencies and unit price Work shown and specified in the detailed design documents.
- 17.5.5 CM@Risk is to track, estimate/price and address the Project Team's overall project cost issues that arise outside of the Baseline Cost Model and the latest approved Detailed Cost Estimate such as: City generated changes, Project Team proposed changes, alternate system analysis, constructability items and value engineering analysis. The system used to implement this process will be referred to as the Design Evolution Log. This is to be addressed between the Baseline Cost Model and the Master Planning and Programming Detailed Cost Estimate, and then (unless other levels of completion are agreed to in writing in the Construction Documents) between the Detailed Cost Estimates for each of the pre-construction milestones thereafter, Schematic Design, 50% Design Development, 100% Design Development, and 50% Construction Documents, and the bid packages for all Phases.
- 17.5.6 Upon request by City, CM@Risk must submit to City a cash flow projection for the Project based on the current updated/revised Detailed Project Schedule and the anticipated level of payments for CM@Risk during the design and construction phases. In addition, if requested by City and based on information provided by City, CM@Risk must prepare a cash flow projection for the entire Project based on historical records for similar types of projects to assist City in the financing process.
- 17.5.7 Construction Water. CM@Risk must estimate the quantity of water to be used and include the cost thereof in each Detailed Cost Estimate and GMP Proposal provided by City.

17.6 **SUBCONTRACTOR AND MAJOR SUPPLIER SELECTIONS**

- 17.6.1 There are two ways to select Subcontractors and major Suppliers prior to submission of a GMP Proposal: (1) qualifications-based selection; or (2) a combination of qualifications and price. Except as noted below, the selection of Subcontractors/Suppliers is the sole responsibility of CM@Risk. In any case, CM@Risk is solely responsible for the performance of the selected Subcontractors/Suppliers, and for compliance with the requirements of

- Title 34 of the Arizona Revised Statutes in the selection of a Subcontractors/Suppliers, to the extent applicable. CM@Risk must comply with its Subcontractor Selection Plan submitted with its Statement of Qualifications.
- 17.6.2 City may approve the selection of a Subcontractor(s) or Suppliers(s) based only on their qualifications when CM@Risk can demonstrate it is in the best interest of the Project. All Work that is performed, after such a qualifications-based selection, for a price that is negotiated by CM@Risk will be billed in accordance with the GMP for actual costs and may be subject to audit by City.
- 17.6.2.1 Qualifications based selection of a Subcontractor(s)/Supplier(s) should only occur prior to the submittal of the GMP Proposal.
- 17.6.2.2 If a Subcontractor/Supplier selection plan was submitted and agreed to by City, CM@Risk must apply the plan in the evaluation of the qualifications of a Subcontractor(s) or Supplier(s) and provide City with its review and recommendations.
- 17.6.2.3 CM@Risk must receive written City approval for each selected Subcontractor(s) and Supplier(s).
- 17.6.2.4 CM@Risk must negotiate costs for services/supplies from each Subcontractor/Supplier selected under this method.
- 17.6.3 All Work must be competitively bid unless a Subcontractor or Supplier was selected pursuant to Section 17.6.2 above.
- 17.6.3.1 CM@Risk must develop Subcontractor and Supplier interest, submit the names of a minimum of three qualified Subcontractors or Suppliers for each trade in the Project for approval by City and solicit bids for the various Work categories. If there are not three qualified Subcontractors/Suppliers available for a specific trade or there are extenuating circumstances warranting such, CM@Risk may request approval by City to submit less than three names. Without prior written notice to City, no change in the recommended Subcontractors/Supplies will be allowed.
- 17.6.3.2 If City objects to any nominated Subcontractor/Supplier or to any self-performed Work for good reason, CM@Risk must nominate a substitute Subcontractor/Supplier that is acceptable to City.
- 17.6.3.3 CM@Risk must distribute Drawings and Specifications, and when appropriate, conduct a Pre- Bid Conference with prospective Subcontractors and Suppliers.
- 17.6.3.4 If CM@Risk desires to self-perform certain portions of the Work, it must request to be one of the approved Subcontractor bidders for those specific bid packages. CM@Risk's bid will be evaluated in accordance with the process identified below. If events warrant and City concurs that it is necessary in order to ensure compliance with the Project Schedule and/or the most recent Detailed Cost Estimate, CM@Risk may be authorized to self-perform Work without bidding or rebidding the Work. When CM@Risk self-performs work without bidding, only the actual costs associated with performing the Work in accordance with the approved GMP will be billed and may be subject to audit by City.
- 17.6.3.5 CM@Risk must receive, open, record and evaluate the bids; provided, however, that if CM@Risk or one of its affiliates is bidding to self-perform the Work that is the subject of the bid, then the bids must be received, opened, recorded and evaluated by Project Manager

instead of CM@Risk. Bids for each category of Work must be opened and recorded at a pre-determined time. The apparent low bidders must be interviewed to determine the responsiveness of their proposals. In evaluating the responsiveness of bid proposals CM@Risk, in addition to bid price, may consider the following factors: past performance on similar projects, qualifications and experience of personnel assigned, quality management plan, approach or understanding of the Work to be performed, and performance schedule to complete the Work. The final evaluation of Subcontractor/Supplier bids must be done with Project Manager in attendance to observe and witness the process. CM@Risk must resolve any Subcontractor/Supplier bid withdrawal, protest or disqualification in connection with the award at no increase in the Cost of Work.

17.6.4 CM@Risk will be required to prepare two different reports on the subcontracting process.

17.6.4.1 Within fifteen days after each major Subcontractor/Supplier bid opening process; CM@Risk must prepare a report for City's review and approval identifying the recommended Subcontractors/Supplier for each category of Work. The report must detail: (a) the name of the recommended Subcontractor/Supplier and the amount of the Subcontractor/Supplier bid for each sub-agreement; (b) the sum of all recommended Subcontractor/Supplier bids received; (c) and trade work and its cost that CM@Risk intends to self-perform, if any.

17.6.4.2 Upon completion of the Subcontractor/Supplier bidding process, CM@Risk must submit a summary report to City of the entire Subcontractor/Supplier selection process. The report must indicate, by bid process, all Subcontractors/Suppliers contacted to determine interest, the Subcontractors/Suppliers solicited, the bids received and costs negotiated, and the recommended Subcontractors/Suppliers for each category of Work.

17.6.5 The approved Subcontractors/Suppliers will provide a Schedule of Values with their bid proposals, which will be used to create the overall Project Schedule of Values.

17.6.6 If after receipt of sub-bids or after award to Subcontractors and Suppliers, City objects to any nominated Subcontractor/Supplier or to any self-performed Work without any reasonable basis, CM@Risk must nominate a substitute Subcontractor or Supplier, preferably if such option is still available, from those who submitted Subcontractor bids for the Work affected. Once such substitute Subcontractors and Suppliers are consented to by City, CM@Risk's proposed GMP for the Work or portion thereof must be correspondingly adjusted to reflect any higher or lower costs from any such substitution.

17.7 **GMP PROPOSAL**

17.7.1 When a GMP Proposal is submitted for a phase of the Work, the GMP will have a Detailed Cost Estimate of the Costs of the Work (as set forth in Section 15.2) in each phase of the Work that is being proposed plus the current estimate for all other Work. City will not approve the GMP for the phase of work without a total estimate for the complete Project. City may request a GMP Proposal for all or any portion of the Project and at any time during pre-construction. Any GMP Proposals submitted by CM@Risk must be based on and consistent with Baseline Cost Model and the current update/revised Detailed Cost Estimate at the time of the request and include any clarifications or assumptions upon which the GMP Proposal(s) are based.

17.7.2 A GMP Proposal for the entire Project will be the sum of the Cost of the Work, CM@Risk Fee, and General Conditions Cost. CM@Risk guarantees to complete the Project at or less than

the final GMP Proposal amount plus approved Change Orders. CM@Risk will be responsible for any costs for expenses that would cause the Cost of the Work actually incurred, including the Construction Fee and General Conditions Costs, to exceed the GMP.

17.7.3 CM@Risk must prepare its GMP Proposal in accordance with City's request for GMP Proposal requirements based on the most current completed Drawings and Specifications at that time, which unless otherwise directed by City in writing, will be at 100% Construction Drawings. CM@Risk must mark the face of each document of each set upon which its GMP Proposal is based. These documents must be identified as the GMP Plans and Specifications. CM@Risk must send one set of those documents to the Project Manager, keep one set and return the third set to the Design Professional.

17.7.4 An updated/revised Detailed Project Schedule, Equipment Plan, and Schedule of Values must be included in any GMP Proposal(s), all of which must reflect the GMP Plans and Specifications the Detailed Project Schedule must be shown in relationship to the Project Schedule and identify any variance to the Baseline Project Schedule. Any such Detailed Project Schedule updates/revisions must continue to comply with the requirements of Section 17.3.1 through 17.3.5.

17.7.5 **GMP Proposals(s) Review and Approval**

17.7.5.1 CM@Risk must meet with the Project Team to review the GMP Proposal(s) and the written statement of its basis. In the event the Project Team discovers inconsistencies or inaccuracies in the information presented, CM@Risk must make adjustments as necessary to the GMP Proposal.

17.7.5.2 If during the review and negotiation of GMP Proposals design changes are required, City may authorize and cause the Design Professional to revise the GMP Plans and Specifications to the extent necessary to reflect the agreed-upon assumptions and clarifications contained in the final approved GMP Proposal. Such revised GMP Plans and Specification will be furnished to CM@Risk. CM@Risk must promptly notify the Project Team in writing if any such revised GMP Plans and Specifications are inconsistent with the agreed upon assumptions and clarifications.

17.7.6 All portions of or items comprising the GMP Proposal are subject to audit by City, as deemed appropriate by City, including, without limitation, any based upon unit prices or Work to be self-performed by CM@Risk, or its affiliates.

17.8 **PAYMENT PROCEDURE FOR PRE-CONSTRUCTION SERVICES**

17.8.1 Requests for monthly payments by CM@Risk for Pre-Construction Services must be submitted monthly and must be accompanied by a progress report, detailed invoices and receipts, if applicable. Any requests for payment must include, as a minimum, a narrative description of the tasks accomplished during the billing period, a listing of any Deliverables submitted, and copies of any Subconsultants' requests for payment, plus similar narrative and listings of Deliverables associated with their Work. Payment for services negotiated as a lump sum will be made in accordance with the percentage of work completed during the preceding month.

17.8.2 In no event will City pay more than seventy-five (75%) of the Agreement Price until acceptance of ALL Pre-Construction Services and award of the final approved Construction Services Agreement for the entire Project by City Council. If CM@Risk does not prepare a

GMP Proposal that is acceptable to City, or the GMP Proposal exceeds the City's Construction Budget, then CM@Risk understands and acknowledges that it will forfeit any right to receive the 25% of the Agreement Price being retained by City.

17.8.3 CM@Risk agrees that no charges or claims for costs or damages of any type will be made by it for any delays or hindrances beyond the reasonable control of City during the progress of any portion of Pre-Construction Services specified in this Agreement. Such delays or hindrances, if any, will be solely compensated for by an extension of time for such reasonable period and may be mutually agreed between the parties. It is understood and agreed, however, that permitting CM@Risk to proceed to complete any such Services, in whole or in part after the date to which the time of completion may have been extended, will in no way act as a waiver on the part of City of any of their respective legal rights herein.

17.8.4 No compensation to CM@Risk will be allowed contrary to Article I, Chapter I, Title 34 of the Arizona Revised Statutes.

17.8.5 If any service(s) executed by CM@Risk is abandoned or suspended in whole or in part, for a period of more than 180 days through no fault of CM@Risk, CM@Risk is to be paid for the services performed prior to the abandonment or suspension.

17.9 **SURVIVAL OF THE DESIGN SERVICES AGREEMENT, DUTIES, OBLIGATIONS AND WARRANTIES**

If the GMP Proposal is accepted by City and a Construction Agreement is entered into between City and CM@Risk, the duties, obligations and warranties of CM@Risk under the Pre- Construction Services Agreement survive and are incorporated into the resulting Construction Agreement.

General Conditions Appendices

SECTION 15 – APPENDICES

The following Appendices attached hereto are referenced in the General Conditions and are incorporated herein.

- Appendix 1 – Policy Statement for Calculating Delays and Damages
- Appendix 2 – Cost Reduction Incentive Proposals for Design Bid Build Agreements
- Appendix 3 – Contractor's Affidavit Regarding Settlement of Claims
- Appendix 4 – Forms of Performance Bond
- Appendix 5 – Forms of Payment Bond
- Appendix 6 – Dispute Resolution
- Appendix 7 – Certificate of Completion
- Appendix 8 – Construction Sign Detail
- Appendix 9 – Cost of the Work (Applicable solely to Construction Manager at Risk and Job Order Contracting)
- Appendix 10 – Landscape Establishment Period

CALCULATING DELAYS AND DAMAGES

The purpose of this policy statement is to establish guidelines and procedures for negotiation between the Contractor and City of Chandler relating to compensation for delays pursuant to Arizona Revised Statutes (A.R.S.) 34-221(F). This policy statement contains notice requirements in addition to those set forth in the Agreement Documents, and will be the Agreement Provision contemplated by that statute.

NOTE: As used herein, the term "Engineer" will refer to the City of Chandler City Engineer or his/her designated representative. Nothing in this Policy Statement will be construed to void any provision in the Agreement which requires timely notice of delays or provides for arbitration or any other procedure for settlement or provides for liquidated damages.

I. TYPES OF DELAYS:

For the purposes of this document, there are essentially four types of delays encountered by City of Chandler Construction Contractors; excusable/compensable, excusable/non-compensable, non-excusable, and concurrent. Only delays that extend Agreement Completion Time set forth in the Agreement Document will be considered for issues relating to Agreement extensions or additional compensation. All other delays are considered to be activity delays and do not entitle the Contractor to either time extensions or additional compensation. Agreement Completion Time will be defined as the date set forth in Maricopa Association of Governments (MAG) Uniform Standard Specification Section 101 and as may be modified by the Agreement Documents.

A. Excusable/Compensable:

These are delays caused solely by the City's actions or inactions, are unreasonable under the circumstances, and which were not within the contemplation of the parties to the Agreement at or prior to the time of execution of the Agreement. Since the Contractor presumably has no control over the events causing the delay, he may be entitled to both Agreement time extensions and additional compensation for delay damages. Further, he/she may be entitled to additional compensation from the impact of that delay on other work. Examples of excusable/compensable (E/C) delays include: failure to properly locate an underground City-owned utility within 2 feet of the actual location; failure to relocate City-owned utilities far enough in advance of construction in an area where the Contractor is scheduled to work that it delays start or completion of the Contractor's regularly scheduled work; failure to provide City-furnished equipment or materials in a timely manner if required by the Agreement; failure to acquire necessary Right-of-Way or Public Utility Easements prior to the Contractor beginning Work in the area; failure to timely return Shop Drawings or other Agreement Submittals in accordance with the Agreement; unreasonable delay by the City in making decisions which affect critical activities; surveying errors when the City is contractually responsible for providing Project Surveying. This list is not meant to be all inclusive, but is intended merely as examples of the type of City action or inaction which can result in a Contractor's claim for additional time and compensation.

General Conditions

Appendix 1

B. Excusable/Non-compensable:

These are delays over which neither the City nor the Contractor had control. Since both parties to the Agreement have been potentially damaged by the delay, but neither have caused it, only Time Extensions are warranted. Examples of excusable, non-compensable (E/N) delays include: unusually severe weather; fire; acts of God; failure of non-City owned utilities (SRP, CenturyLink, Cable TV, Southern Pacific Railroad, and Southwest Gas, etc.) to properly or timely locate accurately; failure of non-City owned utilities to relocate in advance of construction; the voluntary or involuntary filing for Bankruptcy protection by a Supplier or Subcontractor which causes the Supplier/Subcontractor to fail to meet a contractual deadline provided the Contractor can provide documentation that he/she executed the required Purchase Orders/Subcontract Agreements and received delivery schedules which, if met, would have eliminated the delay; delays as a result of an incomplete shutdown of a City or non-City owned utility main (the City does not guarantee a complete shutdown). This list also is not necessarily all inclusive but merely indicative of type and class of E/N delays.

C. Non-excusable/Non-compensable:

These are delays caused by the actions or inactions of Contractor or an officer, employee, agent, Subcontractor, Supplier or any other party for whom the Contractor is responsible. Since the Contractor has assumed responsibility for the risks associated with the events that caused the delay, he/she are not entitled to either time extensions or monetary delay damages. All non-excusable delays are also non-compensable. Examples of non-excusable, non-compensable (N/N) delays include: failure to perform by the Contractor, its Subcontractors and/or Suppliers (except as noted in section I.B above); failure to provide adequate labor, materials, and/or equipment on the Project; failure to perform contractually-required coordination with utilities, agencies and other Contractors; failure to notify the Engineer, in writing, of delay impacts within two working days, as required by MAG 104.2.3, or the next work day, as required by MAG 109.8.2; failure to timely submit Shop Drawings; failure to pothole or otherwise visually locate utilities sufficiently ahead of the Work to allow the Engineer to direct corrective action when necessary; delays due to retesting of previously failed work, re-inspection, and/or restaking resulting from faulty workmanship, poor quality control, or lack of compliance with Agreement Specifications. Again, this list is not necessarily all inclusive.

D. Concurrent:

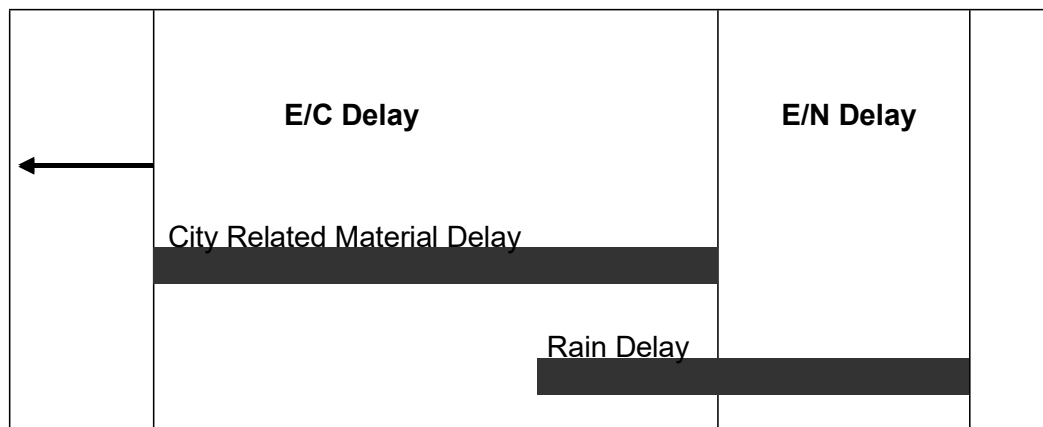
When two or more delays occur simultaneously or overlap, each delay is analyzed separately to determine its impact on the overall project completion date based on when that delay started. Once again, only those delays which actually extend the Agreement completion time are considered as delays. The concurrent delay is considered an additional delay only to the extent it prolongs the delay to the Agreement Completion Time beyond the date that the one it is concurrent with had already delayed that date. For example, if two delays are concurrent, and one is five days long and the second is seven days long, the second concurrent delay will only extend the Agreement Completion Time by two days. The same method of analysis is

General Conditions Appendix 1

used when there are multiple concurrent delays. Only those extending the Project Completion Date are considered to be delays for the purposes of this policy. The portion of each concurrent delay that delays the completion of the work is classified in the same manner as described previously for individual delays and being either E/C, E/N or N/N.

An example of a concurrent delay is where the City delays furnishing material, but the Contractor could not have installed it anyway due to unusually severe weather. The effect of the first delaying activity will extend for the full duration and will be considered controlling on the Contractor's schedule. A subsequent, concurrent delay will thereafter only be considered to effect the project completion (if at all) once the first delaying activity has ceased to impact the project completion. In this case, if the unusually severe weather continued and delayed the work after the material was delivered, the first portion of the delay would be classified E/C (delay for material) and the second as E/N (delay due to unusually severe weather). Using the procedure set forth above, the entire concurrent portion would be considered E/C as shown in the chart which follows.

Example of a Concurrent Delay:



II. ANALYZING THE DELAY:

The Contractor must provide all documents required or requested by the Engineer to analyze the delay(s). It is important to understand that, prior to the delay analysis, delays and their impacts are alleged issues. The information the Contractor provides will be compared with the Inspector's Daily Log, Schedules and other available Project information and together they will support or refute that delays occurred and, if so, how they impact other work and the overall project completion. The Contractor's delay in providing these documents will be considered prima facie evidence that either the delay did not occur or it did not impact the Project Completion Date and any claim for time extension or damages will be denied.

The Engineer will accept delay analyses in CPM format, as these may demonstrate to his/her satisfaction whether or not Project Completion has been impacted by a specific event. If the Contractor chooses not to use CPM scheduling procedures, then the burden will be on the Contractor to prove to the Engineer's satisfaction that the Project Completion has been impacted. The procedures

General Conditions Appendix 1

below assume that the Contractor is using CPM scheduling methods. As a minimum the Contractor must provide the following materials to the Engineer:

- A. As-Planned Schedule. The initial construction schedule, required by the Agreement Documents, will be considered the baseline schedule. It is to the Contractor's advantage that the As-Planned (baseline) Schedule be as detailed as possible in order for delays, as they occur, to be incorporated into the schedule in representative locations. It is also to the Contractor's advantage to use a computer software program to generate the schedule since updated schedules are required monthly by the Agreement Documents and since updated schedules are required to support delays and requests for additional compensation for delays. The As-Planned (baseline) Schedule must be presented in network format which clearly shows the interrelationships of the activities. The Contractor must also provide a printout of the activities showing early start, early finish, late start, late finish, duration and float. The activity list printout must also indicate predecessor and successor activities.
- B. As-Built Schedules. The as planned (baseline) schedule must be updated with complete progress-to-date information (actualized) up to the date of the start of the alleged delay. Each updated schedule will serve as the as-built schedule for analyzing the alleged delay and provide a new baseline as-planned schedule for the next delay. This process must be repeated for each alleged delay as it occurs. In updating the baseline schedule, the alleged delay must be treated as an activity and inserted into the schedule as a predecessor to the impacted activity (ies). When an activity that has already started is impacted, it is preferable to divide this activity into two parts and show the impact affecting the second part. For schedules which incorporate a time line (or data date), the delay activity must be inserted at the time it actually occurred. Some software scheduling programs have a PAUSE-RESUME feature that can be used to facilitate the requirements. The updated schedule must also be accompanied by a listing of activities as with the baseline schedule. This activities list must contain the alleged delay as an activity showing the duration and the activities which are predecessors and successors to it. When computer generated schedules are used, the Contractor must provide, in electronic media format, the complete data files for the updated schedule that included the delay activity, preferably either in Suretrak or in Microsoft Project. Each electronic media must contain a label identifying the Project name, Contractor's name, program name and version number, data date and project finish date.
- C. Other Documents: In order to determine the amount of the alleged delay and if it is compensable, the Contractor must provide all backup documentation relevant to the issue and as required by the Engineer. This documentation must include copies of such items as: purchase orders; delivery schedules; correspondence; memoranda of telephone calls; force account daily worksheets (initialed by the Inspector); payroll data; estimating (bid) worksheets; and any other materials which may be requested by the Engineer.
- D. Procedure: Only after receipt of sufficient documentation will the Engineer analyze each alleged delay and determine if it is supported or refuted. If supported, the Engineer will determine if it is excusable or non-excusable, compensable or non-

General Conditions Appendix 1

compensable. If the Engineer determines that the delay did not affect the Project Completion, the as-planned schedule, which has been updated to the date of the alleged delay, must be revised to indicate this. If the Engineer determines the delay did occur but was N/N, then no time extension will be granted. It is imperative that an actualized schedule be submitted as soon as the occurrence of the alleged delay is known. In no event will the Contractor submit an actualized updated schedule later than 60 days after the occurrence of the alleged delay becomes known.

If the Engineer determines the delay did occur and was excusable but, due to a reason listed in section I.B. is non-compensable, he/she will determine the length of the E/N delay and prepare a Change Order to add that time to the Agreement. The Engineer's decision will be final.

If the Engineer determines the delay was excusable and compensable, he/she will determine the length of the E/C delay and proceed to review the Contractor's damage calculations in accordance with Section III. The Engineer will check the Contractor's calculations, review the backup documentation provided, and prepare a Change Order to cover both the additional compensation and the time extension. The Engineer's decision on both the time extension and additional compensation for the delay will be final.

If the issue involves a concurrent delay, the Engineer will analyze available data to determine the portions which are E/C, E/N, and/or N/N as described above. The Engineer will proceed to determine the length of E/C delay and verify the Contractor's delay damage calculations, if any are provided. Upon completion of this review, the Engineer will prepare a Change Order for the Contractor's review and signature. The Engineer's decision regarding excusable delay and additional compensation for the delay is final.

The amount of time the Engineer will require to analyze the alleged delay(s) will depend upon the Engineer's workload, the complexity of the delay analysis, availability of supporting data, extent of cooperation by the Contractor, and other factors beyond the Engineer's control. It is entirely possible other delay(s) may occur while the Engineer is analyzing particular claim for delay(s). The Engineer's failure to respond to the Contractor in a set period of time will not be used as the basis for a further delay claim or as justification for extending and existing delay claim. The time required for delay analysis by the Engineer will not be counted against the time allotted for processing Final Payment as required by (MAG Section 109.7(B)) or the release of retention and Final Payment as prescribed by A.R.S. Arizona Revised Statutes §34-221.

III. CALCULATING MONETARY DELAY DAMAGES:

Additional compensation for delay, when authorized by the Engineer, will be calculated in accordance with MAG Section 109.5 ACTUAL COST WORK with the following exceptions:

- A. No additional compensation or other monetary damages will be awarded or paid for any loss of anticipated profits by the Contractor, Subcontractors or Suppliers.

General Conditions Appendix 1

- B. No additional compensation or other monetary damages will be awarded for home office overhead or non-project general conditions of the Contractor, Subcontractors or Suppliers.
- C. Equipment:
 - 1. Contractor-owned equipment rate calculations must be computed in accordance with Section 109.04(D)(3), Arizona Department of Transportation "Standard Specifications for Road and Bridge Construction," 2008 or latest edition and as modified herein. Year and regional adjustment factors must be based on the most recent publications of the Rental Rate Blue Book for Construction Equipment, published by the Equipment Guide-Book Company, San Jose, CA, same as provided by ADOT and in print as of the date of alleged delay. In no event will the compensation for Contractor-owned equipment exceed the purchase price, including tax, paid by the Contractor for the equipment. Compensation will not be allowed for small tools or equipment that show a daily equipment rental rate of less than \$5.00 per day or for unlisted equipment that has a value of less than for hundred dollars (\$400.00).
 - 2. For leased and rented equipment or equipment not otherwise listed in the Blue Book, rental contracts, or other supporting data will be used to establish the hourly rate. No hourly operating expense will be allowed for delay on standby equipment. In no case will equipment be considered for rental which exceeds the hourly rate for the first eight hours and the daily rate divided by eight for all additional hours as compared with similar equipment listed in the Blue Book. The hourly standby rate must be computed as the lesser of:
 - a. Dividing the monthly invoice or rental value by 176 hours per month when the equipment is utilized by the Contractor for more than three weeks;
 - b. Dividing the monthly invoice or rental value by 40 hours per week when the equipment is utilized by the Contractor for more than three days.

In no event will compensation be paid for delay at more than 8 hours per day or 40 hours per week.
 - 3. Except for vehicles used by supervisory personnel, all equipment will be paid at the "standby" rate during the delay period.
 - 4. Equipment brought solely to mitigate the delay (such as pumps, light plants, etc.) may be paid in accordance with ADOT section 109.04(D) (3).
 - 5. The Blue Book regional adjustment will apply in determining rental rates.
- D. Material:

Allowable material charges may include, in addition to material incorporated in the

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work material used to mitigate the delay such as barricades, plates, shoring, cold mix, etc. Except in emergencies the Contractor will not employ such material without the prior written approval of the Engineer.

E. Labor:

1. Except for Supervisory Personnel (Superintendent, Project Engineer, and Foremen), labor wages will not be paid after the first one-half day of claimed delay or impact. It is expected the Contractor will reassign or layoff unneeded employees.
2. For Foreman wages to be included, that Foreman must have been actively employed on the project prior to the commencement of the delay and be directly responsible for the activity being delayed.
3. Labor burden must be actual amounts incurred but must not exceed the ADOT approved rate.

F. All costs (equipment, material, and labor) must be substantiated by the City of Chandler's Daily Work Reports.

IV. DOCUMENT REQUIRED FOR CLAIM ANALYSIS:

For purposes of reviewing the Contractor's request for additional compensation, it will be required that the Contractor submit the following listed information. Information requested must be prepared on forms which are substantially similar to the City of Chandler's Daily Work Report form, a copy of which is attached as an exhibit.

A. Labor:

For each employee, laborer, and foreman, for which compensation is requested: Name, classification, dates of work performed, daily hours worked, total hours worked, labor rates, labor burden rates, overtime or premium time charges. Further, the Contractor must make available for inspection and copying to the Engineer the following listed documentation.

1. Certified payroll reports for the period of work claimed.
2. Accounting of Fringe Benefits – certified by a CPA.
3. Contractor's and Subcontractor's daily field reports and daily diaries.

B. Materials:

For all materials for which compensation is requested, if any, total quantities of materials, prices, extensions and transportation costs must be provided on a daily basis. Further, the Contractor must make available for inspection and copying to the Engineer the following listed documentation.

1. Invoices for all materials incorporated.
2. Weigh tickets.
3. Purchase orders.

General Conditions

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4. Delivery schedules.
5. Quotes or proposals from manufacturers or supplier.
6. Freight bills, Bills of Lading, or other documentation to show transportation costs.
7. Restocking charges-invoices from vendor.

C. Equipment:

For all equipment, the Contractor must provide the Engineer with the designation, dates and hours of usage, dates and hours of standby, if any, daily hours, total hours, rental rates and extension for each unit of equipment and machinery. Rental rates will be as established in Section III. Further, the Contractor must make available for inspection and copying to the Engineer the following listed documentation.

1. Owned:
 - a. Purchase contracts(s).
 - b. Depreciation schedule(s).
 - c. Invoices for fuel, lube, repairs and other operating costs.
2. Leased:
 - a. Lease agreement with hourly rate, overtime rate, double shift rate, etc.
 - b. Invoices or other documentation showing hours worked on a daily basis.

D. Subcontractors/Owner-Operators:

In the event the Contractor submits a claim which includes requests for compensation for Subcontractors of Owner-Operators, the same information requested of the Contractor must be provided by the Subcontractor/Owner-Operator. Further, the Contractor must make available for inspection and copying to the Engineer the following listed documentation.

1. Bid/Estimate work sheets and/or spreadsheets.
2. Subcontract Agreements or Agreements with Owner-Operator.
3. All invoices and billing statements received from the Subcontractor/Owner-Operator which relates to the amount requested.

E. Miscellaneous:

Further, the Contractor must make available for inspection and copying to the Engineer the following listed documentation.

1. Evidence of payment for bonds and insurance premiums (MAG 109.5.6).
2. Taxes – unless the Contractor can show otherwise, taxes are reimbursable at 65% of the total cost (less bonds and insurance).

V. TIME LIMIT ON SUBMISSIONS OF CLAIM FOR DELAY OR IMPACT DAMAGES:

No claims for delay or impact damages will be considered or allowed more than 45 days after the event or occurrence which the Contractor claims gives rise to the delay or impact. In no event will a

General Conditions
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claim for delay or impact damages be considered after submission by the Contractor of the Final Payment Request.

COST REDUCTION INCENTIVE PROPOSALS FOR DESIGN BID BUILD AGREEMENTS

The Contractor may submit to the Engineer proposals for modifying the Plans, Specifications, or other requirements of the Agreement for the sole purpose of reducing the total cost of Project construction. The proposals must not impair in any manner the essential functions or characteristics of the project; including but not limited to service life, economy of operations, ease of maintenance, desired appearance, compatibility with existing or planned equipment, standardization of systems, or design and safety standards.

It must not be inferred from this Policy that the Engineer is required to consider any proposal submitted.

Submissions that propose changes in the basic design of a bridge, propose changes in pipe line size, materials, bedding conditions, pipe specifications; or that propose any change in pavement design will not be considered.

Proposals submitted pursuant to this Policy will be identified as Cost Reduction Incentive Proposals. They must be submitted in writing and, at a minimum, contain the following.

1. Complete the attached or similar cost reduction incentive proposal form.
2. A description of both the existing Agreement Requirements for performing the work and the proposed changes.
3. All Engineering Drawings and computations necessary for the thorough and expeditious evaluation.
4. An itemization of the existing Agreement Requirements that must be changed if the Proposal is adopted and a recommendation as to the manner in which the change should be made.
5. A detailed estimate of the cost of performing the Work under the existing Agreement and under the proposed changes, including the cost of developing and implementing the changes.
6. The Agreement items affected by the proposed changes and any variations in quantities resulting from the changes.
7. An objective estimate of any effects the proposal will have on collateral cost to the City, costs of related items, and cost of maintenance and operation.
8. A statement as to the effect that the Proposal will have on the time for the completion of the Project.
9. A statement as to the time by which a Change Order adopting the Proposal must be executed or when the Engineer must have given verbal approval.

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Proposals will be processed expeditiously; however, the City will not be liable for any delay in acting upon any Proposal nor for any failure to accept any Proposal pursuant to this Special Provision.

The Engineer will be the sole judge of the acceptability of a Proposal and of the estimated net savings in construction costs from the adoption of all or any part of the Proposal. The Contractor will be notified in writing by the Engineer as to whether his/her Proposal has been accepted. The decision by the Engineer is final.

When the City deems such action to be appropriate, it reserves the right to require the Contractor to share equally in the cost to the City of investigating, evaluating, and processing the proposal as a condition for the consideration of such Proposal. Such cost must be shared whether the Proposal is accepted or rejected. When such a condition is imposed, the City will estimate these costs and the Contractor must indicate his acceptance thereof in writing. Such acceptance will authorize the City to deduct the Contractor's share of the costs from any monies due or that may become due to the Contractor under the Agreement.

If the Contractor's Proposal is accepted in whole or in part, the necessary Agreement Modifications and Agreement Price Adjustments will be affected by the execution of a Change Order which will specifically state that it is executed pursuant to this Special Provision.

The Contractor must continue to perform the work in accordance with the requirements of the Agreement until a Change Order incorporating the Proposal has been executed or until he/she has been given verbal approval by the Engineer that his/her Proposal has been accepted. If the Change Order has not been executed or he/she has not been given verbal approval on or before the date specified on the attached cost reduction incentive proposal form or on or before such other date as the Contractor may have subsequently specified in writing, the Proposal may be deemed to be rejected.

The executed Change Order will incorporate the changes in the Plans, Specifications, or other requirements of the Agreement which are necessary to permit the Proposal, or such part of it which has been accepted, to be put into effect, and will include any condition – upon which the City's approval thereof is based, if such approval is conditional. The executed Change Order may also extend the time for the completion of the Agreement if such an extension has been deemed to be warranted by the Engineer as a result of his evaluation of the Proposal.

The executed Change Order will also establish the estimated net savings in the cost of performing the Work attributable to the Proposal effectuated by the Change Order. In determining the net savings, the right is reserved to the Engineer to disregard the Agreement bid prices if, in his/her judgment, such prices do not represent a fair measure of the value of the Work to be performed or to be deleted. The net savings will be established by determining the Contractor's cost of performing the Work, taking into account his/her cost of developing the Proposal and implementing the change, and reducing this amount by any ascertainable collateral costs to the City. The executed Change Order may provide that the Contractor be paid 50 percent of the estimated net savings amount.

The executed Change Order may also provide for the adjustment in Agreement prices. Agreement prices may be adjusted by subtracting the City's share of the accrued net savings.

The amount specified to be paid to the Contractor in the executed Change Order which effectuates a Cost Reduction Proposal will constitute full compensation to the Contractor for the Cost Reduction

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Proposal and the performance of the work thereof pursuant to the said Change Order.

Upon acceptance of a Cost Reduction Incentive Proposal, any restrictions imposed by the Contractor on its use or on disclosure of the information will become void, and the City thereafter will have the right to use all or any part of the Proposal without obligation or compensation of any kind to the Contractor.

COST REDUCTION INCENTIVE PROPOSAL FORM

TO: CIP City Engineer

FROM:

PROJECT NAME: 101/202 REDUNDANT 66-INCH SEWER LINE

CITY PROJECT NUMBER: WW2402.401

DATE:

Summary of Change (Brief description of proposed change including advantages and disadvantages):

ESTIMATED COST SUMMARY (Attached detailed estimate):

A.	Original Cost:	\$	
B.	Proposed Cost:	\$	_____
C.	Construction Savings (A-B):	\$	_____
D.	Gross Savings (Included OH %, Bond %)	\$	_____
E.	Contractor Implementing	\$	_____
F.	City Implementing Cost:	\$	_____
	Reduction in Agreement Price (C+D-E-F) x 50%:	\$	_____

Date by which a Change Order must be issued so as to obtain maximum cost reduction:

**CITY OF CHANDLER, ARIZONA
PUBLIC WORKS & UTILITIES DEPARTMENT**

CONTRACTOR'S AFFIDAVIT REGARDING SETTLEMENT OF CLAIMS

_____, Arizona
Date _____

**Project Name: 101/202 Redundant 66-inch Sewer Line
City Project No.: WW2402.401**

To the City of Chandler, Arizona

Gentlemen:

This is to certify that all lawful claims for materials, rental of equipment and labor used in connection with the construction of the above project, whether by subcontractor or claimant in person, have been duly discharged.

The undersigned, for the total compensation of \$_____, as set out in the final pay application, as full and complete payment under the terms of the Agreement, hereby waives and relinquishes any and all further claims or right of lien under, in connection with, or as a result of the above described project. The undersigned further agrees to indemnify and save harmless the City of Chandler against any and all liens, claims or liens, suits, actions, damages, charges and expenses whatsoever, which said City may suffer arising out of the failure of the undersigned to pay for all labor performances and materials furnished for the performance of said installation.

Signed and dated at _____, this _____ day of _____ 20____.

CONTRACTOR

By _____

STATE OF ARIZONA)
) SS
COUNTY OF MARICOPA)

The foregoing instrument was subscribed and sworn to before me this _____ day of _____ 20_____.

Notary Public

My Commission Expires

PERFORMANCE BOND

ARIZONA STATUTORY PERFORMANCE BOND
PURSUANT TO TITLES 28, 34, AND 41, ARIZONA REVISED STATUTES
(Penalty of this bond must be 100% of the Agreement amount)

KNOW ALL MEN BY THESE PRESENTS THAT:_____

(hereinafter "Principal"), and _____ (hereinafter "Surety"), a corporation organized and existing under the laws of the State of _____ with its principal office in the City of _____, holding a certificate of authority to transact surety business in Arizona issued by the Director of Insurance pursuant to Title 20, Chapter 2, Article 1, as Surety, are held and firmly bound unto _____(hereinafter "Obligee") in the amount of _____ (Dollars) (\$_____), for the payment whereof, Principal and Surety bind themselves, and their heirs, administrators, executors, successors and assigns, jointly and severally, firmly by these presents.

WHEREAS, the Principal has entered into a certain written Agreement with the Obligee, dated the _____ day of _____, 20__ for construction of **WW2402.401, 101/202 Redundant 66-inch Sewer Line** which Agreement is hereby referred to and made a part hereof as fully and to the same extent as if copies at length herein.

NOW, THEREFORE, THE CONDITION OF THIS OBLIGATION IS SUCH, that if the Principal faithfully performs and fulfills all the undertakings, covenants, terms, and conditions of the Agreement during the original term of the Agreement and any extension of the Agreement, with or without notice of the Surety, and during the life of any guaranty required under the Agreement, and also performs and fulfills all the undertakings, covenants, terms, conditions, and agreements of all duly authorized modifications of the Agreement that may hereafter be made, notice of which modifications to the Surety being hereby waived, the above obligation is void. Otherwise it remains in full force and effect.

PROVIDED, HOWEVER that this bond is executed pursuant to the provisions of Title 34, Chapter 2, Article 2, Arizona Revised Statutes, and all liabilities on this bond will be determined in accordance with the provisions of Title 34, Chapter 2, Article 2, Arizona Revised Statutes, to the same extent as if it were copied at length in this Agreement.

The prevailing party in a suit on this bond may recover as part of the judgment reasonable attorney fees that may be fixed by a judge of the court.

Witness our hands this _____ day of _____, 20__.

AGENT OF RECORD

PRINCIPAL SEAL

By _____

AGENT ADDRESS

SURETY SEAL

PAYMENT BOND

ARIZONA STATUTORY PAYMENT BOND
PURSUANT TO TITLES 28, 34, AND 41, OF THE ARIZONA REVISED STATUTES
(Penalty of this Bond must be 100% of the Agreement amount)

KNOW ALL MEN BY THESE PRESENTS THAT:_____

(hereinafter "Principal"), as Principal, and _____(hereinafter "Surety"), a corporation organized and existing under the laws of the State of _____ with its principal office in the City of _____, holding a certificate of authority to transact surety business in Arizona issued by the Director of the Department of Insurance pursuant to Title 20, Chapter 2, Article 1, as Surety, are held and firmly bound unto _____ (hereinafter "Obligee") in the amount of _____ (Dollars) (\$_____), for the payment whereof, the Principal and Surety bind themselves, and their heirs, administrators, executors, successors and assigns, jointly and severally, firmly by these presents.

WHEREAS, the Principal has entered into a certain written Agreement with the Obligee, dated the _____ day of _____, 20__ for construction of **WW2402.401, 101/202 Redundant 66-inch Sewer Line** which Agreement is hereby referred to and made a part hereof as fully and to the same extent as if copied at length herein.

NOW, THEREFORE, THE CONDITION OF THIS OBLIGATION IS SUCH, that if the Principal promptly pays all moneys due to all persons supplying labor or materials to the Principal or the Principal's subcontractors in the prosecution of the work provided for in said Agreement, this obligation is void. Otherwise it remains in full force and effect.

PROVIDED, HOWEVER that this bond is executed pursuant to the provisions of Title 34, Chapter 2, Article 2 Arizona Revised Statutes, and all liabilities on this bond will be determined in accordance with the provisions, conditions and limitations of Title 34, Chapter 2, Article 2, Arizona Revised Statutes, to the same extent as if it were copied at length in this Agreement.

The prevailing party in a suit on this bond may recover as part of the judgment reasonable attorney fees that may be fixed by a judge of the court.

Witness our hands this _____ day of _____, 20__.

PRINCIPAL

SEAL

AGENT OF RECORD

By _____

SURETY

SEAL

AGENT ADDRESS

DISPUTE RESOLUTION

A. INFORMAL DISPUTE RESOLUTION

The parties to the Agreement agree that time is of the essence in relation to performance of the Agreement and completion of the Project, therefore any and all disputes in relation to the Agreement will initially be referred to the Project Manager, the Design Professional Representative and/or the Contractor Representative as applicable to the dispute, for immediate resolution. If, after good faith efforts to reach a resolution, none is reached, any party to the dispute may submit the dispute to the Dispute Resolution Representative ("DRR") process set forth below, which is intended to be an expedited process.

B. DISPUTE RESOLUTION REPRESENTATIVE ("DRR") PROCESS

1. The Parties under the Agreement agree that all claims and disputes in relation to the Project which are not resolved in the ordinary course of the Project ("Claim" or "Claims") will, as a prerequisite to any mediation, or litigation of the Claim, first be submitted for resolution between the designated Dispute Resolution Representatives of the Parties as set forth herein (the "DRR Process").
2. The DRR Process will be initiated through service of a DRR Notice as set forth below:
 - a. For claims by the Contractor or the Design Professional, the DRR Process will be initiated by the party asserting the claim serving written notice on the City setting forth in detail: (i) the basis for the claim; (ii) the effect of the Claim upon the construction of, and/or Project Schedule for, the Project; (iii) the specific relief requested, the amount thereof, and how such was calculated; (iv) the parties involved in the Claim, and how they are involved; (v) the specific Agreement provisions in the Agreement Documents (including, if applicable, drawings and specifications) which apply; and (vi) efforts made to date to resolve the Claim.
 - b. For claims by the City, the DRR process will be initiated by the City providing written notice to the other parties of the basis and amount of its claim, the parties involved in the Claim, and how they are involved, the provisions in the Agreement Documents that apply, and the relief requested.
 - c. The DRR Notice will be hand-delivered and e-mailed to the other parties' designated Dispute Resolution Representatives.
3. The other parties will respond in writing to the DRR Notice ("DRR Response") within ten (10) calendar days of receipt of the DRR Notice, setting forth those items set forth in the DRR Notice that they agree with, dispute, and/or have questions concerning. The DRR Response will be hand-delivered and e-mailed to the other parties' Dispute Resolution Representatives.

4. The designated Dispute Resolution Representatives for the Parties to the claim will then meet as soon as possible and in any event within twenty (20) calendar days of submission of the DRR Notice (regardless of whether a DRR Response has been submitted by all parties involved in the dispute), at a mutually agreed upon time and place, to attempt to resolve the Claim based upon the DRR Notice and DRR Response.
5. At any time after the first meeting required above, either party may terminate the DRR Process by written notice to the other party.
6. The parties may agree, in writing, to extend or modify the time limits or other provisions of the DRR process in relation to a specific pending Claim.
7. Unless otherwise designated in a written notice to the other parties, the City and the representatives of the Contractor and of the Design Professional will act as the parties' designated Dispute Resolution Representatives.
8. If a resolution of the Claim is reached, that resolution must be set forth in writing and must be signed by the Parties' designated Dispute Resolution Representative. If the resolution involves a change in any Agreement Documents, the Agreement Price, the Project Schedule, or any other change requiring a written Change Order or Amendment, the parties must execute an appropriate written Change Order or Amendment pursuant to the terms of the Agreement Documents.

C. MEDIATION

1. Unless extended by written agreement of the parties involved in the dispute, any Claim not resolved through the DRR process set forth above within five (5) calendar days after the meeting required under B (4) above, or after the DRR is terminated pursuant to B (5) above, whichever is earlier, will be submitted to mediation as a condition precedent to litigation by either party.
2. The mediation will be commenced by written demand upon the other party for mediation. If the parties cannot agree upon a mediator within ten (10) calendar days of the written demand, either party may make a request to the Civil Presiding Judge of the Maricopa County Superior Court to appoint a mediator. The mediation will occur within forth (40) calendar days of the written demand for mediation, unless the parties agree, in writing, to a longer period of time.
3. The qualifications for the mediator will be that he/she be: (a) an experienced mediator, arbitrator or litigator of construction disputes; and (b) having engaged a significant portion of his/her time involving and/or resolving construction disputes for at least the past five (5) years.
4. Each party will provide to the other party and the mediator all of the information and documentation required under B(1) and (2) above, together with any additional information and documentation which the party believes relevant. In addition, the parties will exchange, and provide to the mediator such additional memoranda, information and/or documentation, as the

mediator may request, and in the form and at such times, as the mediator may direct.

5. The parties will share the mediator's fee and any filing fees equally. The mediation will be held in Chandler, Arizona, unless another location is mutually agreed upon. Agreements reached in mediation will be specifically enforceable in any court having jurisdiction thereof.

D. LITIGATION

Any claim arising out of or related to the Agreement, except Claims relating to aesthetic effect and except those claims waived as provided for in the Agreement Documents, must be resolved through litigation in the Maricopa County, Arizona Superior Court.



**CITY OF CHANDLER, ARIZONA
PUBLIC WORKS & UTILITIES DEPARTMENT**

CERTIFICATE OF COMPLETION

PROJECT NAME:	101/202 REDUNDANT 66-INCH SEWER LINE		
CITY PROJECT NO.:	WW2402.401		
<i>If Federally Funded:</i>			
FEDERAL NO.:		ADOT NO:	

(This section to be completed by Prime)

I HEREBY CERTIFY THAT ALL GOODS AND/OR SERVICES REQUIRED BY CITY OF CHANDLER FOR THIS PROJECT HAVE BEEN DELIVERED IN ACCORDANCE WITH THE AGREEMENT DOCUMENTS AND BID SPECIFICATIONS AND ALL ACTIVITIES REQUIRED BY THE AGREEMENTOR UNDER THE AGREEMENT HAVE BEEN COMPLETED AS OF THE COMPLETION DATE LISTED HERE:

<u>FINAL ACCEPTANCE DATE:</u>	
--------------------------------------	--

PRIME CONTRACTOR:

FIRM NAME:			
PRINCIPAL:			
TITLE:			
SIGNATURE:		DATE:	

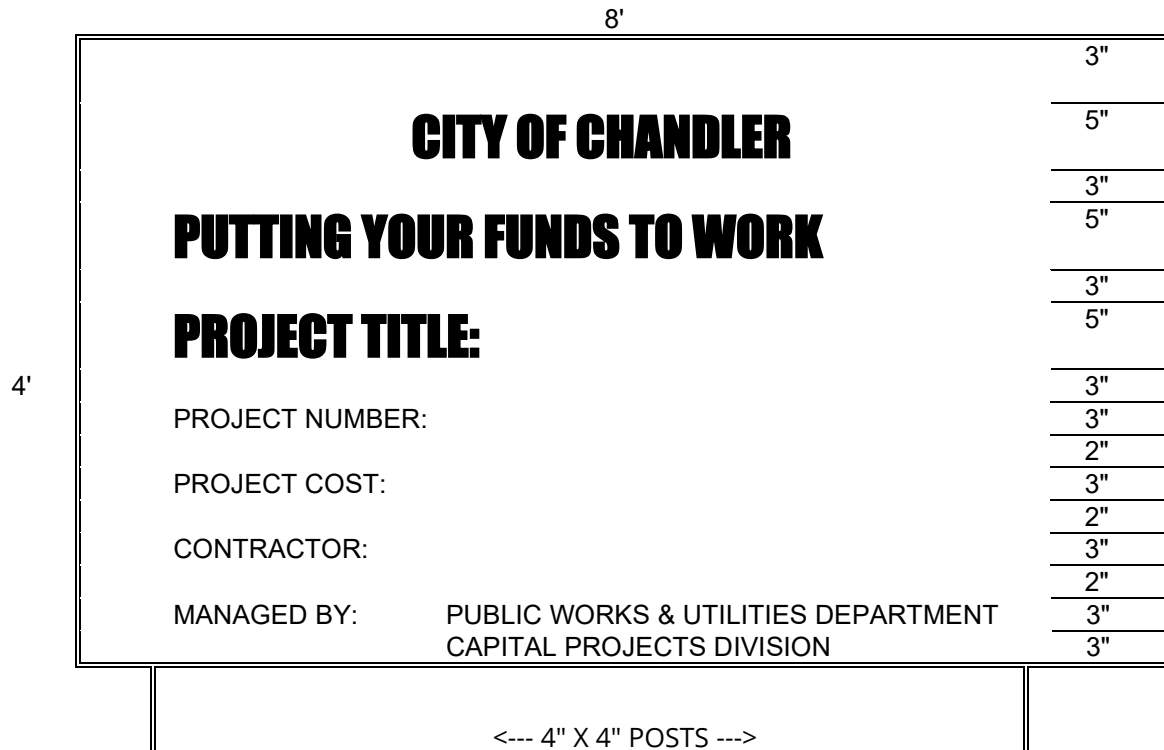
CERTIFIED BY [INSERT NAME AND TITLE OF PARTY THAT OVERSEES CONSTRUCTION]:

FIRM NAME:			
SIGNATURE:		DATE:	

PROJECT ACCEPTED BY CITY OF CHANDLER:

NAME:			
SIGNATURE:		DATE:	

CONSTRUCTION SIGN DETAIL



NOTES:

SIGN(S) MUST BE FURNISHED AND ERECTED PRIOR TO COMMENCEMENT OF CONSTRUCTION. POSTS MUST BE ANCHORED A MINIMUM OF TWO FEET INTO THE GROUND. BOTTOM OF SIGN MUST BE A MINIMUM OF FOUR FEET ABOVE THE GROUND.

TYPICAL PROJECT IDENTIFICATION SIGN FOR GENERAL PROJECTS MUST BE NON-REFLECTORIZED GREEN BACKGROUND, AND NON-REFLECTORIZED WHITE LETTERS AND NUMERALS.

ONE SIGN MUST BE ERECTED FOR BUILDINGS AND OTHER LIMITED AREA SINGLE SITES. FOR MULTIPLE SITES, ONE SIGN MUST BE ERECTED AT EACH SITE.

FOR LINEAR PROJECTS ONE HALF MILE OR LONGER, PLACE ONE SIGN AT EACH END OF THE PROJECT.

COST OF THE WORK

(APPLICABLE SOLELY TO CONSTRUCTION MANAGER AT RISK AND JOB ORDER CONTRACTING)

SECTION 1 – COSTS TO BE REIMBURSED

1.1 Cost of the Work

The term Cost of the Work will mean costs necessarily incurred by Contractor in the proper performance of the Work. Such costs must be at rates not higher than the standard paid at the place of the Project except with prior consent of City. The Cost of the Work will include only the items set forth in this Section 1.

1.2 Labor Costs

- 1.2.1 Wages of construction workers directly employed by the Contractor to perform the construction of the Work at the site or, with City's approval, at off-site workshops. Cost to be reimbursed will be the actual wages paid to the individuals performing the work.
- 1.2.2 Wages or salaries of the Contractor's supervisory and administrative personnel when stationed at the site with City's approval. No Contractor personnel stationed at the Contractor's home or branch offices will be charged to the Cost of the Work. Non-field office based Contractor management and support personnel are expected to provide service and advice from time to time throughout the job and his/her time devoted to Project matters is considered to be covered by the Contractor's Fee.
- 1.2.3 Wages and salaries of Contractor's supervisory or administrative personnel who would normally be stationed at the field office in accordance with Section 1.2.2 but who become engaged, at factories, workshops or on the road, in expediting the production or transportation of materials or equipment required for the Work, but only for that portion of his/her time required for the Work. Employee bonuses and/or costs associated with Employee Stock Ownership Plans ("ESOP") will not be considered reimbursable labor or labor burden costs and will be considered non-reimbursable costs considered to be covered by the Contractor's Fee.
- 1.2.4 Costs paid or incurred by Contractor for taxes, insurance, contributions, assessments and benefits required by law or collective bargaining agreements and, for personnel not covered by such agreements, customary benefits such as sick leave, medical and health benefits, holiday, vacations and pensions, provided such costs are based on wages and salaries included in the Cost of the Work under Subparagraphs 1.2.1 through 1.2.3.
 - 1.2.4.1 Cost of the Work will include the actual net cost to Contractor for worker's compensation insurance attributable to the wages chargeable to the Cost of Work per this Agreement. The actual net cost of worker's compensation must take into consideration all cost adjustments due to experience modifiers, premium discounts, policy dividends, retrospective rating plan premium adjustments, assigned risk pool rebates, any applicable weekly maximums, etc. Contractor may charge an estimated amount for worker's compensation insurance costs, but will make appropriate cost adjustments to actual costs within 45 days of receipt of actual cost adjustments from the insurance carrier.

- 1.2.4.3 Overtime wages paid to salaried personnel (if approved in advance in writing by City) will be reimbursed at the actual rate of overtime pay paid to the individual. No time charges for overtime hours worked on the Project will be allowed if the individual is not paid for the overtime worked.
- 1.2.4.4 Any overtime premium or shift differential expense to be incurred by Contractor for hourly workers will require City's advance written approval before the incremental cost of the overtime premium or shift differential will be considered a reimbursable cost. If the Contractor is required to work overtime as a result of an inexcusable delay or other coordination problems caused by the Contractor or anyone he/she is responsible for, the overtime premium and/or shift differential expense portion of the payroll expense and related labor burden costs will be considered as cost not to be reimbursed.
- 1.2.4.5 Reimbursable labor burden costs will be limited to payroll taxes, worker's compensation insurance, the employer's portion of union benefit costs for union employees working on the Project, and the actual verifiable fringe benefit costs incurred by Contractor for non-union individuals working on the Project subject to the following maximum percentages for the following reimbursable non-union fringe benefit costs. The following maximums (as a percentage of reimbursable actual wages by individual) will apply for each of the following types of fringe benefit costs specifically attributable to each of the non-union personnel working on the Project:

• Medical Insurance, Dental, Life & AD&D Insurance:	12.00%
• Holiday, vacation and other paid time not worked:	10.00%
• Pension Plan Contributions to Vested Employee Account, Simplified Employee Pension Plans, or 401K matching plans (Note: ESOP related costs are covered by the Contractor Fee)	10.00%

For non-union personnel, no other fringe benefit costs (other than the three specific categories listed immediately above, will be considered reimbursable Cost of Work. Any labor burden costs that are in excess of the amounts considered reimbursable or are otherwise not considered reimbursable under the terms of this agreement are intended to be covered by the Contractor Fee.

1.3 Subcontract Costs

- 1.3.1 Payments made by Contractor to Subcontractors in accordance with the requirements of the subcontracts.
- 1.3.2 For Scope of Work Bid Packages typically performed by Subcontractors, Contractor may "self-perform" such work on an actual cost basis subject to an agreed upon Guaranteed Maximum Price for the "self-performed work". The Contractor must, unless agreed to by City in writing, bid his/her proposed guaranteed Maximum Price for the work to be "self-performed" against at least three other interested trade Contractors. All savings under any such Subcontract for "self-performed work" must be applied to reduce the Cost of Work under the Agreement and the Guaranteed Maximum Price. For purposes of defining "self-performed work" subject to this provision, any division of Contractor, or any separate Contractor or Subcontractor that is partially owned or wholly owned by the Contractor or any of his/her employees or employee's relatives will be considered a related party entity and will be subject to this provision regarding "self-performed work". No self-performed work will

be allowed to be performed on a Fixed Price basis.

- 1.3.3 Contractor (with respect to its Suppliers, Subcontractors and all lower tier Subcontractors) must provide City advance written notice and must obtain City's approval for any proposed Subcontract Change Order, Material Purchase Order, or other financial commitment in an amount in excess of \$5,000 prior to placing such order or entering into such agreement (regardless of whether or not any such commitment will affect the prime Agreement Guaranteed Maximum Cost). It is agreed that sums applicable to any Subcontract Change Order, Purchase Order or other financial commitment entered into in violation of the above notice and approval requirement will not be included in the amounts owing to Contractor, Subcontractors or Suppliers whether as Costs of the Work or as reasonable termination costs in the event of termination.

1.4 Costs of Material and Equipment Incorporated in the Completed Construction

- 1.4.1 Costs, including transportation and storage, of materials and equipment incorporated or to be incorporated in the completed construction.
- 1.4.2 Costs of materials described in the preceding Subparagraph 1.4.1 in excess of those actually installed to allow for reasonable waste and spoilage. Unused excess materials, in any, will become City's property at the completion of the Work or, at City's option, may be sold by the Contractor. Any amounts realized from such sales must be credited to City as a deduction from the Cost of Work.
- 1.4.3 Proceeds from the sale of recyclable materials, scrap, waste, etc. will be credited to job cost.

1.5 Costs of Other materials and Equipment, Temporary Facilities and Related Items

- 1.5.1 Costs, including transportation and storage, installation, maintenance, dismantling and removal of materials, supplies, temporary facilities, machinery, equipment and hand tools not customarily owned by construction workers, that are provided by the Contractor at the site and fully consumed in the performance of the Work; and cost (less salvage value) of such items if not fully consumed, whether sold to others or retained by Contractor. Cost for items previously used by Contractor will mean fair market value.
- 1.5.2 Rental charges for temporary facilities, machinery, equipment, and hand tools not customarily owned by construction workers that are provided by Contractor at the site, whether rented from Contractor or others, and costs of transportation, installation, minor repairs and replacements, dismantling and removal thereof. Rates and quantities of equipment rented will be subject to City's prior written approval.
- 1.5.2.1 The Projected usage for each piece of equipment to be rented for use on the Project and the estimated total rentals must be considered by Contractor before the piece of equipment is rented so that an appropriate rent versus buy decision can be made. Purchased equipment must be considered "job owned". At the completion of the Project, Contractor must transfer title and possession of all remaining job-owned equipment to City, or Contractor may keep any such equipment for an appropriate fair market value credit to job cost, which will be mutually agreed to by City and Contractor.
- 1.5.2.2 Each piece of equipment to be rented must have hourly, daily, weekly and monthly rates and

the most economical rate available will be reimbursed based on the circumstances of actual need and usage of the piece of equipment while it is stationed at the jobsite. When the piece of equipment is no longer needed for the work, no rental charges will be reimbursed if the piece of equipment remains at the jobsite for the convenience of Contractor.

1.5.2.3 Equipment Rental Rates

1.5.2.3.1 Compensation for equipment used on the Project will be paid in accordance with the Equipment Plan submitted by Contractor in the accepted GMP Proposal and no payments will be made in excess of the rates set forth in the Equipment Plan, or actual documented costs, whichever is less.

1.5.2.3.2 All equipment rental rates and costs are subject to City's right to audit when submitted as part of Equipment Plan and/or at any time during the Project.

1.5.2.4 The aggregate rentals chargeable for each piece of Contractor owned tools or equipment must not exceed 50% of the fair market value of such equipment at the time of its commitment to the Work. The original purchase price and date of purchase of the equipment will be documented with a copy of the purchase invoice for the piece of equipment. Such aggregate limitations will apply and no further rentals will be charged even if a piece of equipment is taken off the job and is later replaced by a similar piece of equipment. For purposes of computing the aggregate rentals applicable to aggregate rental limitations, rental charges for similar pieces of equipment will be combined if the pieces of equipment were not used at the same time.

1.5.2.5 Fair market value for used material and equipment as referred to in the Agreement Documents will mean the estimated price a reasonable purchaser would pay to purchase the used material or equipment at the time it was initially needed for the job. Note: This is usually lower than the price a reasonable purchaser would pay for similar new construction material or construction equipment.

1.5.2.6 All losses resulting from lost, damaged or stolen tools and equipment will be the sole responsibility of Contractor, and not City, and the cost of such losses will not be reimbursable under the Agreement.

1.5.2.7 Contractor will be required to maintain a detailed equipment inventory of all job-owned equipment (either purchased and charged to job cost or job-owned through aggregate rentals) and such inventory must be submitted to City each month. For each piece of equipment, such inventory should contain at a minimum (1) original purchase price or acquisition cost (2) acquisition date (3) approved Fair Market Value at the time the piece of equipment was first used on the job and (4) final disposition.

1.5.2.8 All costs incurred for minor maintenance and repairs will be reimbursed at actual cost. Such costs include routine and preventative maintenance, minor repairs and other incidental costs. Repairs and/or replacement of a capital nature are considered to be covered by the rental rates. Major repairs and overhauls are not considered routine and ordinary; consequently such costs are not reimbursable and are intended to be covered by the rental rates.

1.5.3 Costs of removal of debris from the Site.

- 1.5.4 Costs of document reproductions, facsimile transmissions and long-distance telephone calls, postage and parcel delivery charges, telephone service at the site and reasonable petty cash expenses of the site office.
- 1.5.5 That portion of the reasonable expenses of Contractor's personnel incurred while traveling in discharge of duties connected with the Work.
 - 1.5.5.1 No travel expenses will be reimbursed to Contractor's representatives unless Project related travel required them to travel to a destination more than 100 miles from the Project location. Any travel involving airfare will require advance written approval by an authorized City's representative.
- 1.5.6 Costs of materials and equipment suitably stored off the site at a mutually acceptable location, if approved in advance by the City.
- 1.5.7 Reproduction costs will be the actual costs of reproduction subject to a maximum of five cents (\$.05) per square foot for prints and a maximum of five cents (\$.05) per 8 ½ by 11-inch page for offset print or photo copied agreement documents, specifications, etc. Telephone costs will be the actual costs paid to the third party telephone company for the field office telephone.

1.6 Miscellaneous Costs

- 1.6.1 That portion of insurance and bond premiums that can be directly attributed to the Agreement. The City will reimburse Contractor for contractually required bond at time of first pay application for GMP and Cost-Based Agreements upon receipt of proof of payment from the Contractor. If the Contractor completes Work for less than the Agreement Price, Contractor must credit the City a pro-rated amount for the unused portion of the bond payment
 - 1.6.1.1 Contractor's actual cost for insurance will be considered to be included within the Maximum limit for General Conditions Costs. All premiums for any insurance and bonds required for the Project must reflect the net actual costs to Contractor after taking into consideration cost adjustments due to experience modifiers, premium discounts, policy dividends, retrospective rating plan premium adjustments, assigned risk pool rebates, refunds, etc.
 - 1.6.1.2 The amount to be reimbursed to Contractor for all contractually required insurance will be actual costs not to exceed a total of 2% of the Agreement Value, unless Contractor establishes to City's satisfaction that the actual cost is higher and City agrees to such actual higher cost in writing. If Contractor's cost of contractually required insurance is greater than the amount agreed to be reimbursed per this Agreement Provision, the difference will be considered to be covered by the Contractor's Fee. The City will reimburse Contractor for contractually required insurance on a monthly basis for GMP and Cost-Based Agreements. If Contractor can demonstrate substantial savings by paying for all insurance in advance, the City may agree to reimburse all insurance costs at time of first pay application for GMP and Cost-Based Agreements with proof of payment from Contractor.
- 1.6.2 Sales, use or similar taxes imposed by a governmental authority that are related to the Work.

- 1.6.3 Fees and assessments for the building permit and for other permits, licenses and inspections for which Contractor is required by the Agreement Documents to pay.
- 1.6.4 Fees of laboratories for tests required by the Agreement Documents, except those related to defective or nonconforming Work and which do not fall within the scope of ¶ 1.7.3 below.
- 1.6.5 Royalties and license fees paid for the use of a particular design, process or product required by the Agreement Documents; the cost of defending suites or claims for infringement of patent rights arising from such requirement of the Agreement Documents; and payments made in accordance with legal judgments against Contractor resulting from such suites or claims and payments of settlements made with City's consent. However, such costs of legal defenses, judgments and settlements must not be included in the calculation of the Contractor's Fee or subject to the Guaranteed Maximum Price.
- 1.6.6 Data processing costs related to the Work. However, any such data processing costs will be limited to the cost of personal computer hardware used at the field office in the normal day to day administration, management and control of the Project. The aggregate charges for any such hardware must not exceed the Fair Market Value of the hardware at the time it was brought to the field office. If the total charges for any particular piece of hardware reach an amount equal to the Fair Market Value, that particular piece of hardware must be turned over to City whenever it is no longer needed for the Project. If Contractor elects to keep the particular piece of hardware, the job costs must be credited with a mutually agreeable amount which will represent the Fair Market Value of the particular piece of hardware at the time it was no longer needed for the job. Software or other costs associated with the use of computer programs will not be considered to be a reimbursable cost and will be considered to be covered by the Contractor's Fee.
- 1.6.7 Deposits lost for causes other than Contractor's negligence or failure to fulfill a specific responsibility to City as set forth in the Agreement Documents.
- 1.6.8 Legal, mediation and arbitration costs, including attorneys' fees, other than those arising from disputes between City and Contractor, reasonably incurred by Contractor in the performance of the Work and with City's prior written approval; which approval will not be unreasonably withheld.
- 1.6.9 Expenses incurred in accordance with Contractor's standard personnel policy for relocation and temporary living allowances of personnel required for the Work, if pre-approved by City in writing. If City authorizes the reimbursement of relocation costs, the reimbursable relocation expenses will be limited to a maximum of \$50,000 per person. Any relocation cost incurred by Contractor in excess of the amount reimbursed by City will be considered to be covered by the Contractor's Fee.

1.7 Other Costs and Emergencies

- 1.7.1 Other costs incurred in the performance of the Work if and to the extent approved in advance in writing by City.
- 1.7.2 Costs due to emergencies incurred in taking action to prevent threatened damage, injury or

loss in case of an emergency affecting the safety of persons and property.

- 1.7.3 Costs of repairing or correcting damaged or nonconforming Work executed by Contractor, Subcontractors or Suppliers, provided that such damaged or nonconforming Work was not caused by negligence or failure to fulfill a specific responsibility of Contractor and only to the extent that the cost of repair or correction is not recoverable by Contractor from insurance, sureties, Subcontractors or Suppliers.

1.8 Related Party Transactions

- 1.8.1 The term “related party” will mean a parent, subsidiary, affiliate or other entity having common ownership or management with Contractor; any entity in which any stockholder in, or management employee of, Contractor owns any interest in excess of ten percent in the aggregate; or any person or entity which has the right to control the business or affairs of Contractor. The term “related party” includes any member of the immediate family of any person identified above.
- 1.8.2 If any of the costs to be reimbursed arise from a transaction between Contractor and a related party, Contractor must notify City in writing of the specific nature of the contemplated transaction, including the identity of the related party and the anticipated cost to be incurred, before any such transaction is consummated or cost incurred. If City, after such notification, authorizes in writing the proposed transaction, then the cost incurred will be included as a cost to be reimbursed, and Contractor must procure the Work, equipment, goods or service from the related party, as a Subcontractor. If City fails to authorize the transaction, Contractor must procure the Work, equipment, goods or service from some person or entity other than a related party.

SECTION 2 – COSTS NOT TO BE REIMBURSED

- 2.1 The Cost of Work must not include:
- 2.1.1 Salaries and other compensation of Contractor’s personnel stationed at Contractor’s principal office or offices other than the site office, except as specifically provided in Subparagraphs 1.2.2 and 1.2.3.
- 2.1.2 Expenses of Contractors’ principal office and offices other than the site office.
- 2.1.3 Overhead and general expenses, except as may be expressly included in Section 1.
- 2.1.3.1 Costs of Contractor’s home office computer services or other outside computer processing services will be considered overhead and general expense. Accordingly, Contractor should not plan to perform any such computer related services or alternatives at the field office when such services or functions can be performed at Contractor’s home or branch offices, or other outside service locations.
- 2.1.4 Contractor’s capital expenses, including interest on Contractor’s capital employed for the Work.
- 2.1.5 Rental costs of machinery and equipment, except as specifically provided in subparagraph 1.5.2.

- 2.1.6 Except as provided in Subparagraph 1.7.3 of the Agreement, costs due to the negligence or failure to fulfill a specific responsibility of Contractor, Subcontractors and Suppliers or anyone directly or indirectly employed by any of them or for whose acts of them may be liable.
- 2.1.7 Any cost not specifically and expressly described in Section 1.
- 2.1.8 Costs, other than costs included in Change Orders approved by City that would cause the GMP to be exceeded.

SECTION 3 – DISCOUNTS, REBATES, REFUNDS AND SAVINGS

- 3.1 Cash discounts obtained on payments made by Contractor will accrue to City if (1) before making the payment, Contractor included them in an Application for Payment and received payment therefore from City, or (2) City has deposited funds with Contractor with which to make payments; otherwise, cash discounts will accrue to Contractor. Trade discounts, rebates, refunds and amounts received from sales or surplus materials and equipment will accrue to City, and Contractor must make provisions so that they can be secured.
- 3.1.1 Cost of the Work will be credited with all insurance policy discounts, performance and payment bond rebates or refunds, refunds or return premiums from any Subcontractor default insurance, refunds or rebates from any Contractor controlled insurance programs applicable to the Project, merchandise rebates of any nature, refunds of any nature, insurance dividends; and a portion of any volume rebates or free material credits earned with purchase of material or other goods and services charged to the job.
- 3.1.2 “Cash” discounts which may accrue to Contractor will be limited to a maximum of 1.5% of invoice cost. Any portion of “Cash” discounts greater than 1.5% will automatically accrue to City if Contractor is eligible to take advantage of the discounts.
- 3.2 Amounts that accrue to City in accordance with the provisions of Paragraph 3.1 will be credited to City as a deduction from the Cost of the Work.
- 3.3 Any and all savings on the GMP, or any separately guaranteed items comprising the GMP, will belong to City, subject to any express right in the Agreement for Contractor to share in savings. Savings are subject to City’s right to audit, and may be audited separately.

SECTION 4 – GENERAL CONDITIONS COSTS

- 4.1 General Conditions Costs may include, but are not limited to, the following types of costs incurred by Contractor during construction of the Work to the extent they are reimbursable Costs of the Work as delineated above: payroll costs for Work conducted at the site, payroll costs for the superintendent and full-time general foremen, payroll costs for management personnel resident and working on the site workers not included as direct labor costs engaged in support (e.g. loading/unloading, clean-up, etc.), administrative office personnel, costs of offices and temporary facilities including office materials, office supplies, office equipment, minor expenses, utilities, fuel, sanitary facilities and telephone services at the site, costs of liability insurance premiums not included in labor burdens for direct labor

costs, costs of bond premiums, costs of consultants not in the direct employ of Contractor or Subcontractors, fees for permits and licenses.

- 4.2 General Conditions Costs may be paid on a percentage of the Agreement Price or on a lump/stipulate sum basis as set forth in the Agreement. All costs included in the General Conditions Costs will not be separately invoiced to or paid by City.
- 4.3 The total amount of General Conditions Costs for the Work may be divided by the number of days allowed for performance of the Work, to determine a fixed daily rate for General Conditions Costs that may be used in computing the General Conditions Costs allocated to any period of time, or for any adjustments in the General Conditions Costs agreed to in writing by City.

LANDSCAPE ESTABLISHMENT PERIOD

Section 1 – Description and General

Contractor is in direct control of work performed under the Landscape Establishment Period. If work is subcontracted, a representative of the Contractor will be present at the site of the work for all hours that the subcontractor works. Subcontracting of Landscape Establishment work will be permitted for weed eradication with herbicides, because of special licensing. A licensed temporary service may be used to supply labor to Contractor if Contractor has received approval from City Representatives. Contractor will submit the required subcontract documentation.

Contractor must provide adequate personnel to accomplish the required maintenance of the plant materials at intervals acceptable to City Representatives.

If not healthy at the end of the maintenance period, the maintenance must be continued until the plant material is approved by City.

1.1 Time and Schedule

Unless otherwise expressly agreed to in writing by City, the Landscape Establishment Period will be per General Conditions Section 6.5.4.

1.2 Planted Stock and Seeding Establishment

- 1.2.1 Tree planting and staking must be per City of Chandler Standard Detail C-801.
- 1.2.2 All trees will stand erect on their own without stakes when brought to this site. If the tree cannot stand on its own when nursery stakes are removed, the tree will be removed and replaced.

1.3 Pre-Emergent Herbicide and Weed Control

- 1.3.1 Contractor will provide three applications of an approved pre-emergent herbicide on all unpaved areas of the project, as directed by City Representatives, to control weed growth in all areas of the project. The number of applications may be increased as directed by the City Representative, and at no additional cost to City, if the City Representative deems additional applications are required to control weed growth.
- 1.3.2 Application sequence will be approved in advance by City. The first application of pre-emergent will be completed prior to the application of Decomposed Granite and will be included with the cost of the Decomposed Granite as specified and part of the Construction Phase portion of work. The second application of pre-emergent will be completed after installation of the Decomposed Granite and no later than half-way through the Landscape Establishment Period. The third and final application of pre-emergent will be applied 15 days prior to completion of the Landscape Establishment portion of the project. The second and third pre-emergent applications will be included with the cost of Landscape Establishment. Watering will be completed in accordance with the manufacturer's recommendations, as included and as related to each application.
- 1.3.3 The pre-emergent herbicide will be applied in accordance with the Technical Specifications and the recommendations of the pre-emergent herbicide manufacturer, as approved by City Representatives.

- 1.3.4 The control of weeds will be accomplished by the use of herbicides. Manual removal of weeds will be required, after herbicides have taken affect.
- 1.3.5 Contractor is responsible for the removal and disposal of all trash and debris that during the Landscape Establishment Period. Contractor will keep the project in a neat and orderly manner during the duration of the Landscape Establishment Period.

1.4 Water

The water used during Landscape Establishment to properly maintain the plant material will be furnished by City, at designated sources from within the project limits, at no charge to Contractor. Contractor will be responsible for all equipment, materials and labor necessary to load, transport and unload water for watering purposes.

1.5 Plant Material Replacements

The plant material replacement will be considered as included in the work for Landscape Establishment, and will be made at no charge to the City.

- 1.5.1 Shrub and Plant Replacement - During the second half of the Landscaping Establishment period, Contractor will provide, where required, plant replacements as follows:

<u>Original Size</u>	<u>Replacement Size</u>
1 gallon	5 gallon
5 gallon	15 gallon
15 gallon	24-inch box
36-inch box	48-inch box

- 1.5.2 Tree Replacement – During the second half of the Landscape Establishment Period, Contractor will provide plant material replacements for existing plants that die as follows:

<u>Existing Plant Material Sizes</u>	<u>Replacement Size</u>
Trees:	
2-inch Caliper	24-inch box
4-inch Caliper	36-inch box
6-inch Caliper and greater	54-inch box
Shrubs:	
All Existing Shrubs	15 gallon

1.6 Measurement and Payment

See Technical Specifications for Measurement Payment provisions.

EXHIBIT C

TECHNICAL SPECIFICATIONS



CITY OF CHANDLER

PUBLIC WORKS AND UTILITIES DEPARTMENT

LOOP 101 AND 202 FREEWAY 66-INCH REDUNDANT SEWER PROJECT

PROJECT NO. WW2402.401

AGENCY REVIEW

Technical Specifications

MAY 2025

MAYOR
KEVIN HARTKE

VICE MAYOR
CHRISTINE ELLIS

CITY COUNCIL

Council Member Angel Encinas
Council Member Od Harris
Council Member Jane Poston

Council Member Matt Orlando
Council Member Jennifer Hawkins

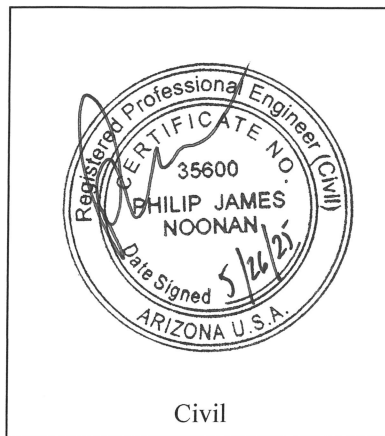
CITY MANAGER
JOSHUA WRIGHT

PUBLIC WORKS & UTILITIES DIRECTOR
JOHN KNUDSON, PE

CITY OF CHANDLER: Public Works and Utilities Department
PROJECT NAME: Loop 101 and 202 Freeway 66-inch Redundant Sewer Project
PROJECT NUMBER: WW2402.401

TECHNICAL SPECIFICATIONS

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FOR
CITY OF CHANDLER
LOOP 101 AND 202 FREEWAY 66-INCH REDUNDANT SEWER PROJECT
CITY PROJECT NO. WW2402.401
AGENCY REVIEW



DIVISION 1 GENERAL REQUIREMENTS

01110	SUMMARY OF WORK
01140	WORK RESTRICTIONS
01311	PROJECT COORDINATION
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02147 REMOTE CCTV INSPECTION OF EXISTING AND REHABILITATED
SEWERS
02148 CIPP LINING OF EXISTING SEWERS
02220 DEMOLITIONS
02230 CLEARING
02315 STRUCTURAL EXCAVATION AND BACKFILL
02532 POLYMER CONCRETE MANHOLE INSERT
02720 FIBER REINFORCED POLYMER

DIVISION 3 CONCRETE

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03200 CONCRETE REINFORCEMENT
03251 CONCRETE JOINTS
03252 ANCHORAGE IN CONCRETE
03300 CAST-IN-PLACE CONCRETE
03600 GROUT
03601 CELLULAR LOW DENSITY GROUT
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05581 FLOOR ACCESS HATCH COVERS

DIVISION 6 WOOD AND PLASTICS

06611 FIBERGLASS REINFORCED PLASTIC GRATINGS AND SUPPORT
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DIVISION 9 FINISHES

09910 SPECIAL FINISHES

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15108.1 CENTRIFUGALLY CAST, FIBERGLASS-REINFORCED
POLYMER MORTAR PIPE

+ + END OF TABLE OF CONTENTS + +

SECTION 01110

SUMMARY OF WORK

PART 1 - GENERAL

1.1 LOCATION AND DESCRIPTION OF WORK

- A. The Work is located near the Loop 101-202 freeway interchange in Chandler, Arizona. The scope of the project is to design and construct a redundant 66-inch diameter gravity sewer line that crosses the Loop 101 and 202 freeway and rehabilitate existing sewer line segments consisting of 48-inch diameter, 30-inch diameter and 66-inch diameter using cured-in-place pipe (CIPP) liner as shown per the Drawings (i.e. plans). In addition, rehabilitation of the Pecos Diversion Structure is required along with a new overflow sewer line as shown per the Drawings.
- B. The Scope of Work to be performed includes, but are not limited to the following, and reflect what is shown in the Drawings.
1. Coordinate with ADOT to obtain encroachment permits as required.
 2. Remove existing 48-inch existing manhole on the existing west 48-inch sewer line to construct new cast-in-place and polymer concrete pre-cast junction structures to divert all the west flows to the existing and new redundant 66-inch sewer line.
 3. Abandon in place by grout filling the existing junction chamber east of the 66-inch sewer.
 4. Abandon a portion of an existing 48-inch sewer line by grout filling the pipe and plugging the ends as shown per the Drawings.
 5. Rehabilitation and replacement of existing manholes.
 6. Installation of new manholes and manhole inserts.
 7. Installation of stop log mechanism, cranes and gratings as shown per the Drawings.
 8. Remove east 48-inch sewer line as shown per the Drawings for construction and add new pre-cast structure to divert east flows to existing and new redundant 66-inch sewer line.
 9. Install approximately new 66-inch diameter redundant gravity sewer line of which a portion will be routed within a steel casing as it crosses the Loop 202 freeway.
 10. CIPP line existing 30-inch diameter sewer crossing Loop 101 freeway.
 11. CIPP line existing 48-inch diameter sewer north of Loop 202 freeway.
 12. CIPP line existing 66-inch diameter sewer line north, south and crossing Loop 202 freeway.
 13. Connecting new portions of sewer to the existing sewer system as shown on the Drawings.

14. Bypassing sewage flows for the completion of the work as shown on the Drawings.

1.2 CONTRACT

- A. The Work shall be constructed under one prime contract.

1.3 SEQUENCE AND PROGRESS OF WORK

- A. CONTRACTOR'S construction schedule may use a different sequence from that shown or specified, if techniques and methods known will result in cost and time savings to the OWNER, still achieve the required objective and maintain the same or greater level of treatment. The ENGINEER'S determination on the acceptability of any alternative sequence from that shown or specified shall be final.

1.4 CONTRACTOR'S USE OF PREMISES

- A. CONTRACTOR shall coordinate use of the premises, for his storage and the operations of his workmen, with OWNER, ENGINEER and utility service companies.
- B. The full use of the premises for storage, the operations of workmen and for all other construction activities will not be available to CONTRACTOR. Must operate entirely within the space allowed to him.
- C. CONTRACTOR shall be solely responsibility for obtaining and paying all costs in connection with any additional work area, storage sites, access to the site, or temporary right-of-way which may be required for proper completion of the Work.
- D. It shall be understood that responsibility for protection and safe-keeping of equipment and materials on or near the site will be entirely that of CONTRACTOR and that no claim shall be made against the OWNER or his authorized representatives by reason of any act. It shall be further understood that should any occasion arise necessitating access to the sites occupied by these stored materials or equipment, the ENGINEER shall direct CONTRACTOR owning or responsible for the stored materials and equipment to immediately move the same.
- E. CONTRACTOR shall be required to share use of the premises with other contractors whose services the OWNER has obtained or will obtain for construction of other facilities on the site.

1.5 EASEMENTS AND RIGHT-OF-WAY

- A. Confine construction operations within the limits indicated on the Drawings. Use due care in placing construction tools, equipment, excavated materials, and pipeline

materials and supplies in order to avoid damage to property and interference with traffic, SRP, or ADOT. Do not enter any private property outside the designated construction easement boundaries without written permission from the ENGINEER and the Owner of the property. Any temporary construction easement for a private property or Rights-Of-Way owned by others than the OWNER, which CONTRACTOR wishes to utilize during the performance of the Work, shall be obtained by the CONTRACTOR.

- B. An encroachment permit must be obtained for any activities within the state right-of-way, including highways or ADOT easements. The CONTRACTOR shall obtain the necessary encroachment permit approval from ADOT prior to initiating any construction operations.

1.6 NOTICES TO OWNERS AND AUTHORITIES OF PROPERTIES ADJACENT TO THE WORK

- A. Notify OWNER of adjacent properties and utilities when prosecution of the Work may affect them.
- B. When it is necessary to temporarily obstruct access to property, or when any utility service connection must be interrupted, give notices sufficiently in advance to enable the affected persons to provide for their needs. Conform notices to any applicable local ordinance and, whether delivered orally or in writing, include appropriate information concerning the interruption and instructions on how to limit inconvenience caused thereby.
- C. Utilities and other concerned agencies shall be notified at least forty-eight (48) hours prior to cutting or closing streets or other traffic areas or excavating near underground utilities or pole lines.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01140

WORK RESTRICTIONS

PART 1 - GENERAL

1.1 USE OF PREMISES

- A. Limit use of premises to Work in areas indicated. Do not disturb portions of site beyond areas in which Work is indicated.
 - 1. Limits: Confine construction operations to designated areas as determined by OWNER, ADOT, SRP and/or as shown on the drawings. Confine storage of materials and support facilities to designated areas as shown on the drawings or as determined by the OWNER. Do not enter areas of the site where no Work is to be conducted.
 - 2. Driveways and Entrances: At all times, keep driveways and entrances serving premises clear and available to OWNER, OWNER'S employees, and emergency vehicles. Do not use these areas for parking or storage of materials.
 - a. Schedule deliveries to minimize use of driveways and entrances.
 - b. Schedule deliveries to minimize space and time requirements for on-site storage of materials and equipment.
- B. Promptly repair damage to premises caused by construction operations. Upon completion of the Work, restore premises to original condition.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

++ END OF SECTION ++

SECTION 01311

PROJECT COORDINATION

PART 1 - GENERAL

1.1 DESCRIPTION

- A. As more fully set forth in of the General Conditions, CONTRACTOR shall be solely responsible for coordination of all of the Work. The CONTRACTOR shall supervise, direct and cooperate fully with all subcontractors, manufacturers, fabricators, suppliers, distributors, installers, testing agencies and all others whose services, materials, or equipment are required to ensure completion of the Work within the Contract Time.
- B. As more fully set forth in of the General Conditions, the CONTRACTOR shall cooperate with and coordinate the Work with the work of any other contractor, utility service companies or OWNER'S employees performing work at the site.
- C. The CONTRACTOR shall not be responsible for damage done by contractors not under CONTRACTOR'S jurisdiction and shall not be liable for any such loss or damage, unless it is through the negligence of CONTRACTOR.
- D. The CONTRACTOR shall coordinate the Work with the work of others to assure compliance with schedules.
- E. The CONTRACTOR shall attend and participate in all project coordination or progress meetings and report on the progress of all Work and compliance with schedules.
- F. It is the duty of the CONTRACTOR to determine that all necessary permits have been obtained. The CONTRACTOR, at his own expense, obtain, maintain and close all the required permits which have not been furnished. The CONTRACTOR to have all copies of permits onsite for OWNER review.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01312

PROGRESS MEETINGS

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Date and Time:
 - 1. Regular Meetings: Weekly on a day and time agreeable to OWNER, ENGINEER and CONTRACTOR.
 - 2. Other Meetings: As needed and/or required in other specific specification sections.
- B. Place: CONTRACTOR'S field office at the project site, or other mutually agreed upon location.
- C. The ENGINEER shall conduct weekly progress meetings, the ENGINEER shall record and distribute minutes of the meeting to all attendees and others as requested. At a minimum, the agenda will include: Requests for Information (RFI) and submittal status, past week's progress and a 3-week look-ahead schedule to include upcoming inspections, current issues, long lead items, critical issues and the next scheduled meeting date.
- D. CONTRACTOR shall provide data required and be prepared to discuss all items on agenda.

1.2 MINIMUM ATTENDANCE

- A. CONTRACTOR:
 - 1. When needed for the discussion of a particular agenda item, CONTRACTOR shall require representatives of subcontractors or suppliers to attend a meeting.
- B. ENGINEER:
 - 1. Construction management staff shall attend weekly meetings.
 - 2. When needed for the discussion of a particular agenda item, ENGINEER shall require representative of sub-consultant to attend a meeting.
- C. Others, as appropriate.
- D. Representatives present for each party shall be authorized to act on their behalf.

1.3 AGENDA

- A. Agenda will include, but will not necessarily be limited to, the following:
1. Transcript of previous meeting
 2. Progress since last meeting;
 - a. CONTRACTOR'S three week schedule. Schedule shall show intended work three weeks ahead as well as completed work since the last progress meeting.
 - b. Subcontractors' three week schedule (may be combined with CONTRACTOR'S schedule).
 3. Completion status.
 4. Planned progress for next period including a 3-week look-ahead schedule to include upcoming inspections.
 5. Document and track to correction and closure any problems, conflicts, issues, and observations that are voiced by anyone of the project team.
 6. Status of Shop Drawings, RFI and RFAs.
 7. Change Orders.
 8. Pay Requests.
 9. Quality Standards and Control.
 10. Schedules, updated Project Schedules, including off-site fabrication and delivery schedules; corrective measures, if required.
 11. Coordination between parties.
 12. Permits.
 13. Safety concerns.
 14. Construction Photographs.
 15. Record Drawings.
 16. Warranty Requests.
 17. Punch List Status.
 18. Other business.
 19. Next meeting date.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01323

CONSTRUCTION PHOTOGRAPHS

PART 1 - GENERAL

1.1 DESCRIPTION

- A. CONTRACTOR shall retain a professional photographer or an acceptable person, as determined by the ENGINEER, to perform the services specified below.
- B. Obtain ENGINEER'S approval of the photographer selected prior to taking first photographs. Submit qualifications and experience record of photographer to ENGINEER.

1.2 PHOTOGRAPHS

- A. The minimum number of color digital photographs required will be 100 per month.
- B. Take a minimum of 200 color digital photographs of the completed or substantially completed Work at Project Completion. These photographs shall be submitted with the Final Application for Payment. These photographs are not part of the photographs required under Paragraph 1.2.A., above.
- C. The CONTRACTOR shall take color digital photographs daily of all underground work in progress. Photographs shall be taken in such a way to include an area or directional landmark. A minimum of twenty five overall general project photographs depicting the overall project activities shall be taken weekly. A photograph log shall be maintained throughout the project with the following information for each photo: photograph number, a brief description, date, and location or station of pipeline. The CONTRACTOR shall submit a plan that illustrates how the photograph log will be organized for approval by the OWNER.
- D. Color digital photographs shall be taken and downloaded weekly. Photos shall be grouped into folders and the folders shall be named with the date they were taken and a brief description of the photo subject. For example: '2016-01-31 Roadway Demolition.'
- E. ENGINEER will approve the views to be taken and select the time at which they will be taken. All photographs need to be viewable (digital), otherwise they will not be allowed off-site (refer to paragraph 1.4.A). Views will vary depending on the Progress Schedule.

- F. A minimum of 20 color digital photographs shall be submitted with the pay application during each pay period; see Section 1.3 below.

1.3 PHOTOGRAPH FORMAT

- A. Furnish all photographs in digital format.
- B. Provide all digital photographs on USB drive. The file format shall be “jpg”. The digital photographs shall be provided in addition to the standard photographs required under Paragraph 1.3.B, above. Provide a file for all required under paragraphs 1.2.A and B.
- C. Provide interior and exterior photographs of each buried structure prior to burial. Provide a minimum of four internal views and four external views of each structure. One view shall be provided of each wall, detail, floor and top of structure.
- D. Place the following information in the digital file name for each photograph:
1. Date photograph was taken.
 2. Description of view shown in photograph.
 3. Station Number

1.4 PRE-CONSTRUCTION PHOTOGRAPHS

- A. General
1. It is the CONTRACTOR’s responsibility to provide 150 pre-construction photographs and video, so as to resolve any disputes which may arise regarding the considerations prior to and subsequent to construction.
 2. If a dispute arises where no Pre-construction photographs were provided, the disputed area shall be restored to the extent directed by the ENGINEER and to the complete satisfaction of the ENGINEER.
 3. Prior to the start of any construction activities the CONTRACTOR shall furnish (2) sets of color pre-construction photographs and video for approval; one for the ENGINEER and one for the OWNER.
 4. Pre-construction photographs taken by CONTRACTOR will not be considered as part of the required number of construction photographs required in Paragraph 1.2, above.
 5. A high quality video of the site in digital format shall be made and submitted by the CONTRACTOR.
 6. Drone photos are required per the contract general requirements.

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PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01330

SUBMITTALS

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Submittal of documents described in the General Conditions and hereinafter are required prior to, during and at the end of the construction period. The submittals shall conform to the requirements described in this Section and all referenced Sections or Articles.
- B. The CONTRACTOR will set up and maintain a project portal for use as a centralized location for storing and conveying project information and for team communication. The portal shall be utilized by the OWNER, ENGINEER and CONTRACTOR for document tracking. The CONTRACTOR shall submit electronic copies of all submittals by uploading the electronic files to the portal.

1.2 GENERAL SUBMITTAL REQUIREMENTS

- A. A submittal shall be made for each complete system. Piecemeal submittals will not be accepted.
- B. Submittals requiring ENGINEER review only will be processed within 21 calendar days after receipt from CONTRACTOR. Submittals requiring ENGINEER and OWNER review will be processed within 30 calendar days after receipt from CONTRACTOR.
- C. CONTRACTOR shall maintain a file of all approved submittal documents at the work site.
- D. CONTRACTOR shall show his executed internal review and approval marking. Submittals which are received from sources other than through CONTRACTOR'S Office or which have not undergone CONTRACTOR review will be returned "Rejected".

1.3 PROCEDURE

- A. Submittals Prior to Beginning the Work: Refer to the General Conditions of the Contract Documents.
- B. Submittals During Construction: During progress of the construction, provide the following submittals in a timely manner to prevent any delay in the Work schedule:

1. Shop Drawings, Product Data and Samples: Submit Shop Drawings, product data and samples in accordance with Section 01332, Shop Drawing Procedures, and as required in various Sections of the Contract Documents.
2. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.
3. Request for Information: Submit a Request for Information when any of the following are required: an interpretation of the Specifications; additional details; information not shown on the Drawings or in the Specifications; or clarification of discrepancies, omissions, or errors is determined. Submit one electronic PDF format copy to the ENGINEER for response. Once the RFI is commented on by the ENGINEER, an electronic PDF format copy will be forwarded to the OWNER.
4. Change Orders: A proposal for a Change Order may be submitted by CONTRACTOR in accordance with the General Conditions. The Change Order Proposal must be in writing and must include sufficient information to assess the need for a change in the Work, the Contract time or the Contract amount. Any Work done by CONTRACTOR prior to execution of a Change Order is entirely at his own risk.
5. CONTRACTOR'S Daily Report: Submit electronic copies of CONTRACTOR'S Daily Report. CONTRACTOR and each subcontractor shall prepare and submit a daily report. The report shall contain, as a minimum, information on the location and description of the Work being performed, size, quantity and description of materials and equipment installed or delivered, coordination or scheduling concerns, requests for clarifications, and any discrepancies noted in the Contract Documents or on the as-built conditions. The report shall also contain CONTRACTOR'S daily workforce count by craft, general weather conditions, any Work performed other than during established working hours, and any other pertinent items relative to the Work, and as

required by ENGINEER. The report is due at the ENGINEER'S office by 9:00 a.m. on the following Work day and shall be signed by a responsible member of CONTRACTOR'S staff.

6. Submittal Schedule: Submit an updated Shop Drawing, Product Data and Sample Submittal Schedule with each Progress Payment Request.
 7. Construction Photographs: Submit Construction Photographs with each month's Progress Payment Request as specified in Section 01323, Construction Photographs.
 8. Operation and Maintenance Manuals and Lesson Plans: Submit Equipment Operation and Maintenance Manuals for approval, by the ENGINEER, within 30 days after approval of Equipment Shop Drawing. Submit Equipment Training Lesson Plans for approval, by the ENGINEER, 60 days prior to commencement of training. Submit Operation and Maintenance Data and Lesson Plans in accordance with Section 01781, Operation and Maintenance Data and Section 01821, Instruction of Operations and Maintenance Personnel.
- C. Submittal at Substantial Completion: Submit all Operations and Maintenance Data for each item of Work commissioned into operation.
- D. Submittal At Project Closeout: With a written Notice of Completion, submit the following items in the proper form as a condition of Final Acceptance of the Work:
1. Project Record Documents: Submit in accordance with Section 01782, Record Documents.
 2. Guarantees, Warranties and Bonds: Submit as required in the General Conditions and listed in various Sections of the Specifications.
 3. Survey notes.
 4. Construction photographs of all completed Work, in accordance with Section 01323, Construction Photographs.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

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APPENDIX A

Submittal Listing: The following is a list of anticipated submittals for the project:
This list does not alleviate Contractor from obligations of requirements of each individual Technical Specifications.

No.	Description	Specification
1.	Shop Drawing Schedule	General Conditions
2.	Shop Drawing Procedures	01332
3.	Storm Water Pollution Prevention Plan	01412
4.	Confined Space Entry Plan	01415
5.	Testing Laboratory Services Furnished by Owner	01451
6.	Testing Laboratory Services Furnished by CONTRACTOR	01452
7.	Testing of Hydraulic Structures	01453
8.	Project Identification Sign	General Conditions
9.	Field Engineering	01722
10.	Cutting and Patching	01723
11.	Record Documents	01782
12.	Diversion of Sewage Flow and Dewatering	02145
13.	Sewer Pipe and Sewer Structure Cleaning	02146
14.	Remote CCTV Inspection of Existing and Rehabilitated Sewers	02147
15.	CIPP Lining of Existing Sewers	02148
16.	Demolition Methods	02220

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No.	Description	Specification
17.	Clearing	02230
18.	Structural Excavation and Backfill	02315
19.	Polymer Concrete Manhole Insert	02532
20.	Fiber Reinforced Polymer	02720
21.	Samples/Concrete Formwork	03100
22.	Concrete Reinforcement	03200
23.	Concrete Joints	03251
24.	Anchorage in Concrete	03252
25.	Concrete Mix Design/Cast In Place Concrete	03300
26.	Grout Mix Design	03600
27.	Cellular Low Density Grout	03601
28.	Floor Access Hatch Covers	05581
29.	Fiberglass Reinforced Plastic Gratings and Support Framing Members	06611
30.	Fiberglass Reinforced Plastic Stop Logs	06614
31.	Special Finishes	09910
32.	Polymer Concrete Structures	13210
33.	PVC Liner	13220
34.	Piping Systems	15050
35.	Thermoplastic Pipe	15106
36.	Centrifugally Cast, Fiberglass-Reinforced Polymer Mortar Pipe (HOBAS)	15108.1

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+ + END OF SECTION + +

SECTION 01332

SHOP DRAWING PROCEDURES

PART 1 - GENERAL

1.1 DESCRIPTION

- A. The submittal of Shop Drawings shall conform to requirements of General Conditions and procedures described in this Section. A separate transmittal form shall be used for each specific item or class of material or equipment for which a submittal is required. Transmittal of Shop Drawings on various items using a single transmittal form shall be permitted only when the items taken together constitute a manufacturer's "package" or are so functionally related that expediency indicates review of the group or package as a whole.
- B. The term "Shop Drawings" as used herein shall be understood to include detailed design calculations, fabrication and installation drawings, lists, graphs, test data, operating instructions, and other items which shall include, but are not necessarily limited to:
1. Drawings and catalog information and cuts.
 2. Specifications, parts list, suggested spare parts lists, and equipment drawings.
 3. Wiring diagrams of systems and equipment, as required.
 4. Applicable certifications.
 5. Anchor bolt templates, mounting instructions and mounting design calculations as required.
 6. Required maintenance operations to allow all installed equipment to remain idle for a period of time not to exceed 24 months.
 7. Other technical, installation, and maintenance data as applicable.
 8. Unloading and handling methods and storage requirements.
 9. Note, highlight, and explain proposed changes to the Contract Documents.
 10. Paint submittal showing type of paint and the mils thickness of coating system used.
 11. Drawings showing CONTRACTOR field verifications illustrating all field dimensions. Field verify all dimensions and existing materials shown on the Drawings. Any modifications required shall be at CONTRACTOR'S expense.
 12. For materials or products which can contact drinking water as part of a Water Treatment Process or Water Supply System Certificate of Compliance with NSF/ANSI 61 Standard or with Arizona Administrative Code R18-4-119.
 13. Complete, detailed written preventive maintenance instructions as defined below.

- C. Preventive Maintenance Instructions:
1. The term "preventive maintenance instructions" includes all information and instructions required to keep a product or piece of equipment properly maintained according to the manufacturer's recommendation to fulfill the equipment warranty conditions and ensure the equipment or asset reaches its maximum potential life.
 2. Preventive maintenance instructions include, but are not limited to, the following:
 - a. A written explanation with illustrations for each preventive maintenance task.
 - b. Recommended schedule for execution of preventive maintenance tasks.
 - c. Lubrication charts.
 - d. Table of alternative lubricants.
 - e. Trouble shooting instructions.
 - f. List of required maintenance tools and equipment.
 - g. Special tools.
 3. The maintenance tasks frequency are driven by calendar, run-time, or meter reading.
- D. Preliminary Submittal Schedule: CONTRACTOR, within 14 days after the Notice-to-Proceed, shall prepare and submit to the ENGINEER a Preliminary Submittal Schedule. Identify on his Preliminary Submittal Schedule all of the submittal items required by the Contract Documents governing the Work.
- E. Submittal Schedule: CONTRACTOR, within 21 days after the Notice-to-Proceed, shall prepare and submit to the ENGINEER a comprehensive Submittal Schedule. Identify on his Submittal Schedule all of the submittal items required by the Contract Documents governing his Work. Indicate, for each submittal item of the Submittal Schedule the following:
1. The date by which that item will be submitted to the ENGINEER.
 2. The date by which response is required.
 3. The date by which the material or equipment must be on site in order not to delay the progress of the Work.
- F. In preparing his Submittal Schedule, consider the nature and complexity of each submittal item and shall allow ample time for review, revision or correction. Submittal will normally be returned to CONTRACTOR within 21 calendar days following receipt of the submittal.
- G. The ENGINEER will review CONTRACTOR'S Submittal Schedule to determine its completeness and compatibility with the Progress Schedule. A Submittal Schedule which is incompatible with the Progress Schedule or a review schedule which places extraordinary manpower demands on the ENGINEER will be sufficient reason(s) to reject the Submittal Schedule. It shall be understood that certain submittals will take

longer than 21 days to review and that these particular submittals will be identified during the review of the Submittal Schedule, by the ENGINEER to allow for very complex submittal reviews. Also, identify submittal for which he anticipates long review periods.

- H. Approval of the Submittal Schedule shall be required prior to processing of the first progress payment.

1.2 PROCEDURE

- A. Submit Shop Drawings to: Electronically through Procure as managed by the Contractor.
- B. A letter of transmittal shall accompany each submittal. If data for more than one Section of the Specifications is submitted, a separate transmittal letter shall accompany the data submitted for each Section.
- C. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.
- D. At the beginning of each letter of transmittal, provide a reference heading indicating the following:
1. OWNER'S Name: City of Chandler Public Works and Utility Department
 2. Project Name: LOOP 101 AND 202 FREEWAY 66-INCH REDUNDANT SEWER PROJECT
 3. City Project No. WW 2402.401
 4. Transmittal No. _____
 5. Section No. _____

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- E. If a Shop Drawing deviates from the requirements of the Contract Documents, specifically note each variation in his letter of transmittal.
- F. All Shop Drawings submitted for approval shall have a title block with complete identifying information satisfactory to ENGINEER.
- G. All Shop Drawings submitted shall bear the stamp of approval and signature of CONTRACTOR as evidence that they have been reviewed and verified to the completeness of the submittal by CONTRACTOR. Submittal without this stamp of approval will not be reviewed by ENGINEER and will be returned to CONTRACTOR. CONTRACTOR'S stamp contain the following minimum information:

Project Name: _____

CONTRACTOR'S Name: _____

Date: _____

-----Reference-----

Item: _____

Specifications:

Section: _____

Page No.: _____

Para. No.: _____

Drawing No.: _____ of _____

Location: _____

Submittal No.: _____

Approved By: _____

- H. In order to identify and track all submittals as separate and unique items, utilize the submittal identification numbering system as follows:

1. The Submittal Number shall be a separate and unique number correlating to each individual submittal that is required to be tracked as a separate and unique item. The Submittal Number shall be a two part, eight character, alpha/numeric number assigned by CONTRACTOR in the following manner:
 - a. The first part of the Submittal Number shall consist of five characters that pertain to the applicable Specification Section number.
 - b. The second part of the Submittal Number shall consist of three digits (numbers 001 to 999) to number each separate and unique submittal submitted under each Specification Section.
 - c. A dash shall separate the two parts of the Submittal Number.
 - d. A typical Submittal Number for the third Working Drawing submitted under Section 15101, Ductile Iron Pipe, would be 15101-003.
2. The Review Cycle shall be a three-digit number indicating the initial submission or resubmission of the same submittal. For example:
001 = First (initial) submission
002 = Second submission (first resubmission)
003 = Third submission (second resubmission)
3. An example of the typical submittal identification numbers for the first submission of the third submittal submitted under Section 15101, Ductile Iron Pipe is:

<u>Submittal Number</u>	<u>Review Cycle</u>
15101-003	001
4. An example of the typical submittal identification numbers for the second submission of the third submittal submitted under Section 15101, Ductile Iron Pipe is:

<u>Submittal Number</u>	<u>Review Cycle</u>
15101-003	002
- I. After ENGINEER completes his review, Shop Drawings will be affixed with a stamp and marked with one of the following notations:
 1. Approved.
 2. Approved as Corrected.
 3. Approved as Corrected, Resubmit.
 4. Revise and Resubmit.
 5. Not Approved.
 6. Not Reviewed.
 7. For Information Only.
- J. If a submittal is acceptable, the ENGINEER will mark it “Approved” or “Approved as Corrected” and will forward electronic copy in PDF format of the submittal to the

- OWNER for review and comment. The OWNERS review process will begin when all required copies of a specific submittal are received. After the OWNERS review is complete, the ENGINEERS and OWNERS comments will be combined and returned to CONTRACTOR electronically.
- K. Upon return of a submittal marked “Approved” or “Approved as Corrected”, CONTRACTOR may order, ship or fabricate the materials included on the submittal, provided it is in accordance with the corrections indicated.
- L. If a Shop Drawing marked “Approved as Corrected” has extensive corrections or corrections affecting other Shop Drawings or Work, ENGINEER may require that CONTRACTOR make the corrections indicated thereon and resubmit the Shop Drawings for record purposes. Such Shop Drawings will have the notation, “Approved as Corrected - Resubmit.” The corrected Shop Drawing shall be a pre-condition for payment for the work item of the Shop Drawing.
- M. If a submittal is unacceptable, an electronic copy will be returned to CONTRACTOR with one of the following notations:
1. “Revise and Resubmit”.
 2. “Not Approved”.
- N. Upon return of a submittal marked “Revise and Resubmit”, make the corrections indicated and repeat the initial approval procedure. The “Not Approved” notation is used to indicate material or equipment that is not acceptable. Upon return of a submittal so marked, repeat the initial approval procedure utilizing acceptable material or equipment.
- O. Any related Work performed or equipment installed without an “Approved” or “Approved as Corrected” Shop Drawing will be at the sole responsibility of CONTRACTOR.
- P. Shop Drawings shall be submitted well in advance of the need for the material or equipment for construction and with ample allowance for the time required to make delivery of material or equipment after data covering such is approved. Assume the risk for all materials or equipment which are fabricated or delivered prior to the approval of Shop Drawings. Materials or equipment will not be included in periodic progress payments until approval thereof has been obtained in the specified manner.
- Q. ENGINEER will review and process all submittals promptly; a reasonable time shall be allowed for this, for the Shop Drawings being revised and resubmitted, and for time required to return the approved Shop Drawings to CONTRACTOR.
- R. Responsibility belongs to CONTRACTOR to review submittals made by his suppliers and subcontractors before transmitting them to the ENGINEER to assure

- proper coordination of the Work and to determine that each submittal is in accordance with CONTRACTOR'S desires and that there is sufficient information about materials and equipment for ENGINEER to determine compliance with the Contract Documents. Incomplete or inadequate submittals will be returned for revision without review.
- S. Furnish required submittals with complete information and accuracy in order to achieve required approval of an item within one submittal. Backcharges for resubmittals that account for a number greater than 20 percent of the total number of first time submittals and will be backcharged for all third submittals. The number of first time submittals shall be equal to the number of submittals agreed to by ENGINEER and CONTRACTOR in accordance with Section 01330 Appendix A. All costs to ENGINEER involved with subsequent submittal of Shop Drawings, Samples or other items requiring approval will be backcharged to CONTRACTOR at the rate of 3.0 times direct technical labor cost by deducting such costs from payments due CONTRACTOR for Work completed. In the event that CONTRACTOR requests a substitution for a previously approved item, all of ENGINEER'S costs in the reviewing and approval of the substitution will be backcharged to CONTRACTOR, unless the need for such substitution is beyond the control of CONTRACTOR.
- T. The OWNER reserves the right to withhold monies, identified in the General Conditions, for Shop Drawing reviews beyond those described herein.
- U. The ENGINEER will implement, if requested by CONTRACTOR, one special Shop Drawing Review Meeting. The purpose of the meeting is to expedite Shop Drawing reviews for the equipment and materials required for the first document of the Work. Requirements of this Section will not be waived, but could be expedited.
- V. Mark each page of a submittal and each individual component submitted with the specification number, paragraph, and subparagraph. Arrange submittal information presentation to appear in the sequence in the Specification Section.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01412

STORMWATER POLLUTION PREVENTION PLAN AND PERMIT

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Comply with the terms and conditions of the Arizona Pollutant Discharge Elimination System (AZPDES) requirements under the Arizona Department of Environmental Quality (ADEQ) General Permit. Under provisions of that permit, CONTRACTOR is designated as permittee and responsible for providing necessary material and for taking appropriate measures to minimize pollutants in stormwater runoff from the Project. Obtain a DeMinimus discharge permit from ADEQ for any discharge that is to Waters of the U.S., and comply with the requirements of the permit.
- B. The Contract Price shall include all material, labor and other permits and incidental costs related to:
 - 1. Preparing, updating and revising the Stormwater Construction Pollution Prevention Plan (SWPPP).
 - 2. Installing and maintaining all structural and non-structural items chosen by CONTRACTOR to comply with the construction SWPPP.
 - 3. Clean-up and disposal costs associated with clean-up and repair following storm events or CONTRACTOR caused spills on the Project.
 - 4. Implementing and maintaining Best Management Practices to comply with the OWNER'S stormwater code.
 - 5. Preparing the Notice of Intent and Notice of Termination shall be covered by the AZPDES General Permit for Arizona.
 - 6. Obtain and comply with DeMinimus permit, if such permit is required.
- C. Coordinate the requirements under this Section with Section 02315, Structural Excavation and Backfill, permit requirements. All necessary SWPPP controls and practices must be implemented prior to commencement of any construction activity.

1.2 SUBMITTALS

- A. Submit, at least two days prior to the initial start of construction on the project; completed and signed Notice of Intent forms to the State of Arizona at the following addresses:
 - 1. Stormwater Program - Water Permits Section / NOI
Arizona Department of Environmental Quality
1110 West Washington, 5415B-3
Phoenix, AZ 85007

- B. Submit to the OWNER, no later than 14 days before submitting to the State agency the following:
1. Notice of Intent (NOI) to be covered by the AZPDES General Permit for Arizona, including certifications of signature.
 2. SWPPP for the Project, including certification of signature. Stormwater Plan shall include CONTRACTOR'S proposed temporary means for stormwater control during all phases of construction and include stormwater pumping/retention plans. This submittal shall be coordinated with CONTRACTOR'S Excavation Plan submittal, specified in Section 02315, Excavation and Backfill.
 3. A manual has been prepared by the Maricopa County Flood Control District to aid in CONTRACTOR'S preparation of the SWPPP. This manual, "Drainage Design Manual for Maricopa County Arizona, Volume III, Erosion Control" is available at the Flood Control District Office, 2801 West Durango Street, Phoenix, Arizona. The complete Construction General Permit is available from the ADEQ website at:
<https://www.azdeq.gov/swppp>
- C. Submit to the OWNER, as part of the Construction SWPPP a construction site inspection report that includes the following:
1. Inspection scope.
 2. Inspector qualifications.
 3. Observations of SWPPP non-compliance and corrective steps taken.
 4. Certificate of Compliance with SWPPP and the AZPDES General Permit for Stormwater Discharge in the event of no incidents. Reports shall be submitted each quarter, at a minimum, throughout the Contract duration.
- D. Submit to the OWNER, upon project completion the Notice of Termination (NOT) of coverage under AZPDES General Permit.
- E. CONTRACTOR shall submit electronic copy per Specification Section 01332, Shop Drawing Procedures. Text shall be in electronic ASCII format. Drawings and figures shall be in pdf ".pdf" format.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01413

CONTRACTOR'S HAZARDOUS MATERIALS MANAGEMENT PROGRAM

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Comply with all Federal, State, and local Laws and Regulations related to environmental protection and environmental safety including, but not limited, to the following:
 - 1. Title 29 Code of Federal Regulations Parts 1910, Occupational Safety and Health.
 - 2. Title 40 Code of Federal Regulations, Environmental Protections.
 - 3. Title 49 Code of Federal Regulations, Transportation.
 - 4. State Occupational Safety and Health Administration (OSHA).
 - 5. Arizona Department of Environmental Quality (ADEQ).
 - 6. Arizona Department of Water Resources (ADWR).
 - 7. Maricopa County Air Pollution Control Regulations.
- B. In order to ensure the OWNER that CONTRACTOR is complying with the intent of the regulations stated in Paragraph 1.1.A, above, as they relate to the on site use of hazardous materials, hazardous wastes and other substances similarly defined in those regulations, CONTRACTOR shall develop and maintain a CONTRACTOR'S Hazardous Materials Management Program that includes as a minimum, but is not limited to the requirements specified herein. The interests of the OWNER are that accidental spills, site contamination, and injury of personnel on the site are avoided. OWNER will not enforce suspected violations of the rules and regulations referenced in Paragraph 1.1.A, above, however the OWNER will notify CONTRACTOR of suspected violations. If in the opinion of the OWNER, CONTRACTOR fails to address the suspected violations in a timely and appropriate manner, OWNER will notify Federal, State, or local regulatory agencies, report the suspected violations to them, and request that they inspect CONTRACTOR'S operations. Any fines that may be levied against OWNER for violations committed on the site by CONTRACTOR, as well as any costs to OWNER associated with cleanup of materials, shall be reimbursed immediately by CONTRACTOR. All documents required by the program shall be made available to the OWNER'S Environmental Representative immediately, upon request.
- C. Responsibility for any hazardous waste, as defined in any of the above listed regulations, and those generated by the CONTRACTOR, belongs to CONTRACTOR. If CONTRACTOR is going to generate, or has generated, a substance that qualifies as a hazardous waste, must obtain an EPA identification

number, listing CONTRACTOR'S name and construction site address as the generator of the hazardous waste. Responsibility for the identification, analysis, profiling, transport and disposal of hazardous wastes generated, belongs to CONTRACTOR. The identification number can be obtained from the Arizona Department of Environmental Quality (ADEQ). This number shall be provided to the ENGINEER within 30 days after the Notice to Proceed, or before any hazardous materials are brought onto the site.

1.2 HAZARDOUS MATERIALS PROGRAM REQUIREMENTS

- A. Within the regulations listed in Paragraph 1.1.A, above, terms such as hazardous material, hazardous wastes, and similar terms have varying definitions. To dispel confusion regarding what materials fall under the Program Requirements and for the purposes of this Article, Hazardous Material is defined as "any material, whether solid, semi-solid, liquid, or gas, which, if not stored or used properly, may cause harm or injury to persons through inhalation, ingestion, absorption or injection, or which may negatively impact the environment through the use or discharge of the material on the ground, in the water (including groundwater), or to the air."
- B. All chemicals brought onto the site must be approved by OWNER. Prior to bringing any chemical on site, request approval from OWNER'S Environmental Representative for each chemical CONTRACTOR proposes to bring on site. At the time of request, OWNER'S Environmental Representative may request and receive from CONTRACTOR, specific information associated with each chemical. The specific information may include, but is not limited to, SDS, manufacture, vendor, container size(s), number of containers, minimum and maximum volume of material intended to be stored on site, as well a description to the process or procedures in which any requested chemical is to be used. OWNER, within 10 working days from receipt of the specific chemical information, will inform CONTRACTOR as to whether the chemical has been approved for use on site.
- C. CONTRACTOR shall maintain on site two notebooks containing (1) a chemical inventory, and (2) current (dated within the past two years) Safety Data Sheets for all materials being used on site, whether or not they are defined as a Hazardous Material in Paragraph 1.2.A, above. One notebook shall be kept in CONTRACTOR'S on-site office and the other shall be kept in a location specified by the OWNER'S Environmental Representative. These notebooks must be kept up-to-date as materials are brought onto and removed from the site. Copies of SDS sheets for chemicals removed from the site shall be provided to the OWNER'S Environmental Representative.
- D. CONTRACTOR shall develop an emergency/spill response plan, for each hazardous material or class/group of materials. As a minimum, the response plan must address the following:

1. Provide a description of equipment on site available to contain or respond to an emergency/spill of the material.
 2. Notification procedures.
 3. Response coordination procedures between CONTRACTOR, OWNER, and ENGINEER.
 4. Provide a Site Plan showing the location of stored hazardous materials and location of spill containment/response equipment.
 5. Provide a description of the hazardous material handling and spill response training provided to CONTRACTOR'S employees.
- E. CONTRACTOR shall in accordance with applicable Laws and Regulations, properly and safely store all hazardous materials, which shall include as a minimum, the following:
1. Have a designated storage site for hazardous materials that includes secondary containment. The site must include barriers to prevent vehicles from colliding with the storage containers and offer protection from environmental factors such as weather.
 2. Provide signage in accordance with applicable Laws and Regulations, clearly identifying the hazardous materials storage site.
 3. All hazardous materials containers must bear the applicable Hazard Diamonds.
- F. CONTRACTOR shall properly label all containers of consumable materials, whether or not they are classified as Hazardous Materials under this Section. The name of CONTRACTOR or subcontractor shall be stenciled on any container containing a hazardous material and on any container over five-gallon capacity containing a non-hazardous material. Any container must have a label clearly identifying the contents. If any such unlabeled containers are discovered on the site, the OWNER'S Environmental Representative will notify CONTRACTOR. Responsibility to remove such containers belongs to CONTRACTOR. Containers will be properly labeled or removed it from the site within one hour. Any containers that are filled from larger containers must also be properly labeled.
- G. OWNER encourages storage of hazardous materials off site until the materials are needed on site.
- H. CONTRACTOR shall provide all documentation required herein available immediately upon request of OWNER'S Environmental Representative. CONTRACTOR'S Safety Representative will meet at least monthly with OWNER'S Environmental Representative to review CONTRACTOR'S Hazardous Materials Management Program documents, procedures, and inspect the storage site and job site to ensure the requirements specified herein are being complied with. Also, provide OWNER'S Environmental Representative and the ENGINEER with copies of all permits obtained from environmental regulatory agencies.

CITY OF CHANDLER: Public Works and Utilities Department
PROJECT NAME: Loop 101 and 202 Freeway 66-inch Redundant Sewer Project
PROJECT NUMBER: WW2402.401

- I. Provide documentation to ENGINEER and OWNER's Environmental Representative that CONTRACTOR, subcontractors, or others hired by CONTRACTOR making deliveries of hazardous Materials (as defined in Title 49 CFR) to the site are in compliance with Title 49 CFR 172.800 – 172.804, which requires each person who offers for transportation in commerce or transports in commerce one or more of the following hazardous materials, as defined by Title 49 CFR, must develop and adhere to a security plan for hazardous materials that conforms to the requirements of this subpart.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

++ END OF SECTION ++

SECTION 01414

EARTHMOVING AND DUST CONTROL

PART 1 - GENERAL

1.1 DESCRIPTION

- A. CONTRACTOR shall obtain all earthmoving permits and any other permits required for earthmoving and dust generating operations related to the Work as required by the Maricopa County Air Pollution Control Regulations.
- B. CONTRACTOR shall not cause or allow any dust generating operation, earthmoving operation, use of property, or any other operation which causes fugitive dust emissions that exceed the 20 percent visible emission opacity limit in Rule 300 of Maricopa County's Air Pollution Control Regulations.
- C. If requested by the OWNER, ENGINEER, or Maricopa County representative, shall conduct opacity observations for visible emissions of fugitive dust in accordance with techniques specified in USEPA Reference Method 9.
- D. In addition to earthmoving permits, CONTRACTOR shall obtain an approved Dust Control Plan from Maricopa County. At a minimum, the Dust Control Plan shall include the following information:
 - 1. Name(s), address(es) and phone number(s) of the person(s) responsible for the preparation, submittal, and implementation of the Dust Control Plan and responsible for the dust generating operations.
 - 2. A site plan that describes the total area of land surface to be disturbed (in acres); the operations and activities to be performed on the site; actual and potential sources of fugitive dust emissions; and the delivery, transportation, and storage areas for the site (including types of materials stored and appropriate size of material stock piles).
 - 3. Description of the Reasonably Available Control Measures (RACM) to be applied during all periods of dust generating operations at all actual and potential sources of fugitive dust.
 - 4. Description of dust suppressants to be applied including product specifications; method, frequency, and intensity of application; type, number, and capacity of application equipment; and certifications related to the suppressant's appropriate and safe use.
 - 5. Description of specific surface treatment(s) or RACM used to control material track-out where unpaved or access points join paved surfaces.
 - 6. Description of at least one alternative RACM for each actual and potential fugitive dust source shall be designated as a contingency measure.

- E. CONTRACTOR shall post a copy of all earthmoving permits as well as the approved Dust Control Plan in a conspicuous location at the worksite and provide a copy of each to the ENGINEER.
- F. CONTRACTOR shall maintain a daily written log that records the actual application or implementation of the RACMS described in the approved Dust Control Plan. Maintain this written log and supporting documentation on site and shall make available for review on request by ENGINEER, OWNER, or Maricopa County representative. Retain copies of the Dust Control Plan, RACM implementation records, and all supporting documentations for a minimum of three years.
- G. CONTRACTOR, at a minimum, CONTRACTOR shall provide all necessary equipment and materials to apply sufficient dust suppressants (e.g., water, etc.), properly clean (sweep, etc.) all track-out areas, and provide adequate physical stabilizations (e.g., gravel, recycled asphalt, etc.) to meet all requirements of the earthmoving permit and approved Dust Control Plan. Use these methods to control fugitive dust generation from all CONTRACTOR operations on all CONTRACTOR areas including, but not limited to:
 - 1. Construction areas.
 - 2. Vehicle and equipment parking areas.
 - 3. Material storage areas.
 - 4. Office and trailer areas.
 - 5. Haul and access roadways.
 - 6. Track-out areas.
 - 7. All other areas where CONTRACTOR work, storing, or parking of vehicles, equipment, and materials.
- H. CONTRACTOR shall pay all fines issued to the OWNER by the USEPA, ADEQ, or Maricopa County due to violation of CONTRACTOR'S earthmoving permit and Dust Control Plan.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01415

CONFINED SPACE ENTRY PLAN

PART 1 - GENERAL

1.1 DESCRIPTION

- A. OWNER has determined that portions of the Work site may constitute “confined spaces” as defined in 29 CFR §1926.21(b)(2) and 1910.146. Accordingly, CONTRACTOR shall incorporate into its Safety Plan for the Work site appropriate measures to protect the health and safety of all persons on the Work site or who may be affected by the Work, including, without limitation thereby, employees and representatives of CONTRACTOR, any subcontractor, OWNER, or ENGINEER while they are present and engaged in the performance of their duties on the Work site.
- B. CONTRACTOR shall comply with all local, State and Federal rules and regulations related to the protection of persons working or entering into confined spaces including, but not limited to the following:
 - 1. 29 United States Code §654.
 - 2. Title 29 Code of Federal Regulations Parts 1910 and 1926, Occupational Safety and Health.
 - 3. Ariz. Rev. Stat. §23-403.
 - 4. City of Chandler Confined Space Program.
- C. To assure OWNER that CONTRACTOR is complying with the intent of the regulations stated in Paragraph 1.1.A, above, as they relate to the protection of all persons on the Work site, CONTRACTOR’S Safety Plan, at a minimum, respond to the following requirements as they relate to Work in confined spaces:
 - 1. Conducting a Site-specific hazard assessment to identify confined spaces that should be characterized as “Permit Required Confined Spaces” within the meaning of 29 CFR §1926.21 (b)(6)(i) and 29 CFR §1910.146.
 - 2. Adopting as an element of its Safety Plan appropriate requirements for safeguarding access to “Permit Required Confined Spaces.”
 - 3. Providing training, personal protective or safety equipment and personnel as needed to perform the Safety Plan’s requirements for “Permit Required Confined Spaces.”
 - 4. Performing all record-keeping required for “Permit Required Confined Spaces”, including the required permits and confined space data sheets located in Section 01331, Reference Forms.

1.2 CONFINED SPACES SAFETY PLAN REQUIREMENTS

- A. For purposes of the Safety Plan requirements listed in Article 1.1, above, “confined spaces” are those areas on or about the Work site that fall within OSHA’s definition as “any space having limited means of egress, which is subject to the accumulation of toxic or flammable contaminants or has an oxygen deficient atmosphere. Confined or enclosed spaces include, but are not limited to, storage tanks, process vessels, bins, boilers, ventilation or exhaust ducts, sewers, underground utility vaults, tunnels, pipelines, and open top spaces more than four feet in depth such as pits, tubs, vaults, and vessels.”
- B. CONTRACTOR shall ensure that those persons who are required to enter a confined space are trained according to OSHA requirements set forth in 29 CFR §1926.21 (b)(6)(i).
- C. If the confined space is a “Permit Required Confined Space”, then CONTRACTOR shall comply with the standards set forth in 29 CFR §1910.146. and the City of Chandler, Confined Space Program.
- D. “Permit Required Confined Space” means a confined space that has one or more of the following characteristics:
 - 1. Contains or has the potential to contain a hazardous atmosphere.
 - 2. Contains a material that has the potential for engulfing an entrant.
 - 3. Has an internal configuration such that an entrant could be trapped or asphyxiated by inwardly converging walls or floors, or by a floor that slopes downward and tapers to a smaller cross-section.
 - 4. Contains any other recognized serious safety or health hazard.

1.3 SUBMITTALS

- A. CONTRACTOR shall prepare and submit a site-specific Confined Space Entry Plan as a portion of the CONTRACTOR’S site-specific Health and Safety Plan
- B. CONTRACTOR shall submit soft copy per Specification Section 01332, Shop Drawing Procedures. Soft copies shall be in pdf format.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01416

SPECIAL INSPECTIONS

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes:
 - 1. This Section describes the requirements for providing special tests and inspections required by the International Building Code.
- B. Purpose:
 - 1. This Document was developed to address the requirements of the 2024 International Building Code IBC, chapter 17.
 - 2. One or more special inspectors will be hired by the OWNER or the OWNER'S Agent to provide inspections during construction on the types of work stated herein.
 - 3. When required, a Certificate of Special Inspections will be submitted to the Building Code Official as a condition for permit issuance. This specification includes a complete list of materials and work requiring special inspections, the inspections to be performed and a list of the minimum qualifications of the individuals, approved agencies or firms intended to be retained for conducting such inspections.

1.2 DEFINITIONS

- A. Special Inspector: Representative of the OWNER approved inspection agency designated for that portion of the work.
- B. Testing Agency: Approved agency, not affiliated or hired by the CONTRACTOR, which is responsible for the materials testing requirements of the project including but not limited to concrete cylinder breaks, soils testing, and masonry materials testing.
- C. Soils Engineer or Geotechnical Engineer: For the purposes of Special Inspection "Soils Engineer," "Geotechnical Engineering," and "Special Inspector" shall be interchangeable as pertains to the Division 02 specifications.
- D. NICET: National Institute for Certification in Engineering Technologies.
- E. RPSI: Registered Professional for Special Inspections.

1.3 CONTRACTOR'S RESPONSIBILITIES

- A. Cooperate with testing agency personnel, special inspector, and agents of the Building Code Official and provide access to the work.
 - 1. Providing access to the work shall include all labor and facilities to perform inspections and tests as listed in the specifications for the duration of the inspections or tests involved.
 - 2. Provide means to obtain and handle samples taken on site.
- B. Attend a pre-construction meeting to coordinate and clarify inspection and testing procedures, requirements.
- C. Notify special inspector and/or testing agency of work to be inspected/tested minimum of 48 hours prior.
- D. Work for which special inspections are required shall remain accessible and exposed for the purposes of special inspections until completion of required special inspections.
- E. Any portion of work that is not in conformance shall be corrected and re-inspected. Such portions of the work shall not be covered or concealed until authorized by ENGINEER.
- F. Work to be inspected should be complete at time of inspector's arrival on-site.
- G. Payment for Special Inspection services will be in accordance with the following:
 - 1. Payment described below is for the Testing Agency and Special Inspector costs and does not include the CONTRACTOR's costs listed in Paragraph 1.3 A.
 - 2. After CONTRACTOR notification, inspector arrives at site and performs inspection within the timeframe defined in Item 4 below.
 - a. Inspection reveals work is satisfactory.
 - b. OWNER pays all costs associated with this inspection.
 - 3. After CONTRACTOR notification, inspector arrives at site and performs inspection within the timeframe defined in Item 4 below.
 - a. Inspection reveals work is deficient.
 - b. CONTRACTOR corrects deficiencies within timeframe defined in Item 4 below.
 - c. Work is re-inspected and work is satisfactory.
 - d. OWNER pays all costs associated with this inspection.
 - 4. After CONTRACTOR notification, inspector arrives at site and work is not ready for inspection when inspector arrives.
 - a. Inspector will remain on-site for a maximum of 2 hours awaiting the completion of the work.

- b. If work is not ready for inspection at the end of this period, inspector will be dismissed until CONTRACTOR requests re-inspection.
 - c. All costs associated with this inspection trip will be charged to the CONTRACTOR.
 5. After CONTRACTOR notification, inspector arrives at site and performs inspection within the timeframe defined above.
 - a. Inspection reveals work is deficient.
 - b. CONTRACTOR attempts to correct deficiencies within 2 HR timeframe and calls for re-inspection.
 - c. Work is re-inspected and found to still be deficient.
 - d. Inspector will be dismissed.
 - e. All costs associated with this inspection trip will be charged to the CONTRACTOR.
 6. OWNER will pay for "passing" soils on the Project. Costs of corrective actions and cost of failed test areas requiring retesting are the sole responsibility of the CONTRACTOR.
- H. Special Inspection is intended to be an independent quality assurance. Special Inspections shall not relieve the CONTRACTOR of any quality assurance, quality control, workmanship, or warranty responsibilities. CONTRACTOR's own personnel shall review all work to be inspected for conformance with Contract Documents prior to calling for inspection.

1.4 REPORTING DUTIES AND AUTHORITY

- A. The meeting is to be attended by:
 1. OWNER.
 2. ENGINEER.
 3. Building Code Official or designee.
 4. Testing Agency and Special Inspectors.
 5. General Contractor.
 6. Appropriate Sub-contractor(s).
- B. Reporting requirements for special inspector per IBC 2024.
 1. Comply with requirements of IBC Section 1704.2.4.
 2. Provide written documentation of all inspections and testing.
 - a. Include exact location of work.
 - b. If testing of specimens is included, include detailed information on storage and curing of specimens prior to testing.
 3. Furnish inspection and test reports to the CONTRACTOR, the ENGINEER'S Project Manager and the OWNER'S on-site representative.
 - a. Indicate that work inspected was done in conformance with approved construction documents.

- b. Immediately report any discrepancies to the CONTRACTOR for correction.
 - 1) Deficient work that has been covered up or concealed prior to re-inspection shall be reported to the ENGINEER and the Building Code Official.
 - c. If the discrepancies are not corrected in a timely fashion, notify the ENGINEER's Project Manager and OWNER'S on-site representative.
 - 4. Issue an electronic report summarizing all inspections, corrective action notifications and resolution of discrepancies and non-conforming work every two weeks (14 calendar days).
 - a. Copy will be available to:
 - 1) ENGINEER's Project Manager.
 - 2) OWNER.
 - 3) The Building Code Official.
 - 4) General Contractor.
 - 5. At the end of the Project, the RPSI shall compile all test reports for each inspected material and for each Special Inspector and summarize into a single PDF and submit to the ENGINEER and Building Code Official.
 - a. Final summary report to be signed and sealed by a Registered Professional for Special Inspections stating:
 - 1) The required Special Inspections have been performed.
 - 2) All discrepancies have been resolved except as specifically stated in the summary report.
- C. Special Inspector does not have authority to stop work or modify the requirements of the Contract Documents.

1.5 QUALIFICATIONS

- A. Qualifications stated here are the minimum recommended by the ENGINEER. If the Building Code Official has more stringent qualifications, the more stringent qualifications will take precedence.
- B. All Special Inspections and Testing to be done under the direction of a Professional ENGINEER registered in the State of Arizona herein referred to as Registered Professional for Special Inspections (RPSI).
- C. Soil related testing.
 - 1. The Testing Agency shall have a minimum of 10 years' experience in the testing of these materials.
 - 2. The Testing Agency's technician(s) conducting this testing:
 - a. Shall have a minimum of five years' experience in the testing of soil, concrete, mortar, grout, steel and aluminum as appropriate.
 - 3. Concrete related work:

- a. International Code Council certification for Reinforced Concrete and American Concrete Institute Concrete Field-Testing Technician – Grade 1.

D. Special Structural Inspections:

1. Professional Engineers, licensed in the State of Arizona, may perform special inspections in accordance with their license qualifications.
2. Other individuals, working under the direct supervision of a licensed engineer and meeting the following qualifications, may perform special inspections.
3. Soils related work:
 - a. NICET Level II Certification in geotechnical engineering technology/construction; or
 - b. Registered Geologist; or
4. Concrete related work:
 - a. International Code Council certification for Reinforced Concrete Special Inspector or American Concrete Institute Concrete Construction Special Inspector.
5. Precast concrete erection related work:
 - a. Professional Engineers, licensed in the State of Arizona.
6. Precast concrete erection welding:
 - a. American Welding Society as a Certified Welding Inspector; or
 - b. International Code Council Structural Steel and Welding Certification and American Welding Society Qualified and one year of related experience; or
 - c. NDT Level II or II Certificate (for non-destructive testing only).
7. Steel and aluminum related work:
 - a. Frame and material verification (Appendix E, Items 3 and 6):
 - 1) International Code Council Structural Steel and Welding Certification and one-year related experience.
 - b. Welding:
 - 1) American Welding Society as a Certified Welding Inspector; or
 - 2) International Code Council Structural Steel and Welding Certification and American Welding Society Qualified and one year of related experience; or
 - 3) NDT Level II or II Certificate (for non-destructive testing only).
8. Other equivalent certifications will not be acceptable unless approved by the Engineer.

1.6 TESTS

- A. Selection of the material required to be tested shall be by the OWNER'S Testing Laboratory and not the CONTRACTOR.

1.7 SPECIAL TESTING AND INSPECTIONS

- A. Testing laboratory: Special tests will be performed by the OWNER'S testing laboratory as specified in Section 01451.
- B. The following types of work require special inspection as described in IBC. Refer to the following verification, testing, and inspection schedules.
 - 1. Appendix A, Cast-In-Place Concrete Special Inspection Schedule.
 - 2. Appendix D, Soils Verification and Inspection Schedule.
 - 3. Appendix E, Structural Steel Special Inspection Schedule.
- C. The following Work also requires special inspection per the Building Code requirements:
 - 1. Special grading, excavating, and filling, as indicated on the Drawings and Specifications.

PART 2 - PRODUCTS

PART 3 - EXECUTION

APPENDIX A
CAST-IN-PLACE CONCRETE SPECIAL INSPECTION SCHEDULE

Verification and Inspection	Reference Standard	Frequency of Inspection	
		Continuous During Task Listed	Periodic During Task Listed
1. Inspection of reinforcing steel, including prestressing tendons, and placement.	ACI 318: Ch. 20, 25.2, 25.3, 26.6.1-26.6.3	—	X
2. Inspect bolts to be installed in concrete prior to placement of concrete.	ACI 318: 17.8.2	X	—
3. Inspection of post-installed mechanical and adhesive anchors into hardened concrete (downward inclined orientations).	ACI 318: 17.8.2	—	X
4. Inspection of post-installed adhesive anchors into hardened concrete (horizontal or upward inclined orientations).	ACI 318: 17.8.2.4	X	—
5. Verifying use of required design mix.	ACI 318: Ch. 19, 26.4.3, 26.4.4	—	X
6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	ASTM C172, ASTM C31, ACI 318: 26.5, 26.12	X	—
7. Inspection of concrete placement for proper application techniques.	ACI 318: 26.5	X	—
8. Inspection for maintenance of specified curing temperature and techniques.	ACI 318: 26.5.3-26.5.5	—	X
9. Inspection of formwork for shape, location, and dimensions of the concrete member being formed.	ACI 318: 26.11.1.2(b)	—	X
10. Verification of in-situ concrete strength, prior to removal of shores and forms from beams and structural slabs.	ACI 318: 26.11.2	—	X

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APPENDIX D
SOILS VERIFICATION AND INSPECTION SCHEDULE

Verification and Inspection	Reference Standard	Frequency of Inspection	
		Continuous During Task Listed	Periodic During Task Listed
1. Verify materials below footings are adequate to achieve the design bearing capacity.	IBC 1705.6	—	X
2. Verify excavations are extended to proper depth and have reached proper material.	IBC 1705.6	—	X
3. Perform classification and testing of controlled fill materials.	IBC 1705.6	—	X
4. Verify use of proper materials, densities, and lift thicknesses during placement and compaction of controlled fill.	IBC 1705.6	X	—
5. Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly.	IBC 1705.6	—	X

APPENDIX E
STRUCTURAL STEEL SPECIAL INSPECTION SCHEDULE

Verification and Inspection	Reference Standard	Frequency of Inspection	
		Continuous During Task Listed	Periodic During Task Listed
1. Material verification of structural steel:			
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	AISC 360: Chapter N	—	X
b. Manufacturers' certified mill test reports.	AISC 360: Chapter N	X	-
2. Material verification of weld filler materials:			
a. Identification markings to conform to AWS specification in the approved construction documents.	AISC 360: Table N5.4-1	—	X
b. Manufacturer's certificate of compliance required.	AISC 360: Table N5.4-1	—	X
3. Inspection of welding:			
a. Structural steel:		—	—
1) Complete and partial penetration groove welds.	AISC 360: Table N5.4-1, N5.4-2, N5.4-3	X	—
2) Multi-pass fillet welds.	AISC 360: Table N5.4-1, N5.4-2, N5.4-3	X	—
3) Single-pass fillet welds > 5/16".	AISC 360: Table N5.4-1, N5.4-2, N5.4-3	X	—
4) Single-pass fillet welds < 5/16".	AISC 360: Table N5.4-1, N5.4-2, N5.4-3	—	X
5) Floor and deck welds.	AISC 360: Table N5.4-1, N5.4-2, N5.4-3	—	X
b. Reinforcing steel:		—	—

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Verification and Inspection	Reference Standard	Frequency of Inspection	
		Continuous During Task Listed	Periodic During Task Listed
1) Verification of weldability of reinforcing steel other than ASTM A 706.	AISC 360: Table N5.4-1, N5.4-2, N5.4-3	—	X
2) "Form Saver" (reinforcing couplers).	AISC 360: Table N5.4-1, N5.4-2, N5.4-3	X	—

++ END OF SECTION ++

SECTION 01420

REFERENCES

PART 1 - GENERAL

1.1 DEFINITIONS

- A. General: Definitions of basic Contract terms are included in the General Conditions.
- B. Definitions of terms commonly found in the Specifications are as follows:
 - 1. Indicated: The term indicated refers to graphic representations, notes, or schedules on the Drawings, or to other paragraphs or schedules in the Specifications and similar locations in the other Contract Documents. Terms such as “shown”, “noted”, “scheduled”, and “specified” are used to help the user locate the reference. There is no limitation on the location.
 - 2. Installer (or applicator, or erector): An installer is CONTRACTOR or another entity engaged by CONTRACTOR, either as an employee or subcontractor to perform a particular construction activity, including installation, erection, application or similar operations. Installers are required to be experienced in the operations they are engaged to perform.
 - a. The term “experienced”, when used with the term “installer”, means having successfully completed a minimum of five previous projects similar in size and scope to this Project; being familiar with the special requirements indicated; and having complied with the requirements of authorities having jurisdiction and of the Supplier of the product being installed.
 - 3. Trades: Use of a term such as “carpentry” does not imply that certain construction activities must be performed by accredited or unionized individuals of a corresponding generic name, such as “carpenter”. It also does not imply that requirements specified apply exclusively to trades persons of the corresponding generic name.
 - 4. Assigned Specialists: Certain Sections of the Specifications require that specific construction activities shall be performed by specialists who are recognized experts in those operations. Said specialists shall be engaged for those activities, and their engagement is a requirement over which CONTRACTOR has no option. These requirements shall not be interpreted to conflict with the enforcement of building codes and similar regulations governing the Work. Also, they are not intended to interfere with local trade-union jurisdictional settlements and similar conventions. Such assignments shall not relieve CONTRACTOR of its responsibility for fulfilling the requirements of the Contract Documents.

1.2 ABBREVIATIONS

A. Common abbreviations, which may be found in the Specifications, are:

alternating current	AC
Ampere	A
ante meridiem	am
Average	avg.
biochemical oxygen demand	BOD
brake horsepower	BHP
British thermal unit	BTU
Centigrade	C
Company	Co.
cubic inch	cu. in.
cubic foot	cu. ft.
cubic yard	cu. yd.
cubic feet per minute	cfm
cubic feet per second	cfs
Decibel	DB
degree Centigrade (or Celsius)	(Say) 20°C

degree Fahrenheit	(Say) 68°F
Diameter	dia.
direct current	DC
Dollars	\$
Each	ea
Efficiency	eff
Fahrenheit	F
feet per hour	fph
Feet	ft.
feet per minute	fpm
feet per second	fps
Figure	Fig.
Flange	flg
foot-pound	ft-lb
gallon	gal
gallons per minute	gpm
gallons per second	gps

gram	g
Hertz	Hz
horsepower	hp
hour	hr
inch	in.
inch-pound	in.-lb
inside diameter	id
kilovolt-ampere	kva
kilowatt	KW
kilowatt-hour	kwhr
linear foot	lin. ft.
liter	l
maximum	max.
mercury	Hg
milligram	mg
milligrams per liter	mg/l
milliliter	ml

millimeter	mm
million gallons per day	mgd
million gallon	mil
minimum	min.
National Pipe Threads	NPT
net positive suction head	npsH
number	No.
ounce	oz
outside diameter	OD
parts per million	ppm
post meridiem	pm
pound	lb
pounds per square inch	psi
pounds per square inch absolute	psia
pounds per square inch gage	psig
pounds per square foot	psf
revolutions per minute	rpm
second	sec.

specific gravity	sp gr
square	sq
square foot	sq ft
square inch	sq in
square yard	sq yd
standard	std
standard cubic feet per minute	scfm
total dynamic head	tdh
totally-enclosed-fan-cooled	tefc
volt	V

1.3 APPLICABLE CODES

- A. When a reference standard is specified, comply with requirements and recommendations stated in that standard, except when they are modified by the Contract Documents, or when applicable laws, ordinances, rules, regulations or codes establish stricter standards. The latest provisions of applicable standards shall apply to the Work, unless otherwise specified. Reference standards include, but are not necessarily limited to, the following:
1. American Association of State Highway and Transportation Officials (AASHTO).
 2. American Concrete Institute (ACI).
 3. American Gear Manufacturers Association (AGMA).
 4. American Institute of Steel Construction (AISC).
 5. American Iron and Steel Institute (AISI).
 6. American National Standards Institute (ANSI).
 7. American Society of Heating, Refrigerating and Air Conditioning Engineers (ASHRAE).
 8. American Society of Mechanical Engineers (ASME).
 9. American Society for Testing and Materials (ASTM).

10. American Water Works Association (AWWA).
11. American Welding Society (AWS).
12. Arizona Administrative Code (AAC).
13. Concrete Reinforcing Steel Institute (CRSI).
14. Factory Mutual (FM).
15. Institute of Electrical and Electronics Engineers (IEEE).
16. National Electrical Manufacturer's Association (NEMA).
17. National Electrical Code (NEC) current adoption..
18. National Fire Protection Association (NFPA).
19. Occupational Safety and Health Administration (OSHA).
20. Prestressed Concrete Institute (PCI).
21. American Petroleum Institute (API)
22. Underwriters' Laboratories, Inc. (UL).
23. All other applicable standards listed in the Specifications and the standards of utility service companies, where applicable.
24. Maricopa Association of Governments (MAG), Uniform Standard Specifications for Public Works Construction, as supplemented by the City of Chandler. References to MAG Standard Details refer to the "Uniform Standard Details for Public Works Construction" sponsored and distributed by the Maricopa Association of Governments 2025, Arizona.
25. International Building Code (2024)
26. National Electric Code – NFPA70 (2023)
27. International Fuel Gas Code (2024).
28. International Fire Code (2024)
29. International Energy Conservation Code (2024)
30. International Mechanical Code (2024)
31. International Plumbing Code (2024)
32. The Code of the City of Chandler, Arizona
33. National Sanitation Foundation (NSF-61) and Arizona Administration Code (AAC # 18-4-213).
 - a. Incorporate the requirements NSF-61, Drinking Water System Components Health Effects and AAC # 18-4-213, Standards for Additives, Materials and Equipment on all potable water systems, water treatment facilities and water distribution facilities.

1.4 OWNER'S REFERENCE SPECIFICATIONS

- A. The CONTRACTOR shall maintain a complete copy of the Reference Specifications on the site.

1.5 INDUSTRY STANDARDS

- A. Applicability of Standards: Except where the Contract Documents include more stringent requirements, applicable construction industry standards have the same force and effect as if bound or copied directly into the Contract Documents to the

- extent referenced. Such standards are made a part of the Contract Documents by reference.
- B. Publication Dates: For applicable publication dates, refer to General Conditions.
 - C. Conflicting Requirements: Where compliance with two or more standards is specified and the standards establish different or conflicting requirements for minimum quantities or quality levels, refer to ENGINEER for a decision before proceeding.
 - D. Copies of Standards: Each entity engaged in construction on the Project is required to be familiar with industry standards applicable to its construction activity. Copies of applicable standards are not bound with the Contract Documents. Where standards are required to perform a required construction activity, obtain copies of same from the publication source.
 - E. Abbreviations and Names: Whenever in these Specifications or the other Contract Documents references are made to the standards, specifications, or other published data of international, national, regional or local organizations, such organizations may be referred to by their acronym or abbreviation only. The following acronyms or abbreviations, which may appear in the Specifications, shall have the meanings indicated herein.

1.	AA	Aluminium Association
2.	AABC	Associated Air Balance Council
3.	AAMA	American Architectural Manufacturers Association
4.	AASHTO	American Association of State Highway and Transportation Officials
5.	ACI	American Concrete Institute
6.	ACS	American Chemical Society
7.	AFBMA	Anti-Friction Bearing Manufacturers' Association
8.	AGMA	American Gear Manufacturers Association
9.	AI	Asphalt Institute
10.	AICHE	American Institute of Chemical Engineers
11.	AISC	American Institute of Steel Construction
12.	AISI	American Iron and Steel Institute
13.	AITC	American Institute of Timber Construction
14.	ALS	American Lumber Standards
15.	AMA	Acoustical Materials Association
16.	AMCA	Air Movement and Control Association
17.	ANSI	American National Standards Institute
18.	APA	American Plywood Association
19.	API	American Petroleum Institute
20.	APHA	American Public Health Association
21.	AREA	American Railway Engineering Association
22.	ARI	Air Conditioning and Refrigeration Institute
23.	ASA	American Standards Association
24.	ASAE	American Society of Agricultural Engineers
25.	ASTM	American Society for Testing and Materials
26.	ASCE	American Society of Civil Engineers

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27.	ASHRAE	American Society of Heating, Refrigerating and Air Conditioning
28.	ASME	American Society of Mechanical Engineers
29.	AWI	Architectural Woodwork Institute
30.	AWPA	American Wood Preservers' Association
31.	AWPB	American Wood Preservers Bureau
32.	AWPI	American Wood Preservers' Institute
33.	AWS	American Welding Society
34.	WWA	American Water Works Associations
35.	BHMA	Builders Hardware Manufacturers' Association
36.	CBMA	Certified Ballast Manufacturers' Association
37.	CDA	Copper Development Association
38.	CGA	Compressed Gas Association
39.	CISPI	Cast Iron Soil Pipe Institute
40.	CMAA	Crane Manufacturers' Association of America
41.	CRSI	Concrete Reinforcing Steel Institute
42.	EPA	Environmental Protection Agency
43.	ETL	Engineering Test Laboratories
44.	FCC	Federal Communications Commission
45.	FEMA	Federal Emergency Management Agency
46.	FGMA	Flat Glass Marketing Association
47.	FM	Factory Mutual Association
48.	FS	Federal Specification
49.	GA	Gypsum Association
50.	HEW	Department of Health, Education and Welfare
51.	HI	Hydraulic Institute
54.	ICBO	International Conference of Building Officials
55.	ICEA	Insulated Cable Engineers' Association
56.	IEEE	Institute of Electrical and Electronic Engineers

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57.	IES	Illuminating Engineering Society
58.	IFI	Industrial Fasteners Institute
59.	IRI	Industrial Risk Insurers
60.	ISA	The Instrumentation Systems and Automation Society
61.	ISO	Insurance Services Office
62.	MAG	Maricopa Association of Governments
63.	MIA	Marble Institute of America
64.	MS	Military Specifications
65.	MMA	Monorail Manufacturers' Association
66.	NAAMM	National Association of Architectural Metal Manufacturers
67.	NACE	National Association of Corrosion Engineers
68.	NARUC	National Association of Railroad and Utilities Commissioners
69.	NBHA	National Builders Hardware Association
70.	NEC	National Electrical Code
71.	NEMA	National Electrical Manufacturers Association
72.	NESC	National Electrical Safety Code
73.	NFPA	National Fire Protection Association
74.	NHLA	National Hardwood Lumber Association
75.	NHPMA	Northern Hardwood and Pine Manufacturer's Association
76.	NLMA	National Lumber Manufacturers' Association
77.	NRCA	National Roofing Contractors Association
78.	NSF	National Sanitation Foundation
79.	NTMA	National Terrazzo and Mosaic Association
80.	NWWDA	National Wood Window and Door Association
81.	OECI	Overhead Electrical Crane Institute
82.	OSHA	Occupational Safety and Health Administration
83.	PCI	Precast Concrete Institute
84.	PEI	Porcelain Enamel Institute

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85.	PPI	Plastic Pipe Institute
86.	PS	Product Standards Section-U.S. Department of Commerce
87.	RMA	Rubber Manufacturers' Association
88.	SAE	Society of Automotive Engineers
89.	SCPRF	Structural Clay Products Research Foundation
90.	SDI	Steel Deck Institute
91.	SDI	Steel Door Institute
92.	SIGMA	Sealed Insulating Glass Manufacturing Association
93.	SJI	Steel Joist Institute
94.	SMACNA	Sheet Metal and Air Conditioning National Association
95.	SPI	Society of the Plastics Industry
96.	SSPC	The Society for Protective Coatings
97.	SWI	Steel Window Institute
98.	TEMA	Tubular Exchanger Manufacturers' Association
99.	TCA	Tile Council of America
100.	UL	Underwriters' Laboratories, Inc.
101.	USGS	United States Geological Survey
102.	USPHS	United States Public Health Service
103.	WCLIB	West Coast Lumber Inspection Bureau
104.	WWEMA	Water and Wastewater Equipment Manufacturers Association

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01451

TESTING LABORATORY SERVICES FURNISHED BY OWNER

PART 1 - GENERAL

1.1 DESCRIPTION

- A. The OWNER will, through the ENGINEER, employ and pay for an independent testing laboratory to perform the specified services specified in the individual specification section.
- B. The OWNER will pay for testing as required in the individual specification sections of the Contract Documents, except for repeat testing which results from CONTRACTOR'S negligence or his repeated failure to meet Contract Document requirements.
- C. The testing laboratory is not authorized to approve or accept any portion of the Work; rescind, alter or augment the requirements of the Contract Documents; or perform any duties of CONTRACTOR.

1.2 QUALIFICATIONS OF LABORATORY

- A. Where applicable, the testing laboratory will meet "Recommended Requirements for Independent Laboratory Qualification", latest edition, published by American Council of Independent Laboratories and the basic requirements of ASTM E 329 "Specification for Agencies engaged in Construction Inspection, Testing, or Special Inspection".
- B. Testing equipment used by the laboratory will be calibrated at maximum twelve month intervals by devices of accuracy traceable to either National Bureau of Standards or accepted values of natural physical constants.

1.3 LABORATORY DUTIES

- A. The testing laboratory shall:
 - 1. Cooperate with CONTRACTOR and provide qualified personnel promptly on notice.
 - 2. Perform specified inspections, sampling and testing of materials and methods of construction; comply with applicable standards; and ascertain compliance with requirements of Contract Documents.
 - 3. Promptly notify ENGINEER and CONTRACTOR of irregularities or deficiencies of Work that are observed during performance of services.

4. Promptly submit electronic copies of reports of inspections and tests to ENGINEER, including:
 - a. Date issued.
 - b. Project title and number.
 - c. Testing laboratory name and address.
 - d. Date of inspection or sampling.
 - e. Record of temperature and weather.
 - f. Date of test.
 - g. Identification of product and Specification Section.
 - h. Location in Project.
 - i. Type of inspection or test.
 - j. Results of tests and observations regarding compliance with Contract Documents.
5. Perform additional tests and services, as required by OWNER.

1.4 CONTRACTOR'S RESPONSIBILITIES

A. General:

1. Cooperate with laboratory personnel and provide access to Work and to manufacturer's operations.
2. Provide to laboratory, preliminary representative samples of materials to be tested, in required quantities.
3. Furnish copies of product test reports.
4. Provide to the laboratory the preliminary design mix proposed for concrete and other material mixes that require testing by the testing laboratory.
5. Furnish labor and facilities:
 - a. To provide access to Work to be tested.
 - b. To obtain and handle samples at the site.
 - c. To facilitate inspections and tests.
 - d. For laboratory's exclusive use for storage and curing of test samples.
 - e. Forms for preparing concrete test beams and cylinders.
6. Notify laboratory and ENGINEER sufficiently in advance of operations to allow for assignment of personnel and scheduling of tests.
7. Arrange with laboratory and pay for additional samples and tests required for CONTRACTOR'S convenience.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01452

TESTING LABORATORY SERVICES FURNISHED BY CONTRACTOR

PART 1 - GENERAL

1.1 DESCRIPTION

- A. The CONTRACTOR shall employ and pay for an independent testing laboratory to perform the specified services as required in various sections of the Contract Documents. Laboratory selected shall be subject to approval by the ENGINEER.

1.2 QUALIFICATIONS OF LABORATORY

- A. Where applicable, meet "Recommended Requirements for Independent Laboratory Qualification," latest edition, published by American Council of Independent Laboratories and the basic requirements of ASTM E 329, "Standards of Recommended Practice for Inspection and Testing Agencies for Concrete and Steel as Used in Construction." Laboratory shall be authorized to operate in the State of Arizona.
- B. Submit electronic copies of report of inspection of facilities made by Materials Reference Laboratory of National Bureau of Standards, for the most recent tour of inspection, with memorandum of remedies of any deficiencies reported by inspection.
- C. Testing Equipment:
 - 1. Calibrated, at maximum 12-month intervals by devices of accuracy traceable to either National Bureau of Standards or accepted values of natural physical constants.
 - 2. Submit copy of certificate of calibration made by an accredited calibration agency.

1.3 LABORATORY DUTIES

- A. Cooperate with ENGINEER and provide qualified personnel promptly on notice.
- B. Perform specified inspections, sampling and testing of materials and methods of construction; comply with applicable standards; and ascertain compliance with requirements of Contract Documents.
- C. Promptly notify ENGINEER and CONTRACTOR of any irregularities or deficiencies of Work that are observed during performance of services.

- D. Promptly submit electronic copies of reports of inspections and tests to ENGINEER, including:
 - 1. Date issued.
 - 2. Project title and number.
 - 3. Testing laboratory name and address.
 - 4. Name and signature of inspector.
 - 5. Date of inspection or sampling.
 - 6. Record of temperature and weather.
 - 7. Date of test.
 - 8. Identification of product and Specification Section.
 - 9. Location in Work.
 - 10. Type of inspection or test.
 - 11. Results of tests and observations regarding compliance with Contract Documents.
- E. Perform additional tests and services as required to ensure compliance with the Contract Documents.

1.4 CONTRACTOR'S COORDINATION WITH LABORATORY

- A. Cooperate with laboratory personnel, and provide access to Work and to manufacturer's operations.
- B. Provide to laboratory representative samples of materials to be tested, in quantities required by the laboratory for testing.
- C. Furnish labor and facilities:
 - 1. To provide access to Work to be tested.
 - 2. To obtain and handle samples at the site.
 - 3. To facilitate inspections and tests.
 - 4. For laboratory's exclusive use for storage and curing of test samples.
 - 5. Forms for preparing concrete test beams and cylinders.
- D. Notify laboratory and ENGINEER sufficiently in advance of operations to allow for assignment of personnel and scheduling of tests.
- E. Arrange with laboratory and pay for, additional samples and tests required for CONTRACTOR'S convenience.

1.5 PRODUCT TEST REPORTS

- A. Furnish copies of product test reports where required by the Specifications or requested by ENGINEER.

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PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01453

TESTING OF HYDRAULIC STRUCTURES

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. Provide all labor, material, tools, equipment and incidentals as shown, specified and required to clean, flush and test structures.
2. The Work also shall include all labor and materials required to prepare a structure for testing, convey water to the testing location, perform the testing, and all labor and materials required to drain and dispose of water used for testing.

B. Hydraulic Structures Scheduled for Hydrostatic Testing: Clean and test the following structures:

Hydraulic Structure Number	Hydraulic Structure Service and Location
Cast-in-Place Junction Structure B	Sanitary Sewer Structure North of 202 Freeway

C. Water for Testing:

1. Water for initial testing will be furnished by the OWNER.
2. Provide all temporary piping, hose, valves, backflow preventers, appurtenances, and services required for testing.
3. Convey the water to the testing location.
4. Water for testing may be withdrawn from:
 - a. As directed by the OWNER and ENGINEER.
5. The maximum rate at which water may be withdrawn is to be determined by the OWNER and ENGINEER. Provide on the withdrawal piping, downstream of the backflow preventer, a valve to control the rate of flow and a flow meter. The flow meter shall be calibrated within one year and field calibrated/verified prior to the date of its use, and its certification of calibration shall be submitted to the ENGINEER for approval.
6. Cost of water for re-testing shall be paid by CONTRACTOR to OWNER at OWNER'S standard rates.

D. Testing and piping is specified under Section 15051, Buried Piping Installation.

1.2 DEFINITIONS

- A. The term “hydraulic structures” is defined as tanks, channels, and other structures through which liquid is conveyed or that hold liquid. Hydraulic structures include structures that are open to the atmosphere and structures with closed tops. Hydraulic structures, include but are not limited to, wet wells, junction chambers equalization tanks, storage tanks, treatment process tanks such as grit chambers, clarifiers, aeration tanks, filter beds, contact tanks, and other channels or tanks as designated herein.
1. Excluded are structures where cleaning and testing are specified under other Sections or contracts.

1.3 SUBMITTALS

- A. Provide written notice of the proposed testing schedule for a given structure for review by the ENGINEER and OWNER at least 15 days prior to the scheduled testing. Include proposed plans for water conveyance, control, and disposal. Testing will not commence without approval of ENGINEER.
- B. Shop Drawings: Submit for approval the following:
1. Cleaning procedures.
 2. Hydrostatic testing procedures, methods, equipment, coordination, and schedules.
 3. Report for each test.
 4. Comply with the requirements of Section 01332, Shop Drawing Procedures.
 5. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures. Text shall be in electronic ASCII format. Drawings and figures shall be in Version 2012 or newer AutoCAD ".dwg," or pdf ".pdf" format.
 6. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be

sufficient cause for rejection of the entire submittal with no further consideration.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Temporary valves, bulkheads, and other water control equipment and materials, shall be determined by CONTRACTOR subject to the ENGINEER'S review. No materials shall be used which would be injurious to the construction or its future function.

PART 3 - EXECUTION

3.1 CLEANING

- A. Cleaning Requirements:
 - 1. Remove all scaffolding, planks, tools, rags, dirt, debris, and material not part of the structure prior to testing.
 - 2. Thoroughly clean the walls, floors, and operating equipment by sweeping, high-pressure wash, scrubbing, or other methods approved by ENGINEER.
 - 3. Remove all water, dirt, or foreign material accumulated during cleaning from the hydraulic structure. Provide temporary pumps, piping, and facilities as required to discharge water from the cleaning operation in a manner approved by ENGINEER.
 - 4. Do not proceed with testing until ENGINEER has approved the results of the cleaning operation.
 - 5. Cleaning shall conform to the requirements of Section 01740, Cleaning.

3.2 GENERAL FOR TESTING AND DISINFECTION

- A. The following requirements apply:
 - 1. Each hydraulic structure shall be tested separately.
- B. Hydraulic structures shall be free of visible leakage. Repair leaks in a manner subject to ENGINEER'S approval and in accordance with the Contract Documents.
- C. The structure shall be tested after interior coating or linings system is installed, if applicable.
- D. Release of water from structures, after testing shall be as approved by the ENGINEER. Release of water to City of Chandler sewer system is acceptable.

3.3 HYDROSTATIC TESTING OF HYDRAULIC STRUCTURES

- A. Analysis of data from hydrostatic tests of hydraulic structures shall be performed by CONTRACTOR in accordance with the requirements of ACI 350.1 and as specified herein. Supply all materials and labor to obtain the test data. Two separate hydrostatic tests shall be completed, one using the stop logs and another without and using other means to retain the liquid contents within the structure.
- B. Prior to the start of hydrostatic testing, the following shall be met.
 - 1. All elements of the structure that will resist pressure exerted by the retained liquid shall be in place and at specified strength levels. Concrete shall be fully cured.
 - 2. Structure walls shall not be backfilled and, if damp proofing is specified, coated with damp proofing prior to leakage testing, unless otherwise approved by ENGINEER.
 - 3. Stop logs that control the flow of or otherwise retain the liquid contents of the structure, shall be checked for water-tightness. If not watertight, provide measures to ensure water-tightness during the hydrostatic test.
 - 4. Defective concrete shall be repaired.
 - 5. Notify ENGINEER and OWNER a minimum of 24 hours prior to the start of filling of the structure for hydrostatic testing.
 - 6. Concrete hydraulic structures shall remain filled with clean water for an initial 48-hour period to allow for adsorption. Following this initial period, add make-up water to fill the hydraulic structure to the specified water surface test elevation.
- C. Fill the hydraulic structure with clean water to ten (10) feet in elevation.
- D. Filling Hydraulic Structures with Water:
 - 1. Fill the portion of the hydraulic structure to be tested at a rate not to exceed two vertical feet per hour.
 - 2. During filling, provide a backflow preventer at the point where water is withdrawn from the existing potable water system, if applicable.
- E. After water has been brought to the test elevation and the specified wetting period has elapsed, inspect the exposed surfaces of the structure for leakage. Repair locations where leakage or weeping is evident prior to the start of hydrostatic testing.
- F. Hydrostatic test duration shall be determined by the ENGINEER based on ACI 350.1, but shall not be less than 24 hours.
- G. Allowable Leakage:
 - 1. Leakage is defined as the quantity of water that must be supplied to the hydraulic structure or any section thereof to maintain the water level within 3-

inches of the specified water surface test elevation during the hydrostatic test, plus the amount of water required to fill the hydraulic structure to the specified water surface test elevation at the conclusion of the hydrostatic test, plus precipitation, minus an allowance for evaporation if applicable.

2. For concrete structures with interior wetted surfaces lined with a waterproof material, the allowable leakage is 0.0 (zero) percent of the volume tested per 24-hour period.

H. Measurement Locations:

1. Structures or structure cells that are less than 1,000 square feet in water surface area shall have measurements of water level taken at a minimum of two locations that are approximately 180 degrees apart.
2. Structures or structure cells that are greater than 1,000 square feet in water surface area shall have measurements of water level taken at a minimum of four locations that are approximately 90 degrees apart.
3. Each measurement location shall be marked and given a reference number. A reference point shall be marked on the face of the wall above the test water surface in a manner that will prevent movement or deterioration of the reference point mark during the test.
4. Measurement locations shall be located so that the effects of wave action and wind are minimized.

I. Evaporation and Precipitation Measurement:

1. In hydraulic structures that are open to the atmosphere, a clear plastic calibrated open-topped container not less than 18-inches in diameter and depth shall be partially filled, floated in the tank, and held in position near each measurement location. Calibration increments shall be 0.1-inch or less.
2. Containers shall be located so that they are not shaded by the structure's walls, and are away from overhead items such as beams, pipes, and walkways.

J. Test Measurements:

1. Do not start hydrostatic tests when severe weather conditions, such as heavy precipitation, high winds, major changes in average daily temperature, and other severe conditions are predicted.
2. Record the following measurements at each test location at the start of the test period and at 12-hour intervals thereafter.
 - a. Distance from reference point to test water surface.
 - b. Depth of water in the evaporation-precipitation containers.
 - c. Temperature of the test water at a point 18-inches below the water surface.
 - d. Temperature of the water in the evaporation-precipitation containers at mid-depth.
3. If the water surface is subject to wave action at the measurement location, the average water surface elevation of the wave oscillations shall be recorded as the data.

4. The change in the water surface elevation at each measurement location shall be averaged and adjusted as follows:
 - a. The total change in the hydraulic structure's water surface elevation shall be adjusted by the average change in water surface elevation in the evaporation-precipitation containers.
 - b. Where the averaged water temperature measurements vary by more than 3 degrees from start to completion of the test period, adjustment in the test volume shall be determined by the change of the density of water resulting from the change in the average water temperature.
 5. Determination of Leakage:
 - a. Provide a test container filled with a known quantity of water at the start of the test. Attach the test pump suction to the test container.
 - b. Pump water from the test container into the hydraulic structure with the test pump to maintain the water level in the hydraulic structure within 3-inches of the specified test elevation for the duration of the test period.
 - c. At the conclusion of the test, pump water from the container into the hydraulic structure to attain the specified test elevation.
 - d. Measure the water remaining in the container and record the amount used during the test on the test report.
- K. Criteria for Acceptance:
1. The hydrostatic test will pass if the measured leakage is less than the allowable leakage and no leaks or weeping are observed.
 2. The hydrostatic test shall be considered to have failed if the allowable leakage is exceeded or if leakage or weeping is observed.
 3. If the test becomes unreliable due to excessive precipitation or other external factors, the test shall be re-started.
 4. If a hydrostatic test fails, the structure may be re-tested immediately without repairs if approved by the ENGINEER. If the subsequent hydrostatic test fails, repair probable areas of leakage and repeat the hydrostatic test.
 5. Re-test the structure until it meets the specified criteria for acceptance. Repair probable leakage areas before testing.
- L. Reuse and Disposal of Water Used in Hydrostatic Tests:
1. Obtain written approval of the ENGINEER before water used in one hydrostatic test is pumped to a different hydraulic structure for reuse in a subsequent test.
- M. The hydraulic structure shall not be backfilled or damp-proofed until acceptance of the hydrostatic test by the ENGINEER.

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3.4 TESTING OF APPURTENANT PIPING

- A. Piping appurtenant to hydraulic structures shall be tested as specified in applicable Sections.

++ END OF SECTION ++

SECTION 01570

TEMPORARY CONTROLS

PART 1 - GENERAL

1.1 DESCRIPTION

- A. CONTRACTOR shall provide and maintain methods, equipment, and temporary construction, as necessary to provide controls over environmental conditions at the construction site and adjacent areas. Remove physical evidence of temporary facilities at completion of Work.
- B. CONTRACTOR shall obtain all City, County and State permits required for the construction of all Work, including Hazardous Material Management, Earth Moving/Dust Control and Storm water/Storm water Pollution Prevention Permits.

1.2 NOISE CONTROL

- A. CONTRACTOR'S vehicles and equipment shall be such as to minimize noise to the greatest degree practicable. Noise levels shall conform to the latest OSHA standards and in no case will noise levels be permitted which interfere with the Work of the OWNER or others.

1.3 PEST AND RODENT CONTROL

- A. Provide rodent and pest control as necessary to prevent infestation of construction or storage areas.
 - 1. Employ methods and use materials that will not adversely affect conditions at the site or on adjoining properties.

1.4 WATER CONTROL

- A. Provide methods to control surface water and water from excavations and structures to prevent damage to the Work, the site, or adjoining properties.
 - 1. Control fill, grading and ditching to direct water away from excavations, pits, tunnels and other construction areas and to direct drainage to proper runoff courses so as to prevent any erosion, damage or nuisance.

1.5 EROSION CONTROL

- A. Plan and execute construction and earth work by methods to control surface drainage from cuts and fills, and from borrow and waste disposal areas, to prevent erosion and sedimentation.

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1. Hold the areas of bare soil exposed at one time to a minimum.
 2. Provide temporary control measures such as berms, dikes and drains.
- B. Construct fills and waste areas by selective placement to eliminate surface silts or clays which will erode.
- C. Periodically inspect earthwork to detect any evidence of the start of erosion; apply corrective measures as required to control erosion.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01722

FIELD ENGINEERING

PART 1 - GENERAL

1.1 DESCRIPTION

- A. The CONTRACTOR will establish a base line for the Project and two benchmarks for use by the ENGINEER and CONTRACTOR during the project. The ENGINEER and CONTRACTOR will coordinate the location of the benchmarks and base line to suit the Work.
- B. CONTRACTOR shall:
1. Develop and make all detail surveys and measurements needed for construction including slope stakes, batter boards, and all other working lines, elevations and cut sheets.
 2. Provide all material required for bench marks, control points, batter boards, grade stakes, structure and pipeline elevation stakes, and other items.
 3. Be solely responsible for all locations, dimensions and levels. No data other than written orders of the ENGINEER shall justify departure from the dimensions and levels required by the Contract Documents.
 4. Safeguard all points, stakes, grade marks, monuments and bench marks made or established on the Work. Re-establish same with the exception of primary control monuments if disturbed and rectify all Work improperly installed because of not maintaining, not protecting or removing without authorization established points, stakes, marks and monuments.
 5. Provide such facilities and assistance as may be necessary for ENGINEER to check line and grade points placed by CONTRACTOR. Do not perform any excavation or embankment work until all cross-sectioning necessary for determining pay quantities has been completed and checked by ENGINEER.
 6. CONTRACTOR shall provide notices and comply with all laws, ordinances, rules and regulations bearing on the conduct of the Work. If CONTRACTOR observes that the Contract Documents are at variance therewith, promptly notify the ENGINEER, in writing.

1.2 CONTRACTOR'S DAILY REPORTS

- A. CONTRACTOR shall provide:
1. Provide daily reports of Project activity. Reports to be submitted to the ENGINEER with all pertinent information pertaining to the project as follows:
 - a. Number of employees.
 - b. Subcontractor employees.

- c. Breakdown of employees by trades.
 - d. Major equipment and materials installed.
 - e. Major construction equipment utilized, in use and standby.
 - f. Location of all areas in which construction was done.
 - g. Materials and equipment received.
 - h. Work and tests performed.
 - i. Safety.
 - j. Delays.
2. CONTRACTOR shall submit electronic copies of CONTRACTOR'S daily reports. The daily report is due at the ENGINEER'S field office by 9:00 a.m. the next working day after the Work was performed and shall be signed by a responsible member of CONTRACTOR'S staff.
 3. CONTRACTOR shall check all formwork, reinforcing, inserts, structural steel, bolts, sleeves, piping, other materials and equipment.
 4. CONTRACTOR shall maintain field office files and drawings, Record Drawings, and coordinate engineering services with subcontractors. Prepare layout and coordination drawings for construction operations.
 5. CONTRACTOR shall check and coordinate Work for conflicts and interferences and immediately advise ENGINEER of all discrepancies noted.
 6. CONTRACTOR shall cooperate with ENGINEER in field inspections, as required.
 7. CONTRACTOR shall review and coordinate Shop Drawings and other submittals.

1.3 CONTRACTOR'S SURVEYOR

- A. CONTRACTOR shall employ and retain, as needed at the Work site a surveyor with the experience and capability of performing all surveyor and layout tasks required of CONTRACTOR. It is also the CONTRACTOR'S responsibility to notify the ENGINEER, in writing, of any discrepancy found between the topographic survey provided and the baseline conditions provided in the Contract. The surveyor shall be a land surveyor registered in the State of Arizona. Tasks included are:
 1. CONTRACTOR shall provide all surveying equipment required including transit, level, stakes and required surveying accessories.
 2. CONTRACTOR shall furnish all required lines and grades for construction of all facilities, structures, pipelines and site improvements.
 3. CONTRACTOR shall keep professional, accurate, well organized, and legible notes of all measurements and calculations made while surveying and laying out the Work.
 4. CONTRACTOR shall survey, locate, and record and redline Drawings to accurately represent all utilities and buried structures prior to backfilling.
 5. CONTRACTOR will report to the OWNER whenever any reference point is lost or destroyed or requires relocation because of necessary changes in grades or locations. The CONTRACTOR will replace and accurately relocate all

reference points so lost, destroyed, or moved at no additional cost to the OWNER

- B. Field surveying shall include GIS location and data of all new construction as required by the City of Chandler. The CONTRACTOR shall provide the GIS location and data at each application of payment to verify content. If data is missing or does not meet City of Chandler requirements, portions of the new construction shall be exposed to obtain the data at no additional cost to the OWNER. The City of Chandler requirements are identified below:
 - 1. Contractor shall provide survey grade GPS / GIS data, per Exhibit F of the contract.
- C. Refer to the included information, following this specification, from the City of Chandler. Contractor shall meet all requirements. Any primary control survey monuments damaged or destroyed, will be re-established by the ENGINEER, at CONTRACTOR'S expense.
- D. CONTRACTOR shall perform such surveys and computations as are necessary to determine quantities of Work performed or placed during each progress payment period, and shall perform all surveys necessary for the ENGINEER to determine final quantities of Work in place.
- E. CONTRACTOR shall notify the ENGINEER at least 24 hours before performing a quantity survey and, unless waived in writing by the ENGINEER, quantity surveys shall be performed in the presence of the ENGINEER.
- F. From established primary control points, establish all lines and grades, and elevations necessary to control the Work, and shall be responsible for all measurements that may be required for execution of the Work to the tolerances prescribed in the Contract Documents.
- G. Establish, place, and replace as required, such additional stakes, markers, and other controls as may be necessary for control, intermediate checks, and guidance of construction operations.
- H. Upon completion of sewer line installation and prior to backfill, CONTRACTOR shall survey and provide survey point data for all sewer piping inverts at a minimum of 25-ft intervals on straight runs. CONTRACTOR shall provide a point file identifying location by northing, easting, and elevation and be included with the as-built documents in AutoCAD format.
- I. Upon completion of the final grade, CONTRACTOR shall perform a survey as-built of the project disturbed areas to confirm that the final grade matches the contract drawings. The survey shall be at 1-foot contours and shall identify all exposed

manhole lids (center), manhole center, structure corners, ADOT retaining wall, irrigation equipment, and ADOT irrigation ditch. The CONTRACTOR shall submit the point file and a base file to the ENGINEER with the as-built drawings. In the event the finished grade determined by the survey does not match the contract drawings or if there are low spots that cause a drainage issue such as ponding, the CONTRACTOR shall re-grade the area at their own expense until the area is corrected and satisfied by the ENGINEER and OWNER.

1.4 SURVEYING

- A. CONTRACTOR shall follow the following construction surveying guidelines for this project:
 - 1. Alignment Staking: Each 50 feet on tangent; each 25 feet on curves.
 - 2. Slope Staking: Each 50 feet on tangent; each 25 feet on curves; restake every 10 feet in elevation.
 - 3. Structure: Stake out structures, including elevations; checkouts prior to and during construction.
 - 4. Pipeline: Stake out pipelines including elevations; checkout prior to and during construction. Survey shots shall be taken on pipeline change of elevation and direction and shall be as-built.
 - 5. Cross-Section: Original, final and intermediate as required, for the structure sites and other locations as necessary for quantity surveys..
 - 6. Survey shots shall be taken on all duct bank change of elevation and direction and shall be as-built.
- B. Temporary survey references set by CONTRACTOR for CONTRACTOR'S own use shall be established to at least second order accuracy (e.g., 1:10000). Construction staking used as a guide for the actual Work shall be set at least third order accuracy (e.g., 1:5000). The basis on which such orders are established shall be sufficient to provide the absolute margin for error specified below.
- C. The horizontal accuracy of easement staking shall be plus or minus 0.1 feet. The accuracy of all other staking shall be plus or minus 0.04 feet horizontally and plus or minus 0.02 feet vertically.
- D. Survey calculations shall include an error analysis sufficient to demonstrate the required accuracy.
- E. Survey Records:
 - 1. Maintain a complete, accurate log of all control and survey Work as it progresses.
 - 2. All survey data shall be in accordance with recognized professional surveying standards. All original field notes, computations, and other surveying data shall be recorded by CONTRACTOR'S surveyor in CONTRACTOR furnished hard-

bound field books, and shall be signed and sealed by CONTRACTOR'S surveyor. The completeness and accuracy of all survey Work, and the completeness and accuracy of the survey records, including the field books, shall be the responsibility of CONTRACTOR. Failure to organize and maintain survey records in a professional manner to allow reasonable and independent verification of all calculations by the ENGINEER, and to allow reasonable identification by the ENGINEER of all elevations, dimensions, and grades of the Work shall be cause for rejection of the survey records, including the field books.

3. Illegible notes or data, or erasures on any page of the field books are not acceptable. Copied notes or data shall not be permitted. Corrections by ruling or lining out errors will be satisfactory only if initialed by the surveyor. Violation of the above may require resurveying the data in question.

F. Survey Submittal:

1. Survey submittal shall be made as described herein. Submittal shall be signed and sealed by CONTRACTOR'S surveyor and shall include:
 - a. A complete survey plan that shall be submitted ten days prior to beginning survey Work.
 - b. Resumes shall be submitted of the Registered Land Surveyors conducting the Work ten days prior to beginning survey Work. During the course of the Work, a resume shall be submitted for each new Registered Land Surveyor working on the project at least ten days prior to the beginning of Work by such new Registered Land Surveyor.
 - c. A sample of the proposed survey field books to be maintained by CONTRACTOR'S surveyor. The sample shall have sufficient information and detail, including example calculations and notes, to demonstrate that the field books will be organized and maintained in a professional manner, meeting the requirements of Article 1.3 and Article 1.4 of this Section.
 - d. The original field books shall be submitted within two days upon completion of the Work.

1.5 SOILS AND MATERIAL TESTING

- A. Refer to Section 01452, Testing Laboratory Services Furnished by CONTRACTOR for Soils and Materials Testing.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01723

CUTTING AND PATCHING

PART 1 - GENERAL

1.1 DESCRIPTION

- A. This Section includes administrative and procedural requirements for the cutting and coring, and rough and finish patching of holes and openings in existing construction.
- B. The CONTRACTOR shall adhere to all applicable safety guidelines in accordance with Federal, State and local ordinances.
- C. All cutting, coring and rough patching shall be performed by CONTRACTOR requiring the opening. Finish patching shall be the responsibility of CONTRACTOR and shall be performed by the trade associated with the application of the particular finish.
- D. CONTRACTOR shall provide cutting, coring, fitting and patching, including attendant excavation and backfill required to complete the Work, or to:
 - 1. Remove and replace defective Work or Work not conforming to requirements of the Contract Documents.
 - 2. Remove samples of installed Work as specified or required for testing.
 - 3. Remove all constructions required to provide for specified alterations or addition to existing work.
 - 4. Uncover Work to provide for ENGINEER'S observation of covered Work or observation by regulatory agencies having jurisdiction.
 - 5. Connect to completed Work that was not accomplished in the proper sequence.
 - 6. Remove or relocate existing utilities and pipes that obstruct the Work in locations where connections must be made.
 - 7. Make connections or alterations to existing or new facilities.
- E. CONTRACTOR shall coordinate the requirements of the Work in this Section along with the requirements of the Sections listed below which includes Work that is directly related to this Section.
 - 1. Division 2, Site Work, through 15, Mechanical, Sections for specific requirements and limitations applicable to cutting and patching individual parts of the Work.

1.2 QUALITY ASSURANCE

- A. Structural Work: CONTRACTOR shall not cut or patch structural elements in a manner that would change their load-carrying capacity as load-deflection ratio.

- B. Operating Elements: CONTRACTOR shall not cut or patch operating elements in a manner that would result in reducing their capacity to perform as intended. Do not cut or patch operating elements or related components in a manner that would result in increased maintenance or decreased operational life or safety.

1.3 SUBMITTALS

- A. CONTRACTOR shall submit a written request to ENGINEER well in advance of executing any cutting or alteration which affects:
1. Design function or intent of Project.
 2. Work of OWNER or any other contractor.
 3. Structural value or integrity of any element of the Project.
 4. Integrity or effectiveness of weather-exposed or moisture-resistant elements or systems.
 5. Efficiency, operational life, maintenance or safety of operational elements.
 6. Visual qualities of sight-exposed elements.
- B. Request shall include:
1. Identification of Project.
 2. Description of affected Work of CONTRACTOR and work of others.
 3. Necessity for cutting.
 4. Effect on work of OWNER or any other contractor, or on structural or weatherproof integrity of Project.
 5. Description of proposed Work, describing:
 - a. Scope of cutting and patching.
 - b. Trades who will be executing the Work.
 - c. Products proposed to be used.
 - d. Extent of refinishing.
 - e. Schedule of operations.
 6. Alternatives to cutting and patching, if any.
 7. Designation of party responsible for cost of cutting and patching, when applicable.
 8. Written permission of any other contractor whose work will be affected.
- C. Should conditions of Work, or schedule, CONTRACTOR shall indicate a change of materials or methods, submit written recommendation to ENGINEER, including:
1. Conditions indicating change.
 2. Recommendations for alternative materials or methods.
 3. Submittals as required for substitutions.
- D. CONTRACTOR shall submit written notice to ENGINEER, designating time Work will be uncovered, to provide for observation. Do not begin cutting or patching operations until authorized by ENGINEER.

- E. CONTRACTOR shall conform to all applicable specifications for application and installation of materials used for patching.
- F. CONTRACTOR shall submit electronic copy per Specification Section 01332, Shop Drawing Procedures.

1.4 WARRANTY

- A. CONTRACTOR shall replace, patch and repair materials and surfaces cut or damaged during cutting and patching operations, by methods and with materials in such a manner as to not void required or existing warranties.

PART 2 - PRODUCTS

2.1 MATERIALS, GENERAL

- A. Use materials identical to existing materials. For exposed surfaces, use materials that visually match existing adjacent surfaces to fullest extent possible. If identical materials are unavailable or cannot be used, use materials whose installed performance will equal or surpass that of existing materials.

PART 3 - EXECUTION

3.1 GENERAL

- A. Perform all cutting and coring in such a manner as to limit the extent of patching.
- B. Core drill all holes to be cut through concrete and masonry walls, slabs or arches, unless otherwise approved by the ENGINEER.

3.2 INSPECTION

- A. Examine surfaces to be cut and patched and conditions under which cutting and patching are to be performed before cutting.
- B. Report unsatisfactory or questionable conditions to ENGINEER, in writing. Do not proceed with Work until the ENGINEER has provided further instructions.

3.3 PREPARATION

- A. Provide temporary support as required to maintain structural integrity of Project, to protect adjacent Work from damage during cutting, and to support the Work to be cut.

- B. Protect existing construction during cutting and patching to prevent damage. Provide protection from adverse weather conditions for portions of the Project that will be exposed during cutting and patching operations.
 - 1. Avoid interference with use of adjoining areas or interruption of free passage to adjoining areas.
 - 2. Do not cut existing pipe, conduit or ductwork serving facilities scheduled to be removed or relocated until provisions have been made to bypass them.

3.4 CORING

- A. Perform coring with a non-impact rotary tool using diamond core drills. Size holes for pipe, conduit, sleeves, equipment or mechanical seals, as required.
- B. Protect existing equipment, utilities and adjacent areas from water and other damage covered by drilling operations.
- C. Vacuum or otherwise remove slurry or tailings from the Work area following drilling.

3.5 CUTTING

- A. Cut existing construction using methods least likely to damage elements retained or adjoining construction and that will provide proper surfaces to receive installation or repair.
 - 1. In general, use hand or small power tools designed for sawing or grinding, not hammering and chopping.
 - 2. Cut through concrete and masonry using a concrete wall saw with diamond saw blades.
 - a. Provide for control, on both sides of walls, of slurry generated by sawing.
- B. Cut holes and slots as small as possible, neatly to size required, and with minimum disturbance of adjacent surfaces. Provide temporary covering over openings where not in use.
- C. To avoid marring existing finished surfaces, cut or drill from exposed or finished side into concealed side.
- D. Provide adequate bracing of area to be cut prior to start of cutting.
- E. Provide equipment of adequate size to remove cut panel.

3.6 PATCHING

- A. Patch construction by filling, repairing, refinishing, closing-up and similar operations following performance of other Work. Patch with durable seams that are as invisible

- as possible. Provide materials and comply with installation requirements specified, in other Sections of these Specifications.
- B. Where feasible, test patched areas to demonstrate integrity of installation.
 - C. Fit Work airtight to pipes, sleeves, ducts, conduit and other penetrations through surfaces.
 - D. Restore exposed finishes of patched areas and extend finish restoration into retained adjoining construction in a manner that will eliminate evidence of patching and refinishing.
 - 1. For continuous surfaces, refinish to nearest intersection.
 - 2. For an assembly, refinish entire unit.
 - E. Patch, repair or rehang existing ceilings as necessary to provide an even-plane surface of uniform appearance.

3.7 CLEANING

- A. Clean areas and spaces where cutting, coring and patching are performed. Clean piping, conduit or similar constructions before applying paint or other finishing materials. Restore damaged pipe covering to original condition.

+ + END OF SECTION + +

SECTION 01740

CLEANING

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Execute cleaning, during progress of the Work, at completion of the Work, and as required by General Conditions. If CONTRACTOR fails to clean areas as specified in this Section, the OWNER will have the areas cleaned and backcharge CONTRACTOR.

1.2 REQUIREMENTS OF REGULATORY AGENCIES

- A. In addition to the requirements herein, maintain the cleanliness of the Work and surrounding premises within the Work limits so as to comply with federal, state, and local fire and safety laws, ordinances, codes and regulations.
- B. Comply with all federal, state and local anti-pollution laws, ordinances, codes and regulations when disposing of waste materials, debris and rubbish.

1.3 PROGRESS CLEANING

- A. General: Clean the Site, Work areas and other areas CONTRACTOR is permitted to occupy by Laws and Regulations at least weekly. Dispose of materials lawfully according to Laws and Regulations:
 - 1. Comply with requirements in NFPA 241, Standard for Safeguarding Construction, Alteration and Demolition Operations, for removal of combustible waste materials and debris.
 - 2. Do not hold other materials more than three days if the temperature is expected to rise above 80° F.
 - 3. Provide suitable containers for storage of waste materials and debris.
 - 4. Containerize hazardous and unsanitary waste materials separately from other waste. Mark containers appropriately.
- B. Project:
 - 1. Maintain Project free of waste materials and debris.
 - 2. Keep exterior dust generating areas wetted down.
- C. Work Areas: Clean areas where Work is in progress to the level of cleanliness necessary for proper execution of the Work.
 - 1. Remove liquid spills promptly and report spills to the OWNER and ENGINEER immediately.

2. Where dust would impair proper execution of the Work, broom-clean or vacuum the entire Work area, as appropriate.
- D. Installed Work: Keep installed Work clean. Clean installed surfaces according to written instructions of manufacturer or fabricator of product installed, using only cleaning materials specifically recommended. If specific cleaning materials are not recommended, use cleaning materials that are not hazardous to health or property and that will not damage exposed surfaces.
- E. Concealed Spaces: Remove all debris from concealed spaces before enclosing the space.
- F. Exposed Surfaces: Clean exposed surfaces and protect as necessary to ensure freedom from damage and deterioration at time of Substantial Completion.
- G. Cutting and Patching: Clean areas and spaces where cutting and patching are performed. Completely remove paint, mortar, oils, putty, and similar materials.
 1. Thoroughly clean piping, conduit, and similar features before applying paint or other finishing materials. Restore damaged pipe covering to its original condition.
- H. Waste Disposal:
 1. Properly dispose of all waste materials, surplus materials, debris and rubbish off the Project site.
 2. Do not burn or bury rubbish and waste materials on the Project site.
 3. Do not dispose of volatile or hazardous wastes such as mineral spirits, oil, or paint thinner in storm or sanitary drains.
 4. Do not discharge wastes into streams or waterways.
 5. Sole responsibility for complying with any federal, state, and local environmental and regulations in disposing of waste, belongs to CONTRACTOR.
- I. During handling and installation, clean and protect construction in progress and adjoining materials already in place. Apply protective covering where required to ensure protection from damage or deterioration at Substantial Completion.
- J. Clean and provide maintenance on completed construction as frequently as necessary through the remainder of the construction period. Adjust and lubricate operable components to ensure operability without damaging effects.

1.4 FINAL CLEANING

- A. General: Provide final cleaning.
 1. Complete the following cleaning and waste-removal operations before requesting inspection for certification of Substantial Completion for entire Project or for a portion of Project:

- a. Clean and remove from the Project rubbish, waste material, debris, and other foreign substances.
- b. Mechanical sweeping of paved areas. Remove petrochemical spills, stains, and other foreign deposits.
- c. Hose clean sidewalks and loading areas.
- d. Rake grounds that are neither planted nor paved to a smooth, even-textured surface.
- e. Leave water courses, gutters, and ditches open and clean.
- f. Repair pavement, roads, sod, and all other areas affected by construction operations and restore them to original condition or to minimum condition specified.
- g. Clean exposed exterior and interior hard-surfaced finishes to a dirt-free condition, free of spatter, grease, stains, fingerprints, films, and similar foreign substances.
- h. Clean, wax and polish wood, vinyl and painted floors.
- i. Remove debris and surface dust from limited access spaces, including roofs, plenums, shafts, trenches, equipment vaults, manholes, and similar spaces.
- j. Sweep concrete floors broom clean in unoccupied spaces.
- k. Clean transparent materials, including mirrors and glass in doors and windows. Remove glazing compounds and other noticeable, vision- obscuring materials. Replace chipped or broken glass and other damaged transparent materials.
- l. Remove tags and labels that are not permanent.
- m. Touch up and otherwise repair and restore chipped, scratched, dented or otherwise marred surfaces to specified finish and match adjacent surfaces.
 - 1) Do not paint over "UL" or similar labels, including manufacturer mechanical and electrical nameplates.
- n. Wipe surfaces of mechanical and electrical equipment, and similar equipment. Remove excess lubrication, paint and mortar droppings, and other foreign substances.
- o. Clean plumbing fixtures to a sanitary condition, free of stains, including stains resulting from water exposure.
- p. Replace disposable air filters and clean permanent air filters. Clean exposed surfaces of diffusers, registers, and grills.
- q. Clean light fixtures, lamps, globes, and reflectors to function with full efficiency. Replace burned-out bulbs, and those noticeably dimmed by hours of use, and defective and noisy starters in fluorescent and mercury vapor fixtures to comply with requirements for new fixtures.
- r. Maintain the cleaning until OWNER occupies the Project or portion thereof.
- s. Leave Project clean and in a neat and orderly condition satisfactory to ENGINEER.

CITY OF CHANDLER: Public Works and Utilities Department
PROJECT NAME: Loop 101 and 202 Freeway 66-inch Redundant Sewer Project
PROJECT NUMBER: WW2402.401

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 01782

RECORD DOCUMENTS

PART 1 - GENERAL

1.1 DESCRIPTION

- A. CONTRACTOR shall maintain and provide the ENGINEER with Record Documents as specified below and per the General Conditions of the contract.
- B. Definitions
 - 1. Contract Documents: The contract documents include the drawings, specifications, and addenda developed and furnished to the CONTRACTOR at the beginning of construction.
 - 2. As-Built Drawings: As-built drawings are an annotated set of drawings prepared by the CONTRACTOR. They show, in red, as-constructed changes to the original Contract Documents that have been made during the construction process. The As-Built Drawings may include supplemental drawings to provide the necessary detail, comply with project standards or where annotation would otherwise be impractical.
 - 3. Record Drawings: Record Drawings are prepared by the ENGINEER and reflect as-constructed changes that the CONTRACTOR annotated in the As-Built Drawings.
 - 4. Record Documents: The Record Documents include Record Drawings, specifications, addenda, approved shop drawings, samples, photographs, change orders, other modifications to the Contract Documents, test records, survey data, field orders, Request for Information, submittals and all other documents pertinent to the CONTRACTOR'S Work.
- C. Maintenance of Documents:
 - 1. Electronic sets of plans, including any Addenda, of the Drawings will be furnished to CONTRACTOR by the ENGINEER.
 - 2. Maintain in CONTRACTOR'S field office in clean, dry, legible condition complete sets of the following: Drawings, Specifications, Addenda, approved Shop Drawings, Samples, Photographs, Change Orders, other modifications of Contract Documents, test records, survey data, Field Orders, and all other documents pertinent to CONTRACTOR'S Work.
 - 3. Make documents available at all times for inspection by ENGINEER and OWNER.
 - 4. Record Documents shall not be used for any other purpose and shall not be removed from CONTRACTOR'S office without ENGINEER'S approval.

5. Any contractually required testing provided by others shall be thoroughly documented by the CONTRACTOR and maintained with the project Record Documents. All testing results shall be maintained in their own separate log for the project; being kept current weekly and made readily available for viewing at any time.
- D. Marking System: Changes, revisions, additions and deletions, to the record set of Drawings shall be marked in Red.
- E. Recording:
1. Submit as-built drawings and make a record of the locations of all work completed as part of the project. The as-builts must indicate the locations of the beginning(s) and end(s) of the construction, and service connections and appurtenances. They must also show locations and elevations where significant elevation changes occur or changes in direction in all pipe alignments. Their locations must be shown by stationing and dimensioning from appropriate monument lines or in their absence appropriate lot lines, property lines or easement line references.
 2. Label the Cover Sheet, Index and each supplemental sheets of each document "PROJECT RECORD" in 2-inch high printed letters.
 3. Keep the As-Built Drawings current. CONTRACTOR'S refusal, failure or neglect to maintain current As-Built Drawings shall constitute sufficient basis for the ENGINEER to recommend the withholding of some or all of any payment due.
 4. Do not permanently conceal any Work until required information has been recorded.
 5. Drawings: Legibly mark to record actual construction including:
 - a. Depths of various elements of foundation in relation to datum.
 - b. Horizontal and vertical location of underground utilities and appurtenances referenced to permanent surface improvements.
 - c. Location of internal utilities and appurtenances concealed in construction referenced to visible and accessible features of structure.
 - d. Field changes of dimensions and details.
 - e. Changes made by Change Order or Field Order.
 - f. Details not on original Drawings.
 6. Specifications and Addenda: Legibly mark up each Section to record:
 - a. Manufacturer, trade name, catalog number, and supplier of each product and item of equipment actually installed.
 - b. Changes made by Change Order or Field Order.
 - c. Other matters not originally specified.
- F. Record Drawings:
1. Record Drawings shall be prepared for all the Work included in the Contract. On a monthly basis, furnish to the ENGINEER a full size annotated electronic copy

of the As-Build Drawings that include changes from the previous month's As-Build Drawing submittal. Annotations shall include redlined "clouds" of only those changes from the previous month's submittal. The redlined As-Build Drawings shall show the actual in-place installation of the items installed under this Contract. The redlined As-Build Drawings shall show the Work in plan and sections as required for clarity with reference dimensions and elevations that will be used to develop complete Record Drawings.

2. In addition to the redlined As-Build Drawings, prepare and submit CADD ".dwg" files, version 2018 or later, for all supplemental drawings used to complete the As-Build Drawings.
3. Survey results shall be posted to the as-builts on a monthly basis.

G. Submittals:

1. Acceptance of CONTRACTOR'S monthly application for payment shall be dependent on the ENGINEER'S acceptance and agreement that CONTRACTOR'S As-Build Drawings and weekly submittals are complete, thorough and acceptable in showing all Work up through and including such work as CONTRACTOR is claiming for completion and payment on CONTRACTOR'S application for payment. Any items which do not appear on the As-Build Drawings in complete and acceptable form shall not be paid for in CONTRACTOR'S monthly payment.
2. Examination by the ENGINEER of CONTRACTOR'S As-Build Drawings will be made on a weekly basis to determine completion for consideration of monthly pay application. Also, make available all As-Build Drawings at all times to the ENGINEER for examination.
3. Prior to Completion of the Work, deliver final As-Build Drawings to ENGINEER.
4. Accompany final and weekly submittals with transmittal letter containing:
 - a. Date.
 - b. Project title and number.
 - c. CONTRACTOR'S name and address.
 - d. Title and number of each As-Build Drawings.
 - e. Certification that each document as submitted is complete and accurate.
 - f. Signature of CONTRACTOR, or his authorized representative.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

+ + END OF SECTION + +

SECTION 02145
DIVERSION OF SEWAGE FLOW AND DEWATERING

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. This Section describes the existing conditions for temporary bypassing and dewatering of water systems or sewers for internal television inspection (CCTV), cleaning operations and/or rehabilitation of project pipelines. This section provides minimum requirements for temporary bypass pump station and dewatering of sewers as required to completing work and prior to acceptance.

B. Requirements:

1. CONTRACTOR shall provide all labor, materials, equipment, and supervision to temporarily bypass pump sewage flow around the CONTRACTOR'S Work and to dewater the pipeline for removal. The bypass strategy shall maintain flow to prevent wastewater backup into the city's sewage collection system or discharge to the environment.
2. The actual design of the bypass arrangement shall be prepared by the CONTRACTOR, and shall be submitted to the OWNER and ENGINEER for review. Means and methods of accomplishing the bypassing shall be the responsibility of the CONTRACTOR.
3. The CONTRACTOR shall successfully test the system to 1.5 times the maximum operating pressure of the system prior to placing the bypass system into operation,
4. The CONTRACTOR shall notify the OWNER 72 hours prior shutting down or operating the bypass. The bypass system equipment and existing flows shall be continuously monitored by the CONTRACTOR.
5. It is the CONTRACTOR's responsibility to arrange all necessary access and temporary construction agreements with all affected parties for the Contractor's proposed location of the bypass pumping system
6. CONTRACTOR is responsible for immediate and proper cleanup should any spill occur, regardless of amount. Immediately contact the ENGINEER if spill occurs. Regardless if amount, the CONTRACTOR shall pay for all damages and fines incurred as a result of a spill.
7. CONTRACTOR shall connect bypass piping to the existing collection system and utilize temporary pumps and control equipment.
8. CONTRACTOR shall utilize a pumping system with 100% back-up capability. In the event a pump shall fail, an alternate equally sized pump shall be ready to be utilized immediately. This requires the alternate pump(s) to be connected to the force main system.

9. CONTRACTOR shall coordinate operation of temporary bypass pump station with authorized City of Chandler personnel. CONTRACTOR shall not operate any equipment in or about temporary bypass pump station without prior written consent from OWNER and copied to ENGINEER.
10. Bypass pump operation shall include a lighting system at the bypass pump operation and the force main discharge. The lighting system shall be gas operated.
11. CONTRACTOR shall provide qualified personnel, at a minimum two (2), on-site 24 hours per day to maintain the bypass pumps and pipeline.
12. CONTRACTOR shall continuously monitor hydrogen sulfide concentrations at the suction side of the bypass pump operation and the force main discharge. CONTRACTOR shall provide temporary covering at each location to minimize hydrogen sulfide releases. This may include providing and installing a temporary odor control system as determined by the OWNER.
13. CONTRACTOR shall coordinate bypass pump elevations and excavate area to meet suction head requirements.

C. Experience:

1. CONTRACTOR shall utilize staff and/or a subcontractor that has been directly responsible for the bypass pumping of sewage flows during the completion of a similar pipeline project.

1.2 SUBMITTALS

- A. Submit drawings and complete design data showing methods and equipment proposed to be utilized in the water piping or sewer bypassing for review by the ENGINEER. Include the following information in the submittal:
1. Detailed Drawings indicating the scheme and location of temporary sewer line plugs, bypass discharge lines and the method and location for discharging the bypass lines.
 2. Capacities of pumps, prime movers and standby equipment.
 3. Pump equipment data sheets. Data sheets shall include a system head curve along with a pump curve to verify its operating point. The system head curve calculations shall be developed by the CONTRACTOR and be included within the submittal for review.
 4. Design calculations proving adequacy of the system and selected equipment sealed by a Professional Civil Engineer, registered in the State of Arizona.
 5. Detailed list of personnel to be utilized for the bypass pump operation. The list shall also include the local supplier of the by-pass pumps and force mains.
 6. Detailed list of additional parts and equipment to be placed at the site should the system not operate as intended.
 7. Standby Power Source
 8. Secondary Containment Provisions
 9. Spill Response Plan
 10. Odor Control Plan

11. Detailed list of items and methods utilized to repair or replace items damaged or modified due to the by-pass pump operation.
- B. The plan shall include a detailed daily schedule and a list of daily activities that will be completed before, during, and after the bypass operation. Include all personnel that are planned to work on the pipeline and the bypass pump operation.
- C. Provide an emergency contact list for all of the personnel to be at the site. Include all of the OWNER'S and ENGINEER'S personnel. Once the emergency contact list is finalized, three (3) copies shall be printed, laminated, and placed at locations along the bypass pump operation as determined by the OWNER.

1.3 JOB CONDITIONS

- A. Available Flow Data:
 1. There is no current flow data available, the CONTRACTOR shall be responsible for any/all monitoring of flows as necessary to properly size the bypass system to ensure against sanitary sewer overflows of upstream sewers
 2. Bypass flows might need to be sent to offsite sanitary sewer lift stations and associated force mains from the project pipelines. It shall be the CONTRACTOR's responsibility to coordinate work and any associated bypass operations with the city to maintain uninterrupted normal operation of these remote facilities.
- B. Protection:
 1. Bypassing to the ground surface, receiving waters, storm drains or bypassing which results in soil or groundwater contamination or any potential health hazards is not permitted.
- C. Scheduling:
 1. The bypassing system is not allowed to be shut down between shifts, on holidays, weekends or during work stoppage without written permission from the city. Provide an attendant, around the clock, whose only duty is to maintain the bypass pumping system until the bypassing of that specific pipeline is no longer required.
- D. Service Lines:
 1. Water or sanitary sewers to be bypassed may have service lines connected to adjacent users. The known service lines have been shown on the construction drawings. Verify the locations of these lines and any other service lines not shown on the drawings.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. CONTRACTOR shall provide temporary piping, fittings, conduits, and all other equipment to bypass sewer flow around the CONTRACTOR'S Work as required. CONTRACTOR shall furnish all necessary labor supervision to set up and operate the pumping system. Equipment shall be equipped with sound attenuation devices to keep noise level within limits specified in the City of Chandler Codes or stated in the Contract Documents. Bypass shall be of adequate capacity and size to handle the required flows.
- B. The pumps and bypass pipes shall be of adequate capacity and size to handle the required capacity.
 - 1. Pumping: The CONTRACTOR shall provide adequately sized pump(s) to accommodate 150 percent of the maximum instantaneous flow. CONTRACTOR shall decide the required flow capacity. The city may provide flow information if available to the CONTRACTOR but shall provide no guarantees regarding the accuracy of such data.
 - 2. The CONTRACTOR shall provide 100 percent redundant bypass pumping capability when only one pump is required to accommodate the flow. When multiple pumps are required to accommodate the bypass flows, the Contractor shall provide a minimum of 50 percent additional number of pumps.
 - 3. The Contractor shall provide fused (HDPE) piping to convey flow around the project area. HDPE pipe shall be used outside of spill containment area.
 - 4. The CONTRACTOR shall provide a full-time operator/inspector with full responsibility for the bypass pumping operation.
 - 5. In no case shall the CONTRACTOR allow any sewage to surcharge and backup into homes or businesses, or in any way overflow into the environment. If the bypass pumping capacity is insufficient to prevent surcharge and/or overflow at any time, the CONTRACTOR shall pull the line plugs to reestablish flow within the sewer irrespective of the status of the application or rehabilitation process. The OWNER shall not be responsible for or pay for any damage created by flow re-establishment.
- C. CONTRACTOR shall maintain on-site sufficient equipment and materials to ensure continuous and successful operation of the bypass and dewatering systems. The CONTRACTOR shall maintain on-site a sufficient number of valves, tees, elbows, connections, tools, sewer plugs, piping, and other parts or system hardware to ensure immediate repair or modification of any part of the system as necessary.
- D. All equipment shall be placed on a new plastic tarp, adequately sized, and bermed to protect against gasoline, oil, and hydraulic fluid spills.

PART 3 - EXECUTION

3.1 PROTECTION

- A. In areas where flows are bypassed, all bypass flow shall be discharged as described above or approved by the city or city's representative. Bypassing to the ground surface, receiving waters, storm drains or bypassing which results in groundwater contamination or potential health hazards is not permitted.
- B. Provide ENGINEER a copy of an emergency spill response plan. Plan shall address notification and clean up procedures. Immediately take action to halt and clean up all spills and immediately notify ENGINEER of any/all spills.
- C. Perform all work in compliance with OSHA standards and in no case will noise levels be permitted which would interfere with the work of the city or others. Utilize sound attenuated bypass pumps with a maximum decibel rating of 70 db or less, measured at a distance of 50 feet. CONTRACTOR shall be responsible to provide and install sound attenuation devices, methods and/or systems to maintain noise levels below stated decibels. Sound measurements shall be made and recorded by the CONTRACTOR in accordance with ANSI/ASA S12.50-2002 and provided to the city's representative upon request.
- D. Odor Control:
 - 1. Employ methods and procedures that mitigate the generation and discharge of objectionable odors to the surface environment at all times. CONTRACTOR shall be responsible for controlling hydrogen sulfide so as not to create nuisance odors and resident complaints per specification section 01570- Temporary Controls
 - 2. In areas where flows are bypassed, all bypass flow shall be discharged to a downstream sanitary sewer manhole following a bypass plan approved by the OWNER and ENGINEER. No bypassing to ground surface receiving waters, canals, storm drains, or bypassing which results in groundwater contamination or potential health hazards shall be permitted in accordance with the General Conditions.

3.2 DAMAGES

- A. The CONTRACTOR shall repair, without cost to OWNER, any damage that may result from his negligence, inadequate or improper mechanical or electrical failures.

++ END OF SECTION ++

SECTION 02146

SEWER PIPE AND SEWER STRUCTURE CLEANING

PART 1 - GENERAL

1.1 DESCRIPTION

- A. This specification section specifies the requirements for cleaning of the sewer pipe and sewer structures prior to remote visual CCTV inspection, pipe and/or manhole/structure rehabilitation. The work to be done under this Contract consists of furnishing all labor, materials, and equipment to remove accumulated sediments, clean, and preparation of the sanitary sewer pipes and/or sewer structures for rehabilitation.

1.2 SUBMITTALS

- A. The following submittal shall be provided in accordance with Technical Specification Section 01330, Submittals:
1. A letter identifying the methods the CONTRACTOR plans to employ to remove sediment, debris, grease, scale, encrustations, and roots throughout the sewer pipe to be lined and in the sewer manholes/structures to be repaired, rehabilitated or interior protective coating applied. The letter report shall include a detailed narrative explanation of the proposed cleaning process, schedule of activities, and detailed references where the CONTRACTOR has used the identified cleaning method successfully in the past.
 2. The CONTRACTOR shall submit the cleaning technique(s), nozzle, and water pressure and/or mechanical means proposed to ensure the interior circumference of the pipe and structures are clean and free from any residual sediment.
 3. The CONTRACTOR shall submit the proposed methods and procedures for preventing debris cleaned from structures and/or manholes from entering the live sewers (if present).
 4. The location of off-site disposal of all material removed from the sewer(s) along with letter of acceptance from the receiving disposal site accepting the debris.
 5. A copy of all haul permits obtained from all regulatory agencies.
 6. The CONTRACTOR shall submit a public awareness plan outlining the means and methods of proposed plans to mitigate impact to the public during the cleaning operation.

PART 2 - MATERIALS

2.1 CHEMICAL ADDITIVES

- A. No chemicals shall be used without written approval of the ENGINEER. In no case shall any chemical additive be used which might be considered hazardous or might be considered detrimental to organisms or equipment of a wastewater treatment plant, or detrimental to old or new pipe materials.
- B. A Safety Data Sheet (SDS) shall be posted onsite with a copy supplied to the ENGINEER for all chemicals anticipated to be used in the project or brought on-site.

2.2 HYDRO BLAST PIPE CLEANING-NOZZLE

- A. The CONTRACTOR shall use a nozzle to ensure the interior 360-degree circumference of the pipe interior is clean and free from any residual sediment prior to lining.
- B. It shall be the CONTRACTOR's responsibility to determine whether the structural condition of the existing pipe will not be detrimentally damaged by use of the spinning nozzle and water pressure to be utilized. The CONTRACTOR shall notify the ENGINEER of pipe condition that would be impacted through the proposed cleaning process and equipment.

2.3 SEWER STRUCTURE CLEANING – HYDRO BLAST

- A. The CONTRACTOR shall use high pressure hydro blasting methods as appropriate and necessary to remove deteriorated concrete and any previously applied protective coatings/linings on the interior of manholes/structures down to solid/sound substrate material.
- B. The CONTRACTOR shall use debris shields or other approved method to prevent debris dislodged from the interior of the manhole/structure from entering the live sewers. Discharge of removed sediment and debris into the interceptor pipe is prohibited. The CONTRACTOR shall remove and legally dispose of dislodged debris offsite. All hauling equipment shall be watertight to prevent spillage during transport. The CONTRACTOR will be responsible for interceptor pipe cleaning and removal of all sediments to a disposal facility.

PART 3 - EXECUTION

3.1 GENERAL

- A. The CONTRACTOR shall conduct work to prevent any detrimental sewage blockage and minimize surcharging in the sewer manholes/structure or connecting sewer pipes.

- B. It shall be the CONTRACTOR's responsibility to determine whether the structural condition of the existing pipe, manhole, and/or structure will not be detrimentally damaged as a result of any proposed cleaning process(es). Damage to existing facilities as a result of the CONTRACTOR's work shall be promptly repaired in kind at the CONTRACTOR's expense. The CONTRACTOR accepts the responsibility of reinstating normal sewer operation once the proposed cleaning operation is initiated.
- C. It is CONTRACTOR's responsibility for all costs incurred with removal of sediment if observed to be discharged into an active interceptor sewer for a minimum distance of one (1) mile upstream and downstream of the work area location.

3.2 SEWER BYPASSING AND DEWATERING

- A. Sewer flow shall be bypassed during the cleaning as specified in Technical Specification Section 02145, Diversion of Sewage Flow and Dewatering if required, or directed by the ENGINEER.

3.3 HYDRO BLAST CLEANING OF PIPES

- A. The CONTRACTOR shall clean and remove existing sediment, debris, roots, scale, encrustations, grout, and FOG accumulations from the sanitary sewer pipes to be inspected or scheduled to be rehabilitated, to adequately prepare the surfaces for liner installation or protective surface application.
- B. Pipe discovered by the CONTRACTOR or his Subcontractor to be physically deteriorated to the extent where cleaning may detrimentally impact the structural integrity shall be brought to the attention of the ENGINEER prior to conducting cleaning operations.
- C. The CONTRACTOR shall clean the pipe to ensure proper installation of the sewer liner. Cleaning shall remove all sediment, rocks, debris, roots, FOG, accumulations, and obstructions from the sewer to be lined. Multiple passes with cleaning equipment may be required at no additional cost to the OWNER. Once the CONTRACTOR initiates the cleaning operation, they accept the responsibility of reinstating normal operation of the sewer.
- D. Sediment or debris from cleaning operations larger than U.S. #8 sieve shall not be deposited downstream in the sewer. Sedimentation deposited downstream shall be removed at no cost to the OWNER. All materials dislodged during cleaning shall be removed from the pipe and delivered to an approved disposal site.
- E. The CONTRACTOR shall be thoroughly familiar with all phases of sewer pipe cleaning to ensure the completion of this Contract without causing a health hazard or damage to the sewage system, public and private property.

- F. It shall be the responsibility of the CONTRACTOR to repeat the cleaning process if root re- growth, FOG accumulation, or other detrimental obstructions are present between the time pre-video inspection and CIPP liner installation operation at no additional cost to the OWNER.

3.4 MECHANICAL CLEANING OF PIPES

- A. The CONTRACTOR shall clean existing interior pipe of tuberculation, hardened FOG, scale, encrustations, root intrusions, and grout accumulations from the sanitary sewer pipes to be inspected or scheduled to be rehabilitated and adequately prepare the surfaces for installation of liner.
- B. Pipe discovered by the CONTRACTOR or Subcontractor to be physically deteriorated to the extent where cleaning may detrimentally impact the structural integrity of the existing pipe shall be brought to the attention of the ENGINEER prior to conducting cleaning operations.
- C. Cleaning shall remove all mineral accumulations and/or attachments from corrosion of metallic surfaces from the sewer to be lined and from the structures to be coated (if applicable). Cleaning of the sewer and structure interior surfaces shall remove all scale, encrustation, and any other foreign material from the surface.
- D. The CONTRACTOR shall mechanically clean the pipe interior to within 95% of original interior pipe diameter as verified through interior pipe CCTV optical laser profiler or approved equal equipment to verify and document the internal pipe diameter prior to ordering and installing CIPP lining. Multiple passes with mechanical cleaning equipment may be required at no additional cost to the OWNER.
- E. It shall be the responsibility of the CONTRACTOR to repeat the mechanical cleaning process if tuberculation, FOG, or detrimental corrosion buildup occurs prior to CIPP lining operation at no additional cost to the OWNER.

3.5 HYDRO BLAST CLEANING OF MANHOLES/STRUCTURES

- A. The CONTRACTOR shall clean existing trough, throat, bench, wall, cone and chimney of debris, encrustations, FOG, and deteriorated concrete by the use of high-pressure hydro blasting. Once cleaning has commenced, the rehabilitation method chosen shall be completed in succession with no more than 48 hours transpiring between cleaning and rehabilitation activities.
- B. Access manholes/structures discovered by the CONTRACTOR or Subcontractor to be physically deteriorated to the extent where cleaning may detrimentally impact the structural integrity of the existing access manhole/structure shall be brought to the attention of the ENGINEER prior to conducting cleaning operations.

- C. Cleaning shall remove all deteriorated concrete such that the remaining surface is sound and free of unstable material.

3.6 PVC "T-LOCK" LINED PIPES

- A. The CONTRACTOR shall clean the interior of any PVC lined pipe per paragraph 3.3 - Hydro- Blast Cleaning of Pipe in conjunction with the following according to the proposed pipe rehabilitation method proposed.
- B. Rehabilitation Method: Cured-In-Place-Pipe Lining Installation:
 - 1. The CONTRACTOR shall remove existing PVC liner and/or joint material that is loose or hanging that may impact the liner installation process along with any hardened encrustations or protrusions that may impact the final diameter profile of the rehabilitated pipe.

3.7 RECLEANING

- A. Recleaning of the sewer pipe/manhole/structure may be required prior to the remote visual pipe inspection preceding pipe rehabilitation at no additional cost to the OWNER.

3.8 REMOTE VISUAL INSPECTION

- A. Cleaning of the sanitary sewer pipe is required prior to remote visual inspection of the sewer pipe to view surface and service lateral conditions (if present) per Technical Specification Section 02147, Remote CCTV Inspection Of Existing and Rehabilitated Sewers.

3.9 DISPOSAL OF SEDIMENTS

- A. The CONTRACTOR shall be responsible for transporting and disposing, including all disposal fees, of any sediments and material removed from the sewer or structures.
- B. All sediment and debris removed from the sewer system and associated appurtenances shall be disposed into a dump bin on-site which would be hauled to Butterfield landfill in a lawful manner. Hauling containers shall be watertight. On-site stockpiling of removed material will not be permitted.
- C. The CONTRACTOR shall be responsible for obtaining all necessary permits, payment of all fees, and approval from all regulatory agencies required to perform the work, including transport of sediments. Off-site disposal of all material removed from the sewer shall be the CONTRACTOR's responsibility.

++ END OF SECTION ++

SECTION 02147

REMOTE CCTV INSPECTION OF EXISTING AND REHABILITATED SEWERS

PART 1 - GENERAL

1.1 DESCRIPTION

- A. This Technical Specification defines the requirements for internal remote Closed-Circuit Television (CCTV) inspection of the existing sewer pipes and access manholes/structures before and after rehabilitation according to the National Association of Sewer Service Companies (NASSCO) standards. Interior pipe and manhole/structure condition (post video inspection) shall be documented following rehabilitation using digital scanning camera equipment.

1.2 SUBMITTALS

- A. Preliminary Submittals:
1. At the preconstruction conference the CONTRACTOR shall submit the following information for review per Specification Section 01330, Submittals following notification of award of the Contract:
 - a. The CONTRACTOR shall submit operator name and NASSCO Pipeline Assessment and Certification Program (PACP) certification number and Manhole Assessment Certification Program (MACP) certification number of proposed CCTV operator for approval. CONTRACTOR shall promptly provide substitute operator and credentials in the event operator approval is denied.
 - b. The CONTRACTOR shall submit an example of work of the proposed camera equipment to demonstrate quality and clarity of video along with proposed methods of documentation. Example shall consist of one (1) remote visual inspection record in proposed format of previous sewer inspection work completed along with associated audio commentary and inspection log(s). The submitted record shall show operational and structural defects in sewers that are of the same diameter as the sewers in this project.
 - c. The information will be reviewed to determine if the quality of the images, technique, and inspection procedures are acceptable and if defects were properly identified and documented. Samples shall be with the same camera, lighting equipment, and data handling procedures proposed for this work.
 - d. The OWNER and/or ENGINEER will review the records, not for accuracy of content, but to make sure that the required information is provided, and the recording is of acceptable quality. If the OWNER and/or ENGINEER determines that the records are defective or not of adequate quality, the CONTRACTOR shall repeat the inspection documentation at the CONTRACTOR's expense.

- e. The CONTRACTOR shall be responsible for modifications to his equipment and/or inspection procedures to achieve report material of acceptable quality. No work shall commence prior to approval of the material by the OWNER and/or ENGINEER. Once accepted, the report material shall serve as a standard for the remaining work.
- f. The CONTRACTOR shall resubmit a final video showing a post inspection after the liner issue(s) have been addressed and approved, and after the manhole rehabilitation is complete and approved. CONTRACTOR shall correct all title information on the video and labels prior to final submittal.
- g. Universal Serial Bus (USB) or external hard drive records shall include labels containing project titles, dates, including OWNER and CONTRACTOR company names. The CONTRACTOR shall maintain a copy of all inspection documentation (disks, tapes, databases, and logs) for the duration of the work and warranty period.

B. Submittals:

- 1. Pre-Construction Inspections: One (1) copy of the pre-construction CCTV inspection stored on external hard drive or USB media, showing the existing sewer pipe after preliminary cleaning, shall be submitted to the OWNER or ENGINEER a minimum of 14 days prior to CIPP lining. The distance to each sanitary sewer lateral shall be recorded on the inspection tape and paper copy record. Each pipe segment to be rehabilitated shall be CCTV inspected and recorded. Multiple pipe segment inspections can be stored on one DVD or USB. If the OWNER or ENGINEER determines that the records are defective, not of adequate quality, not containing pipe identification information, or footage counter, the CONTRACTOR shall repeat the inspection documentation at the CONTRACTOR's expense.
- 2. One (1) copy of the CCTV inspection stored on external hard drive or USB media or external hard drive, showing the existing sewer pipes after cleaning, just prior to CIPP installation, along with a final inspection video (to be panoramic) following liner installation and any lateral opening reinstatement. Videos shall be submitted to the OWNER or ENGINEER within one (1) day of collecting the final inspection video. The OWNER or ENGINEER will track and compile a final record of initial cleaning, pre-lining CCTV, post-lining CCTV, and if necessary secondary post-lining CCTV after any liner issues have been addressed and submitted to the OWNER or ENGINEER by the CONTRACTOR for work acceptance.
- 3. One (1) copy of the CCTV inspection stored on external hard drive or USB media or external hard drive, showing the existing access manhole/structure after cleaning, just prior to the repair material installation along with a final inspection video (to be panoramic) after rehabilitation. Video shall be submitted to the OWNER or ENGINEER within one (1) day of the final inspection video collection. The OWNER or ENGINEER will track and compile a final record of initial cleaning, pre-rehabilitation CCTV, post-rehabilitation CCTV, and if necessary secondary post-rehabilitation CCTV after any issues have been addressed and submitted to the OWNER or ENGINEER by the CONTRACTOR for work acceptance.

4. The CONTRACTOR shall submit written inspection logs and CCTV procedures according to NASSCO PACP for the sewer pipes inspected and/or rehabilitated.
5. The CONTRACTOR shall submit written inspection logs and CCTV procedures according to NASSCO MACP standards for the sewer access manholes/structures inspected and/or rehabilitated.

PART 2 - PRODUCTS

2.1 MATERIALS

A. Television Inspection Camera(s):

1. Camera(s) shall be intrinsically safe and shall be capable of safely operating in 100 percent humidity conditions. Lighting intensity shall be remote controlled and shall be adjusted to minimize reflective glare. Lighting and camera quality shall provide a clear, focused picture of the entire inside periphery of the sewer. Analog type camera shall have tilt and zoom capabilities to view 90 degrees to the pipe centerline for the entire 360 degrees of the pipe circumference. Digital type cameras shall possess forward and rear facing cameras with 360-degree view capabilities.
2. The CONTRACTOR shall utilize equipment that allows for CCTV to be performed within conditions noted in the Technical Specification. CONTRACTOR's equipment shall be sized to fit and maneuverable within final diameter of rehabilitated pipe segments. Any scrapes or damage to the interior pipe liner or newly installed protective manhole/structure coating or lining resulting from equipment operation shall be addressed and corrected by the CONTRACTOR at no additional cost to the OWNER.
3. The CONTRACTOR shall utilize a digital, Panaramo 360 or approved equal for post CCTV inspection of pipes 12-inch diameter and larger and for use in manholes/structures. Either digital or analog CCTV equipment is permitted for use in pipes less than 12-inches in diameter.

B. Video Storage Devices:

1. Video recordings of all sewer line inspections shall be made on USB media or digital external hard drive. The audio portion of the video shall be sufficiently free from electrical interference and background noise to provide complete intelligibility of the oral report. A copy of the video recordings shall be consolidated on USB or digital external hard drive media with corresponding OWNER unique appurtenance quarter section number.

C. Footage Counter:

1. A footage counter device, which measures the distance traveled by the camera in the sewer, shall be accurate to plus or minus 2 feet in 1,000 feet.

D. Video Titling:

1. Video equipment shall include genlocking capabilities to the extent that computer generated data, (e.g., footage, date, size, address and location, etc.) as determined by the OWNER can be overlaid onto video, and both indicated on the television monitor and permanently recorded on the inspection video.

PART 3 - EXECUTION

3.1 FLOWS IN SEWERS

- A. The CONTRACTOR shall provide temporary dry conditions in the sewer pipes during remote visual CCTV inspections and continuously until completion of liner installation and final documentation CCTV video inspection. The bypassing requirements are provided in Technical Specification Section 02145, Diversion of Sewage Flow and Dewatering.

3.2 CLEANING

- A. The remote visual inspection shall be performed after cleaning the sewer pipe or access manhole as specified in Technical Specification Section 02146, Sewer and Sewer Structure Cleaning, and prior to lining the pipe. The CONTRACTOR shall be responsible for properly inspecting and documenting the final condition of the rehabilitated pipe.

3.3 SCHEDULE

- A. The CONTRACTOR shall submit post CCTV of segments within one (1) working day from the date of installation. The CONTRACTOR shall also be responsible to produce and submit electronic inspection logs of the inspection process.

3.4 SEQUENCE OF INSPECTION

- A. The sewer pipe shall be remote visually inspected, in the same direction, prior to and following the lining of the sewer. Final CCTV inspection shall be performed after lateral reinstatement has been completed.

3.5 INSPECTION METHODS

- A. The CONTRACTOR shall inspect the sewer interior using a camera or process capable of producing a color image for permanent record and documentation of inspection in digital format (mpeg or jpeg) with audio location and date information, data title information.
- B. Verbal Commentary:
 1. The CONTRACTOR shall audio record narrative on the remote visual inspection location, upstream and downstream control points, date, and time of the inspection.

C. Access:

1. The OWNER or ENGINEER shall have full access to observe and monitor all other operations at all times. The system of cabling employed to transport the camera and transmit its signal shall not obstruct the camera's view.

D. Inspection Rate:

1. The camera shall be pulled through the sewer in either direction, but all inspections at each location shall be in the same direction. Maximum rate of travel shall be 30 feet per minute when recording with analog camera systems.

E. Image Perspective:

1. The camera image shall be down the center axis of the pipe when the camera is in motion. The CONTRACTOR is required to provide a 360-degree view of the pipe interior. Points of interest shall also be documented and shall include, but not be limited to, defects, encrustations, mineral deposits, debris, sediment, any location determined not to be clean or part of a proper liner installation, or defects in the liner (including, but not limited to, bumps, folds, tears, dimples, etc.).
2. Each individual sanitary sewer lateral opening to the sewer main shall be remote visually inspected. Camera shall first pan and tilt to the lateral opening prior to zoom for analog camera systems. Full 360-degree views of the lateral opening shall be documented without zoom. CONTRACTOR shall not utilize pan, tilt and/or zoom simultaneously.

F. Sewer Identification:

1. All inspection documentation shall include the sewer location identified by OWNER quarter section manhole numbers.

G. Sewer Flow Control:

1. During remote visual inspections, and continuously until completion of liner installation and final CCTV documentation inspection, the CONTRACTOR shall provide temporary dry conditions in the sewer pipes. The bypassing requirements are provided in Technical Specification Section 02145, Diversion of Sewage Flow and Dewatering.

H. Quality Control:

1. The OWNER's Representative will review USB or external hard drive to ensure compliance with the requirements listed in this Technical Specification and Contract Documents. If the sewer pipe or access manhole is determined not to be adequately cleaned, as required in Technical Specification Section 02146, Sewer and Sewer Structure Cleaning it shall be recleaned and remote visual inspection repeated by the CONTRACTOR at no additional cost to the OWNER. If any portion of the liner or manhole rehabilitation is determined not acceptable, the liner shall be repaired or replaced, whichever the OWNER's Representative deems appropriate, and re-inspected and documented.

CITY OF CHANDLER: Public Works and Utilities Department
PROJECT NAME: Loop 101 and 202 Freeway 66-inch Redundant Sewer Project
PROJECT NUMBER: WW2402.401

++ END OF SECTION ++

SECTION 02148

CIPP LINING OF EXISTING SEWERS

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. This Technical Specification Section specifies the Cured-In-Place-Pipe (CIPP) rehabilitation lining, sets acceptable material and experience parameters, defines minimum performance requirements for the finished, installed product and details testing requirements for the completed product.

B. Requirements:

1. The CONTRACTOR shall provide and install a resin saturated flexible tube (lining/liner) with an interior plastic coating wearing surface into pipes identified for CIPP lining in accordance with ASTM International D5813 – Type II (fully deteriorated), (ASTM) F1216, ASTM F2019, and ASTM F1743 for the inversion installation method with resin cured by exposure to heated water or using pull-in place method with Glass Reinforced Plastic (GRP) cured by exposure to UV-light. When cured, the CIPP lining shall be continuous and tight fitting.

1.2 CLIMATIC CONDITIONS

- A. The CONTRACTOR is made aware that the Work may occur during the summer months with temperatures exceeding 115 degrees F. The CONTRACTOR shall be prepared for these conditions and alter catalyst, installation procedures, or provide protection from detrimental exposure to adverse temperature consistent with the materials proposed for use.
- B. Severe thunderstorms could occur that may increase volume of flow present within the sewer system. The CONTRACTOR shall be prepared for these conditions and alter the installation procedures and/or provide bypass pumping capabilities accordingly. The CONTRACTOR shall be responsible for review of the weather forecast prior to commencement of liner installation that restricts or impedes the flow capacity of the sewer pipe being rehabilitated. When the anticipated weather conditions are such that the resulting sewer flow may exceed upstream sanitary system storage volume or the capacity of the bypass pumping system or otherwise adversely affect the CIPP liner installation then the ENGINEER shall be informed, and the installation shall be delayed until favorable weather is forecasted.

1.3 EXPERIENCE RECORD OF THE CONTRACTOR

- A. The CONTRACTOR shall provide to the satisfaction of the OWNER written documentation from the CIPP lining manufacturer that the CONTRACTOR's personnel have been adequately instructed and trained in the installation procedures of the CIPP liner process proposed. Such written documentation shall describe manufacturer's procedures for the installers, safety instructions, training program, identification, and protocol of a course of action(s) in the event installation deviates from recommended procedures and, if applicable, licensing policies.
- B. The CONTRACTOR or its CIPP liner subconsultant, shall have successfully managed, in addition to an experienced onsite superintendent possessing a minimum of 5,000 linear feet of CIPP liner installation experience on projects utilizing similar materials, installation techniques, and CIPP liner diameter like those proposed on this Project within the previous three (3) years of the bid date of this project.

1.4 QUALITY ASSURANCE REFERENCES

- A. This Technical Specification section contains references to the following documents. In case of conflict between the requirements of this section and those of the listed documents, the requirements of this section shall prevail.
 - 1. AASHTO Loading, American Association of State Highway and Transportation Officials - Anticipated external load to be placed on the pipe.
 - 2. ASTM F412, Standard Terminology Relating to Plastic Piping Systems.
 - 3. ASTM F1216 (including Appendix XI): Standard Practice for Rehabilitation of Existing Pipelines and Conduits by the Inversion and Curing of a Resin-Impregnated Tube.
 - 4. ASTM F3240: Standard Practice for Rehabilitation of Existing Pipelines and Conduits by the Inversion and Curing of a Resin-Impregnated Tube.
 - 5. ASTM F3240: Standard Practice for Installation of Seamless Molded Hydrophilic Gaskets (SMHG) for Long- Term Watertightness of Cured-in-Place Rehabilitation of Main and Lateral Pipelines
 - 6. ASTM C58: Standard Practice for Determining Chemical Resistance of Thermosetting Resins used in Glass-Fiber Reinforced Structures, Intended for Liquid Service.
 - 7. ASCE MOP145: Design of Close Fit Liners for the Rehabilitation of Gravity Pipes
 - 8. ASTM F2019: Standard Practice for Rehabilitation of Existing Pipelines and Conduits by the Pulled in Place Installation of Glass Reinforced Plastic Using the UV-Light Curing Method
 - 9. ASTM D543: Standard Practice for Evaluating the Resistance of Plastics to Chemical Reagents.
 - 10. ASTM D638: Standard Test Method for Tensile Properties of Plastics.
 - 11. ASTM D790: Standard Test Methods for Flexural Properties of Unreinforced and Reinforced Plastics and Electrical Insulating Materials.

12. ASTM D883: Standard Terminology Relating to Plastics. ASTM D1600 Standard Terminology for Abbreviated Terms Relating to Plastics.
13. ASTM D2990: Standard Test Methods for Tensile, Compressive, and Flexural Creep and Creep- Rupture of Plastics.
14. ASTM D5813: Standard Specification for Cured-in-Place Thermosetting Resin Sewer Piping Systems.
15. ASTM D3039 / D3039M: Standard Test Method for Tensile Properties of Polymer Matrix Composite Material.
16. ASTM E797: Standard Practice for Measuring Thickness by Manual Ultrasonic Pulse-Echo Contact Method.
17. NASSCO: National Association of Sewer Service Companies

1.5 SUBMITTALS

- A. The following Submittals shall be provided in accordance with Technical Specification

Section 01330, Submittals:

1. Shop drawings which detail short- and long-term properties (providing all supporting test data) of all component materials, construction procedural requirements for material for material handling, storage, sunlight control (if applicable), temperature control (if applicable), and any other CIPP liner handling/installation constraints.
2. Submit contractor and liner manufacturer qualifications.
3. Representative cured liner sample (1-foot in length) or sample plate for large diameter (1-foot square) for each diameter size of the same resin and CIPP tube components proposed for the project depicting the final quality of workmanship that can be expected on this project.
4. Structural CIPP liner calculations per the design parameters stated in paragraph 2.5. CIPP liner calculations shall be professionally sealed by a Civil Engineer licensed in the State of Arizona.
5. 10,000-hour third party, 50-year Flexural Creep Modulus test data. The test shall be in accordance with ASTM D2990 at 10,000 hours. If results of a 10,000-hour tests are not available, the CONTRACTOR shall use a minimum 50 percent reduction (50 percent retention) of Flexural Modulus of Elasticity (per ASTM F1216) for all formula calculations.
6. Independent laboratory test report(s) of CIPP sample(s) and tests as specified in 02148, paragraph 2.2 and 2.3. Chemical resistance and GRP/resin properties testing shall be performed on a previously prepared sample of the finished product proposed for the project. A Certified Affidavit, signed by an officer of the CIPP liner installation company, shall be provided stating that the resin/GRP the test applies to, and the resin/GRP submitted for this project are the same.
7. Independent laboratory test report(s) of CIPP sample(s) and tests as specified in paragraph 1.6.B. Testing shall be performed on prepared samples of the finished product installed as part of this project. A copy of the completed Chain of Custody

- form shall be submitted to the ENGINEER along with independent laboratory test results.
8. Detailed method(s), locations, and procedures, for creating a representative CIPP sample on-site for independent laboratory testing.
 9. Certified Test Results for abrasion resistance in accordance with DIN EN 295-3 / DIN 19565.
 10. Certification from the manufacturer that the resin, carrier felt and CIPP tube material(s) supplied complies with ASTM D5813 and meets the intended service condition(s) set forth in this Technical Specification.
 11. Manufacturer's recommended installation procedures per ASTM F1216 and ASTM F2019.
 - a. If the "inversion" method is employed per ASTM F1216, provide maximum inversion head (psi for air inversion, or feet of head for water inversion) pressure and proposed method of monitoring/documenting pressures.
 - b. If the "pull-in place" method is employed per ASTM F2019, provide maximum blower air pressure and proposed method of monitoring/documenting pressures.
 12. Cure information from the resin/GRP manufacturer for the properties of the materials proposed for use on this project, including recommended method(s) of application of heat/UV, the medium that initiates the curing process. Recommended step "cooking" temperatures and hours at each stage including maximum rate (degrees per hour) of liner heating/cooling, target maximum temperature, duration of target cure temperature, acceptable fluctuation in target temperature with description of remedial actions to be taken if fluctuations occur, and the lower target temperature when the controlled temperature curing process is terminated, and the heated water is released from the liner interior or the UV light train is pulled out. CONTRACTOR shall provide written plan for installation crew use in the event manufacture recommended temperature is not attained in a portion of the liner.
 13. Certification showing the CONTRACTOR is currently licensed by the appropriate licenser to perform CIPP installation materials and processes proposed. Certification shall be given to the ENGINEER before any materials are delivered to the job site.
 14. A certified affidavit, signed by an officer of the installation Company, stating, and certifying that the on-site superintendent has received proper training in the manufacturer's recommendations for CIPP liner installation methods and procedures.
 15. Proposed method of measuring dimensional size and height of observed defects and an outline of specific repair or replacement procedures as recommended by the CIPP manufacturer for defects that may occur in the installed liner. Potential defects within the CIPP liner that cannot be repaired in-place shall be clearly defined by the CONTRACTOR based on the manufacture's recommendations shall be accompanied by a compensation-in-lieu of repair proposal submitted to the OWNER for potential consideration. Acceptance of the proposal shall not be binding on the OWNER.
 16. Certification stating CIPP tube has been manufactured in accordance with ASTM F1216 and resin ASTM D5813 is suitable for its intended use.

17. Certification stating CIPP GRP has been manufactured in accordance with ASTM F2019 and ASTM D578 and is suitable for intended use.
18. Manhole connection (liner termination) detail and material proposed to seal any annular space between the liner and host pipe. Sealing product manufacturer's literature and technical data for provided products. A preformed hydrophilic expanding material, able to with-stand hydration and de-hydration cycles in sanitary sewer service, shall be used between the liner and host pipe at all locations where the CIPP enters and exits the pipe at manholes or similar access points
19. Warranty information and certificates.
20. Safety Data Sheets (SDS) for all hazardous chemicals used or expected to be on-site. At a minimum, sheets for the resin, catalyst, cleaners, and repair agents should be submitted.
21. Proposed equipment and installation methods for fiber optic cable temperature monitoring system cable for documenting the wall temperature during cure along the entire length of the CIPP liner being installed. CONTRACTOR shall provide intended backup method(s) for temperature monitoring in the event of fiber optic system failure.
22. Baseline resin spectrograph for resin material proposed.
23. 24-hour written notification of cure water release (if applicable).
24. Remote visual inspection video and reports as specified in paragraph 3.1.D for each installation including pre-inspection, installation curing protocol and post-installation.
25. Contractor shall submit a repair plan to OWNER or ENGINEER should corrections to the Work be required.

1.6 INSTALLED CIPP LINER QUALITY CONTROL

A. Affidavit Of Sample Authenticity:

1. A certified affidavit, signed by a representative of the installation company, shall be provided stating that the samples collected, and the samples submitted to the laboratory for testing are the same and apply to the location intended.

B. Laboratory Test Sample:

1. The CONTRACTOR shall prepare samples of the CIPP liner installed as part of this project for subsequent testing of its physical properties. Previous test data will not be acceptable. Samples shall be prepared and tested using the flat plate sampling or clamped mold method in accordance with the procedures of ASTM F1216. The sample will be constructed of the same materials (tube and resin/catalyst) as is used for that given liner installation held in place by a suitable heat sink. The flat plate or molded sample shall be large enough to provide five (5) sample specimens each.
 - a. The CONTRACTOR shall prepare a redundant sample for potential use in the event primary test samples are lost or damaged during transport. Redundant samples shall be labeled, initialed, and dated by the ENGINEER, protected, and retained by the CONTRACTOR until such time as acceptable liner test results are received by the ENGINEER. The CONTRACTOR shall inform the

ENGINEER in the event redundant samples are to be sent to the testing laboratory. The ENGINEER shall verify redundant samples identification prior to shipment. CONTRACTOR shall obtain authorization from the ENGINEER prior to disposal of redundant samples.

2. The cured sample shall be sent to and tested by an independent testing laboratory, as recommended by the CIPP liner manufacturer, and approved by the ENGINEER. The CIPP liner samples taken shall be sent to an independent laboratory within three (3) days of collection. The samples are to be signed by the ENGINEER and accompanied by a completed Chain of Custody form. Results of laboratory testing shall be submitted directly to the ENGINEER within three (3) working days of receipt by the CONTRACTOR.
 - a. Final payment for the CIPP liner installation will not be made until the test results are received and approved by the ENGINEER. The CONTRACTOR shall be responsible for any deviation from the specified physical properties and those evaluated through testing. Failure to meet the specified physical properties will result in the CIPP liner being considered defective work which will be handled in accordance with MAG Section 106. The CONTRACTOR shall be responsible for all costs associated with the testing of the liner's physical properties.
3. Samples taken for testing shall be individually labeled and logged to record the following:
 - a. OWNER name.
 - b. OWNER's project number and project title.
 - c. Unique sample number.
 - d. Pipe segment number as noted on Drawings or Pipe Rehabilitation List.
 - e. Date and time sample was taken.
 - f. Name of CONTRACTOR.
 - g. Date, location, and name of person by whom the sample was taken.

Updated copies of the test log shall be submitted to the ENGINEER at the regularly scheduled coordination meeting.

4. Test Sample Frequency: The above-stated sampling shall be performed for each manufacturing lot of CIPP liner materials (per diameter size).
 - a. A sample shall be taken every 3,000 lineal feet or one semi-trailer load of liner whichever is less. The ENGINEER reserves the right to request the CONTRACTOR to perform one (1) additional sample test for every 3,000 lineal feet or one semi-trailer load of liner whichever is less.
 - b. If felt liner is saturated with resin on-site, sampling frequency shall be per each pipe segment rehabilitated. The ENGINEER reserves the right to request the CONTRACTOR to perform one (1) additional sample test per pipe segment installed.
5. Grounds For Installed Liner Rejection:

Grounds for rejection of installed CIPP liner include, but not limited to the following:

 - a. CIPP sample sent to a laboratory that is missing the ENGINEER's signature.

- b. CIPP samples missing the Chain of Custody form.
- c. Chain of Custody form missing information or signatures of all those who handled or processed the sample, including the signature of the laboratory Manager responsible for performing the quality control testing.
- d. Laboratory test result(s) do not meet minimum Technical Specification requirements.

C. Sample Labeling:

Each liner tube sample shall be labeled with: CONTRACTOR Name, Date, Project Name, OWNER's Unique Pipe Identification Number, Host Pipe Diameter. Documentation shall also include the following:

- 1. Liner manufacturer's name.
 - 2. Location of the liner manufacturer.
 - 3. Felt supplier.
 - 4. Resin supplier.
 - 5. Resin identification number.
 - 6. Resin weight – lbs/ft.
 - 7. GRP Supplier in case of UV cured-in place pipe
 - 8. Manufactured liner length both resin saturated felt “wet” and dry felt “dry”.
 - 9. Liner thickness.
 - 10. Liner diameter.
 - 11. Date of resin saturated into felt liner.
 - 12. Unique identification number of liner.
7. ASTM F2019 reference: Section 7, X2, X3, modified test procedure to ASTM D790 and ASTM D2990 for GRP-CIPP UV-cured liner. Prior to construction, the CONTRACTOR shall submit certified type test reports of long-term and short-term flexural modulus of elasticity, flexural strength (bending stress). Tested samples shall meet the liner manufacturer's declared values or exceed the minimum values of Table 1 shown in ASTM F2019.

1.7 CIPP LINER HANDLING:

- A. The CONTRACTOR shall exercise adequate care during transportation, handling, and installation to ensure the CIPP material is not torn, cut, or otherwise damaged. If any part or parts of the CIPP material becomes torn, cut or otherwise damaged before or during insertion, it shall be repaired or replaced in accordance with the manufacturer's recommendations and approval by the ENGINEER before proceeding further, and at the CONTRACTOR's expense.

1.8 WARRANTY

- A. The work performed under this Project shall be warranted to be free from defects in workmanship and material for a period of two **(2) years** from the date of Final Acceptance by the OWNER. If the OWNER or ENGINEER determines that the process/material/installation has failed during the warranty period, the CONTRACTOR shall perform any and all repairs, including ancillary costs associated with performing the repair at no additional cost to the OWNER.
- B. The warranty period of any part of the Work repaired or replaced under the provisions of the warranty shall be extended for **one (1)** additional year.

1.9 LINER MANUFACTURE INSPECTION

- A. The CONTRACTOR /manufacturer of CIPP liner shall allow access to either the on-site or remote liner wet-out facility to observe the manufacturing/saturation process of the CIPP liner by the OWNER and/or the ENGINEER. The ENGINEER, while not acting as quality control agent for the CONTRACTOR, shall be allowed to view and document any portion of this contract work, including but not limited to verifying type and quantities of resin used at any point during manufacturing/installation or performance of this work.

PART 2 - PRODUCTS

2.1 GENERAL

- A. All materials and installation procedures provided by the CONTRACTOR for use in the CIPP installation process shall be equal to or exceed the requirements of ASTM F2019, ASTM F1216, and ASTM D2990.
- B. The CONTRACTOR shall be responsible for control of all material and process variables to provide a finished CIPP possessing the minimum properties specified in ASTM F1216 and ASTM 5813 or ASTM F2019 and D578 and/or supplemented herein.

2.2 COMPONENT PROPERTIES

- A. Felt Carrier Material:
 - 1. The liner tube carrier material shall consist of one or more layers of flexible needled felt or an equivalent non-woven and/or woven material meeting ASTM 1216 Section 5 or ASTM F1743 Section 5.2.1 and capable of carrying resin, withstanding installation pressures, and curing temperatures, and is compatible with the resin system used. The carrier material shall be fabricated to a size that, when installed, will be tight fitting to the internal circumference of the existing sanitary sewer host main. Allowances shall be made for circumferential stretching due to host pipe manufacturing tolerances and/or extent of deterioration of existing pipe walls.

2. The minimum material length shall be that deemed necessary by the CONTRACTOR to span the distance effectively and continuously from the inlet interior wall face to the outlet interior wall face of the respective manholes, unless otherwise specified. The CONTRACTOR shall verify the lengths in the field before felt liner saturation and installation of the CIPP tube. Individual insertion runs may be made over one or more manhole sections as determined in the field by the CONTRACTOR and approved by the ENGINEER.
3. Prior to insertion, the carrier material shall be free of all visible tears, holes, cuts, foreign materials, and other defects.
4. The CONTRACTOR shall provide manufacturer's data on the maximum allowable stresses and allowable elongation of the carrier material. Insertion forces imposed shall not exceed the allowable stress of the material. The overall elongation of the CIPP tube shall be measured after pull-in completion.

B. Resin:

1. The resin system to be used shall be manufactured by an approved company selected by the CIPP system manufacturer. Only corrosion resistant polyester or vinyl ester resins shall be used.
2. The resin used shall produce a properly cured-in-place pipe system appropriate for conditions encountered, which will be resistant to abrasion caused by solids, grit, and/or sand. The cured-in-place pipe system shall also be resistant to corrosion due to acids and gases such as sulfuric acid, carbonic acid, hydrogen sulfide, methane, and carbon monoxide. The cured-in-place pipe system utilized shall withstand the corrosive effect of the existing residential, commercial, and industrial effluents, liquids and/or gases.
3. The physical properties stated in paragraph 2.3 apply to CIPP manufactured polyester or vinyl ester resin. Resin shall be tinted for visibility and provide positive indication of adequate liner wet-out. The CONTRACTOR shall not change resin manufacturer or type without written approval obtained from the ENGINEER. Written request shall be accompanied by manufacturer's documentation per Technical Specification Section 01330 – Submittals for approval prior to use.

C. Glass Reinforced Plastic:

1. The manufacturer shall furnish a general-purpose polyester or vinyl ester ultra-violet light curing resin and catalyst system compatible with the ultra-violet light curing process that provides cured physical strengths specified herein if UV light is used for curing.
2. For Light-Cure products, the fiberglass shall be Environmentally Corrosive Resistant (ECR) fiberglass and the finished ultra-violet light cured fiberglass pipe liner shall be fabricated from materials which when complete are chemically resistant to and will withstand internal exposure to domestic sewage and temperatures up to 150 F.
3. All ultra-violet light cured-in-place fiberglass lining products shall comply with ASTM F2019 or the intent thereof as determined by the Engineer, minimum finished liner thickness as defined by design calculation.

4. The liner shall be provided with interior and exterior films to prevent resin migration and contamination. Films shall be in accordance with ASTM F2019 and shall be a styrene gas barrier. Inner films shall be fully removed during the installation process unless it is a permanent part of the system and fabricated as an integral part of the fabric tube by bonding or fusing. Exterior films shall block ultra-violet light.
5. The interior and exterior films shall be free from tears, holes, cuts, foreign materials and other surface defects.
6. The layers of the cured CIPP shall be uniformly bonded. It shall not be possible to separate any two layers with a probe or point of a knife blade so that the layers separate cleanly or the probe or knife blade moves freely between the layers; nor shall separation of the layers occur during testing performed under the requirements of this specification.

D. CIPP Liner Tube:

1. The CIPP tube shall be comprised of a carrier material, resin, and polyurethane, polyethylene, or polypropylene coating(s) on its interior surface to protect the resin from contaminate intrusion or loss of resin through migration. Any temporary interior resin containment coatings or films shall be removed following CIPP tube installation but prior to lateral seal installation (if present) at no additional cost to the OWNER.
2. The liner tube shall be fabricated to a diameter size that when installed will fit tightly into the internal circumference of the existing sanitary sewer main without any annular space between liner and walls of host pipe. Allowances shall be made in the CIPP tube diameter sizing to allow for potential circumferential stretching due to host pipe manufacture tolerances and/or deterioration of the host pipe walls.
3. The minimum liner tube length shall be that deemed necessary by the CONTRACTOR to span the distance effectively and continuously from the inlet to the outlet of the respective manholes, unless otherwise specified. The CONTRACTOR shall verify the lengths in the field before impregnation and installation of the CIPP tube. Individual insertion runs may be made over one or more manhole sections as determined by the CONTRACTOR.
4. The CONTRACTOR shall measure the overall elongation of the CIPP tube after installation completion and report to the ENGINEER immediately following insertion. Summary of CIPP tube elongation shall be submitted per Technical Specification Section 01330, Submittals.
5. Prior to insertion, the CIPP liner tube shall be free of all visible tears, holes, cuts, foreign materials, and other defects. The CONTRACTOR shall perform necessary repairs according to CIPP tube manufacturer recommendations or replace the tube in its entirety at no additional cost to the OWNER.
6. CONTRACTOR shall submit according to Technical Specification Section 01330, Submittals, the recommended step "cure" temperatures and hours at each stages including maximum rate (degrees per hour) of liner heating/cooling, target temperature, duration of target cure temperature, acceptable fluctuation in target temperature with description of remedial actions to be taken if fluctuations occur,

and target temperature when the controlled temperature curing process is terminated and heated water or air is released from the liner interior.

E. CIPP Tube Dimensions:

1. The CONTRACTOR shall make allowances in determining the felt tube length and circumference for stretching during installation and shrinkage during curing. The CONTRACTOR shall verify the lengths in the field before the felt tube is cut and saturated. Individual installation runs may include one or more manhole-to- manhole sections as approved by the ENGINEER.
2. The CONTRACTOR shall determine the length of any dry tube, if any, that may be necessary for the installation process proposed. OWNER shall not entertain payment for excess CIPP length, including dry tube not saturated with resin.
3. The diameter of the host pipe may be larger than the nominal inside diameter due to corrosion of the existing host pipe. It is the CONTRACTOR's responsibility to field measure the diameter of existing host pipe and determine the required diameter of the CIPP tube to produce a close-fit lining.
4. "Minimum Design" thickness shall be the CIPP calculated thickness in accordance with ASTM F1216 and D5813 for **Type II fully deteriorated** design loading, rounded to the next manufacturer's standard incremental thickness above the minimum calculated design thickness. Calculations of liner thickness shall be submitted to the ENGINEER according to Technical Specification Section 01330, Submittals prior to installation.
 - a. Unless otherwise specified to provide for excess resin migration, the gap thickness of the wetting out equipment shall be sized to allow an excess of 5 to 10 percent (5-10%) resin to pass during saturation. The measurement of liner thickness, installed and cured, shall be determined at a minimum of three (3) locations cut from a CIPP restrained liner sample, minus the thickness of any plastic coatings or CIPP layers not included in the structural design of the CIPP, using a method of measurement accurate to the nearest 0.005-mm.
5. The nominal wall thickness shall be at least the final design thickness, or the minimum specified, and may be up to fifteen percent (15%) greater except at the locations where the felt layers overlap, in which case it may be in excess of this value.

2.3 CHEMICAL RESISTANCE

- A. The chemical resistance of the actual resin system used by the CONTRACTOR shall be tested by the resin manufacturer in accordance with ASTM F1216, D543, or C581 as applicable for heat/steam cured or with ASTM F2019 and D5813 for UV cured CIPP. Exposure to the chemical solutions listed below shall result in a loss of not more than 20 percent of the initial physical properties when tested in accordance with ASTM C581 when subjected to the following solutions:

Chemical Solution	Concentration (%)
Tap Water (pH 6-9)	100
Nitric Acid	5
Phosphoric Acid	10
Sulfuric Acid	10
Gasoline	100
Vegetable Oil	100
Detergent	0.1
Soap	0.1

- B. The CONTRACTOR shall be responsible for all costs associated with the chemical resistance tests. Proof of meeting these requirements shall be provided to the ENGINEER for approval at least seven (7) days prior to commencement of work.

2.4 FINISHED AND CURED CIPP LINER PROPERTIES

- A. The physical properties of the cured CIPP shall have minimum initial test values as given in Table 1 of ASTM F1216 for resin impregnated felt tube and supplemented below in Table 1. Properties for these or any other enhanced resins shall be substantiated with test data.

Table 1: CIPP Liner Properties		
Resin Saturated Felt Tube		
Test Property	Test Value	Test Method
Flexural strength	5,000 psi, min	ASTM D790
Flexural modulus	250,000 psi	ASTM D790
50-year flexural creep modulus	150,000 psi	ASTM D2990

- B. The physical properties of the cured CIPP shall have minimum initial test values as given in Table 1 of ASTM F2019 for liner cured by UV light and supplemented below in Table 1. Properties for these or any other enhanced resins shall be substantiated with test data.

Table 1: CIPP Liner Properties		
Resin Saturated Felt Tube		
Test Property	Test Value	Test Method
Flexural strength	15,000 psi	ASTM D790
Flexural modulus	725,000 psi	ASTM D790

2.5 DESIGN CRITERIA

- A. The liner thickness shall be designed in accordance with the procedures of ASTM F1216, Appendix XI, ASTM D2990, ASTM 2019 (if UV cured), Appendix X1, and these Technical Specifications. All material properties used in design calculations shall be long-term (time-corrected) values. The CONTRACTOR shall familiarize itself with site conditions when preparing liner design.
- B. The CONTRACTOR shall calculate the required minimum CIPP liner thickness for each pipe based on Section 2.2.E.4 of these Technical Specifications.
- C. The following parameters shall be assumed for the liner design:
1. Modulus of soil reaction, $E'_s = 1,000$ psi (**fully deteriorated**).
 2. Unit weight of soil = 140 pcf.
 3. The minimum ovality shall be 2.0 percent.
 4. AASHTO HS-20 Traffic loads.
 5. Safety Factor = 2.0.
 6. Groundwater from surface elevation to actual invert depth.

2.6 PAYMENT PENALTY / REDUCTION

- A. A penalty will be assessed by the OWNER to the CONTRACTOR for payment on all installed CIPP liners that do not meet or exceed the minimum CIPP wall thickness requirements as stated in Technical Specification Section 02148 - paragraph 2.2.E.4. Payment to the CONTRACTOR will be reduced on CONTRACTOR's unit price per foot payment for the total length of CIPP liner installed as measured from center of upstream manhole to center of downstream manhole. Payment/reduction shall be based on the following Table 2 – Payment Penalty / Reduction:

TABLE 2 - PAYMENT PENALTY / REDUCTION	
Payment Penalty (see note 1)	Installed Dry Cured Liner Thickness Below Minimum Design (mm)
0%	0% up to and including 5%
10%	Greater than 5% up to and including 10%
20%	Greater than 10% up to and including 20%
Rejected	Greater than 20%
Note 1 – Reduction in payment assessed in CONTRACTOR's pay request to	

2.7 HYDROPHILIC GASKET SEAL AT CIPP TERMINATION

- A. The CONTRACTOR shall provide hydrophilic gasket seal at the CIPP liner terminations per ASTM F3240. The gasket seal shall produce a watertight seal between the host pipe and liner pipe at the manhole connection.

2.8 CURE TEMPERATURE MONITORING

- A. The CONTRACTOR shall provide equipment and fiber optic cable for monitoring the CIPP liner wall temperature during cure along the entire length being installed. Sensor technology shall be manufactured by Vortex Technology Group LLC of Houston, Texas or approved equal.
- B. Failure to submit temperature cure logs shall be grounds for rejection of installed liner.

PART 3 - EXECUTION

3.1 PREPARATORY PROCEDURES

- A. Set-Up:
 - 1. Prior to CIPP installation, all necessary traffic control measures shall be put into place in accordance with the City of Chandler Engineering & Design Standards Manual for Public Works Construction Chapter 7 – Traffic Barricade Design and the project General Conditions.
- B. Sewer Cleaning:
 - 1. Prior to CIPP tube installation, the CONTRACTOR shall clean the existing sewer in accordance with Technical Specification Section 02146, Sewer Pipe And Sewer Structure Cleaning. The CONTRACTOR shall clear the existing sewer of obstructions such as solids, any existing detached liner (if present), or collapsed/offset pipe that will prevent or hinder CIPP liner installation. The CONTRACTOR shall utilize techniques customized to the size and pipe configuration such as, but not limited to man entry and/or a high-pressure water spinning nozzle to ensure that the host pipe is free from any residual sediment, fats-oils-grease (FOG), or accumulated debris prior to CIPP lining. A time lapse no greater than 30 calendar days shall be allowed between initial host pipe cleaning and liner installation. If there is greater time lapse, CONTRACTOR shall re-clean or do touch up cleaning prior to installing the CIPP liner.
- C. Sewer Bypassing And Dewatering:
 - 1. The CONTRACTOR shall be responsible for bypassing flow around their work and dewatering sewer lines in accordance with the requirements of Technical Specification Section 02145, Diversion of Sewage Flow and Dewatering.
- D. Interior Pipe Verification:
 - 1. The interior of the sewer shall be inspected in accordance with Technical Specification Section 02147, Remote CCTV Inspection of Existing and Rehabilitated Sewers, or other means proposed by the CONTRACTOR to determine the location of conditions, which may prevent proper installation of CIPP. The CONTRACTOR shall furnish the cleaning CCTV and report to the ENGINEER

for host pipe segments that the CONTRACTOR determines existing conditions may impact the final operation of the lined pipe, either grade or condition.

E. Offset Pipe Grade:

1. The CONTRACTOR shall bring to the attention of the ENGINEER any offset in pipe grade that is greater than five percent (5%) of the internal diameter for authorization to install CIPP liner. Installation of CIPP tube prior to obtaining authorization shall be grounds for CIPP liner rejection and non-payment.

F. Defective Lining Removal/Void Repair:

1. The CONTRACTOR shall remove all deteriorated concrete and/or hanging, loose, or otherwise defective lining/coatings that were previously installed but have failed and that would adversely impact the profile of the CIPP liner. Once deteriorated materials are removed, CONTRACTOR shall repair host pipe interior profile back to original dimensions with approved material(s) prior to CIPP tube installation. The CONTRACTOR shall bring to the OWNER's attention area for potential repair where evidence of structural strength deterioration has occurred, or voids are observed behind the existing host pipe wall.

G. CIPP Liner Diameter Verification:

1. The internal diameter of the existing host pipe may be larger (or smaller) than their nominal size. It is the CONTRACTOR's responsibility to measure the actual inside diameter of the sewer to determine the appropriate size of CIPP liner to use.

H. Pre-Constructon Site Condition Documentation:

1. CONTRACTOR shall take pre-construction videos of work areas according to the General Conditions and Technical Specification Section 02147, Remote CCTV Inspection of Existing and Rehabilitated Sewers before beginning work. It shall be the responsibility of the CONTRACTOR to review the video and bring it to the attention of the ENGINEER any site conditions that are not conducive to the lining system installation or its operations proposed. Once the CONTRACTOR has reviewed the video and accepted the existing on-site conditions, it shall be the responsibility of the CONTRACTOR to complete the work as proposed.

3.2 INSTALLATION METHODS

A. General:

This section is intended to provide the CONTRACTOR with general guidance on the methods to be used to rehabilitate sewer pipe using the CIPP liner installation method. Nothing contained herein shall relieve the CONTRACTOR from completing the pipe rehabilitation in the most feasible, efficient, and safe manner.

1. The CONTRACTOR shall designate the location where the CIPP felt tube would be saturated with resin ("wet-out"). Locations shall be subject to approval by the ENGINEER and applicable local agencies. The CONTRACTOR shall allow the OWNER and/or ENGINEER to inspect the materials and "wet-out" procedure.

2. If the “wet-out” location is not at the project site, the saturated CIPP tube shall be transported to the work site under controlled environmental conditions. Transport vehicles shall include a tamper resistant, sealed temperature-recording device which always records the temperature of the CIPP tube after leaving the wet-out site. The CONTRACTOR shall decide when to transport the impregnated CIPP tube to the site and when to commence insertion with respect to weather conditions.
3. The CONTRACTOR shall conduct operations in accordance with applicable OSHA standards, including but not limited to those safety requirements involving work on an elevated platform and entry into a confined space. Make suitable precautions to eliminate hazards to personnel near construction activities when pressurized air or hot water is being used.
4. The CONTRACTOR is responsible for obtaining construction / fire hydrant meters, if necessary, and for paying all applicable deposit fees and water usage fees.

B. Pre-Installation:

1. Prior to installation of the liner, the following activities are required to be completed:
 - a. Receipt and approval of pre-installation Submittals by the ENGINEER.
 - b. Verification of pipe condition and any obstructions by video inspection.
 - c. Verification of existing laterals and location of services by flowing water, dye test, or visually with a pan and tilt head camera or other means.
 - d. Cleaning of sewer main (recorded on video).
 - e. Perform required point repairs at pipe locations that would prevent or hinder proper CIPP liner. The CONTRACTOR shall obtain approval for any point repairs from the ENGINEER prior to proceeding with the liner installation. The CONTRACTOR assumes all responsibility for CIPP tube quality and finished pipe grade once installation begins if prior written notification is not received and acknowledged by the ENGINEER.
 - f. The CONTRACTOR is to ensure CIPP tube is free of all visible tears, holes, cuts, foreign materials, and other defects.

C. Handling:

1. The CONTRACTOR shall exercise adequate care during transportation, handling, and installation to ensure the CIPP tube material(s) are not torn, cut, or otherwise damaged. If any part or parts of the CIPP material becomes torn, cut or otherwise damaged before or during insertion, it shall be repaired or replaced in accordance with the manufacturer’s recommendations and approval by the ENGINEER before proceeding further, and at the CONTRACTOR's expense.

D. Installation Process:

1. No CIPP installations will be undertaken in weather conditions that could jeopardize the installation of the CIPP or be detrimental to the long-term performance of the CIPP. The liner will be installed by the inversion tube method as follows.
 - a. INVERSION METHOD: The CIPP tube shall be saturated with a viscous thermosetting resin and lowered into the insertion pit through an inversion tube.

The inversion tube will then be inverted into the existing pipe by the hydraulic weight of a column of water or by air pressure or by a combination of a static head of water and air pressure which will invert and push the tube through the existing sewer pipe inside out, while pressing the resin impregnated side of the CIPP tube firmly against the inside walls of the existing host pipe. The smooth coated side of the CIPP tube shall become the new interior surface of the sewer pipe following inversion. The maximum inversion installation rate shall be thirty (30) feet per minute or as recommended by the CIPP tube manufacturer whichever is less. After the CIPP tube is inverted through the pipe section, the CONTRACTOR shall cure the tube by heating the water by circulating it through a boiler, where the hot water will cause the resin to chemically react and harden or by introducing steam or a combination of compressed air and steam to initiate the hardening cure process.

- b. **PULLED IN PLACE METHOD:** Resin impregnated glass fiber tube is pulled-in place into an existing pipe using a winch followed by its inflation with compressed air pressure to expand it firmly against the wall surface of the host structure. The photo-initiated resin system in the tube is then cured by the CONTRACTOR by exposing to ultraviolet (UV) light using a light train. CIPP installation shall be in accordance with ASTM F2019. The light train for UV curing shall be assembled according to the Manufacturer's recommendations for the sewer pipe and liner diameter. Cure the liner in accordance with the curing protocol and submit record to the OWNER and ENGINEER. CONTRACTOR shall obtain samples during installation in accordance with ASTM F2019 for UV cured lining. Samples shall be taken in the presence of the OWNER or ENGINEER. The protocol and recording shall include the following minimum information – curing speed, lighting sources working and wattage, inner air pressure, exothermic temperature, date and time, length of liner, location installation.

E. Curing:

1. The curing process shall be per ASTM F1216 or similar approach recommended by the manufacturer and approved by the ENGINEER, to ensure that the liner design physical properties are attained.
2. For UV-cured CIPP lining the CONTRACTOR shall provide a curing control protocol in accordance with Section 6.7.3 of ASTM F2019 for each installation. The protocol shall include actual materials and equipment (e.g., light train) to be utilized by the CONTRACTOR. The Liner Manufacturer shall approve the control protocol in writing.
3. The rate of temperature rise and fall during heating and cooling shall not exceed two (2) degrees F per minute or manufacturer's recommendation, whichever is slower. CONTRACTOR shall submit per Technical Specification Section 01330, Submittals the manufacturer recommendation for temperature control requirements during heat-up, cure, recommended cure temperature, and length of time maintained at recommended cure temperature, and maximum rate of cool down of temperature,

and temperature when heated water can be safely released from the tube without detrimental CIPP tube shrinkage occurring.

F. Cure Temperature Monitoring:

1. The CONTRACTOR shall monitor the temperature of the CIPP liner wall to verify and document manufacturer recommended curing temperature has been achieved for pipe 15-inches in diameter and larger. A temperature monitoring system shall be placed between the host pipe and the liner in the bottom of the host pipe (invert) to record the heating and cooling that takes place on the outside of the liner.
2. The CONTRACTOR shall install and monitor temperature during the curing process with system/equipment as follows:
 - a. CIPP Liner in Pipe 15-inch Diameter and Larger: Fiber optic cable along the entire length of CIPP liner in addition to point temperature sensors installed at the upstream, downstream, and intermediate (if present) access manholes. The monitoring shall be by a computer which can record the temperatures at this interface throughout the processing of the CIPP utilizing a tamper-proof database.
 - b. Prior to installing the CIPP liner in the host pipe, the temperature monitoring system's function shall be confirmed by hooking it up to the computer and seeing that the sensors are reporting ambient temperatures. No more than two (2) sensors in sequence can be found faulty during this test. If three (3) or more sensors in sequence are discovered faulty, a new sensor array shall be pulled into the host pipe replacing the previously installed array; and the new array shall be again tested for its proper function.
3. As a secondary backup of liner cure temperature, the CONTRACTOR shall monitor the temperature of the CIPP tube wall to verify manufacturer recommended curing temperature has been achieved. Temperature sensors shall be placed at the upstream and downstream ends of the reach being lined to monitor the pressurized liner (air or water) temperature during the curing process. Temperature sensors shall be placed between the host pipe and the CIPP tube in the bottom of the host pipe (invert) to record the heating and cooling that takes place on the outside of the CIPP tube. Additionally, sensors shall be strategically placed at points where a significant heat sink is likely to be anticipated. The temperatures at these interfaces shall be monitored by the CONTRACTOR throughout the curing of the CIPP tube.

G. Reinstatement Of Lateral Connections:

1. It is the intent of this project that all service lateral connections be reopened.
2. Service laterals shall be reopened to 100% of original lateral pipe diameter without excavation, utilizing a remotely controlled cutting device, monitored by CCTV. The cut liner shall have no jagged edges and shall be brushed smooth with a wire brush. CONTRACTOR shall reinstate laterals back to a smooth finish with the lateral opening free of debris or obstruction to flow. CONTRACTOR shall remove any "coupons" that fall within laterals prior to performing a post-installation CIPP

CCTV. The CONTRACTOR shall certify they have a minimum of two (2) complete working cutter units plus spare key components on the site before the insertion of the liner. No additional payment shall be made for excavations for the purpose of reopening connections unless previously approved by the ENGINEER prior to the start of work. The CONTRACTOR is responsible for all costs and liability associated with such excavations and restoration work.

H. Reinstatement Of Drop Sewer Connections:

1. The CONTRACTOR shall reinstate the opening of all vertical drop sewer connections to 100% of original drop pipe diameter. CONTRACTOR shall reinstate drop back to a smooth finish with the drop opening free of debris or obstruction to flow.

I. Release Of CIPP Cure Water:

1. CONTRACTOR shall be responsible for notification and obtaining written authorization from the OWNER operating the downstream wastewater treatment facility prior to release of heated water into the sewer system. OWNER reserves the right, but not limited to, requesting the CONTRACTOR to restrict the rate of release of hot water into the waste stream so that elevated waste stream temperature do not detrimentally impact any biological treatment process or equipment.
 - a. The Cure temperature shall be monitored exterior to the liner utilizing thermocouple sensors along with fiber optic temperature monitoring system. Circulation cure water shall be cooled down to at least 100 degrees F before releasing the hydrostatic head into the existing collection system further than one (1) mile upstream of the water reclamation facility.
 - b. Cure water temperature shall be cooled to 95 degrees F if the hydrostatic head water is to be release within one (1) mile upstream of the water reclamation facility unless prior authorization and/or alternative arrangements are obtained from the OWNER.
 - c. Ammonium Persulfate may be used to mitigate the presence of styrene in the cure water. The CONTRACTOR shall comply with manufacturer's recommendations on quantity and safety and shall coordinate with the downstream water reclamation facility prior to its use.
2. The CONTRACTOR shall be responsible for all damages and pay all compliance fines imposed onto the OWNER by State Regulatory Agencies for treatment processes impacted by the release of heated water into the facility.

3.3 FINISHED PRODUCT

A. Finish:

1. The finished CIPP shall be continuous over the entire length of an installation run. The CONTRACTOR shall be responsible for control of all material and process variables to provide a finished CIPP possessing the minimum properties specified in ASTM F1216 and supplemented herein.

2. Defects such as foreign inclusions, dry spots, pinholes, delamination, seam separation and wrinkling beyond the allowances stated in Technical Specification paragraphs 3.3.A.3 and 3.3.A.5, or as determined by the ENGINEER as affecting the integrity or strength of the CIPP, or as adversely affecting the hydraulic capacity of the CIPP, shall be repaired, or replaced at the CONTRACTOR's expense.
3. Wrinkles in the finished liner pipe which cause a backwater of one (1) inch or more or reduce the hydraulic capacity of the pipe (wrinkles which exceed five (5) percent of the pipe diameter) are unacceptable and shall be removed or repaired by the CONTRACTOR at no additional cost to the OWNER. Wrinkles in the finished liner pipe that reduce the structural stability of the pipe are unacceptable. If a void between the wrinkle and the pipe exists, the CONTRACTOR shall repair or replace that section of the pipe at no additional cost to the OWNER. Methods of repair shall be proposed by the CONTRACTOR and submitted to the ENGINEER for review.
4. Bumps or wrinkles shall be removed within ten (10) feet of a manhole insertion and/or termination end to produce a smooth wall surface which allows for a proper seal if a flow control plug installation is necessary in the future.
5. Separations of liner seams in the finished liner pipe are unacceptable and shall be removed or repaired by the CONTRACTOR at no additional cost to the OWNER. If a separation of a liner seam exists, the CONTRACTOR shall repair or replace that section of the CIPP tube at no additional cost to the OWNER. Methods of repair shall be proposed by the CONTRACTOR and submitted to the ENGINEER for approval.
6. Longitudinal shrinkage of liner beyond the interior face of the manhole wall shall be repaired. Proposed repair materials and procedures shall be submitted to the ENGINEER for approval prior to commencement of repairs.

B. Manhole Connection:

1. The CONTRACTOR shall provide a watertight seal at the sewer connection so that no leakage of fluids may infiltrate between the liner and the existing sewer surface/manhole. The cut liner shall have no jagged edges and shall be sanded or brushed smooth. After the curing of CIPP liner is complete, the liner termination ends shall be left in the manholes for a minimum of one (1) day to relieve longitudinal forces created by liner shrinkage.
2. CONTRACTOR shall install Fiber Reinforced Polymer (FRP) coating per Technical Specification Section 02720, Fiber Reinforced Polymer (FRP) to provide a durable surface and gas tight seal between the end(s) of the CIPP liner, through the manhole trough, and up the throat section of the manhole base (if present) and connect into the barrel section of the access manhole per the details in Design Drawings.

C. Vehicular Access:

1. If required all hoses/pipes used for introducing water, air, and/or steam shall be ramped during the inversion and curing process to allow for the ease of vehicular and pedestrian traffic.

D. Hose Identification:

1. All hoses/pipes shall be color-coded for identification to prevent the use of hoses/pipes used for water conveyance are not used for wastewater conveyance or visa-versa. Any hose connected to a public water supply shall be at the discretion of the OWNER and be equipped with an approved backflow prevention device.

E. Inspection:

1. The CONTRACTOR shall inspect the CIPP after installation. The inspection will be performed and recorded using remote visual television inspection equipment in accordance with Technical Specification Section 02147, Remote CCTV Inspection of Existing and Rehabilitated Sewers. If defects exist within the liner, the CONTRACTOR shall repair or replace that section of liner at no additional cost to the OWNER. Methods of repair shall be proposed by the CONTRACTOR and submitted to the ENGINEER for review and approval prior to initiating repair work. ENGINEER shall be notified a minimum of 24 hours prior to performing repair work.
2. The CONTRACTOR shall resubmit a final video showing a post inspection after the liner issues have been addressed and approved.

F. Field Testing:

1. Sampling and testing of materials: ASTM Standards
2. CIPP Testing: ASTM F1216

G. Acceptance:

1. Inspect CIPP tube by CCTV inspection per Technical Specification Section 02147, Remote CCTV of Existing and Rehabilitated Sewers
2. Visual inspection - Infiltration: NONE
3. Defects: If ENGINEER deems the following:
 - a. Repairable - repair defect per Paragraph 3.3.A.
 - b. Not Repairable - remove and replace the CIPP tube at no additional cost to the OWNER.

3.4 CLEANUP

- A. Following inspection, the CONTRACTOR shall clean up the entire project area. The CONTRACTOR shall dispose of all excess material and debris not incorporated into the permanent installation off site.

3.5 WARRANTY

- A. The CONTRACTOR shall CCTV per Technical Specification Section 02147, Remote CCTV of Existing And Rehabilitated Sewers rehabilitated **one (1) year following final completion**. A copy of the CCTV video records shall be submitted to the OWNER according to Technical Specification Section 01330, Submittals. Procedures for repair of liner defects identified shall be submitted to the OWNER for approval prior to

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authorization to proceed. CCTV video investigation and repair of defects shall be at no additional cost to the OWNER.

++ END OF SECTION ++

SECTION 02220

DEMOLITIONS

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. Provide all labor, materials, equipment and incidentals as shown on the Drawings, specified and required for demolitions, removal and disposal work.
2. Included, but not limited to, are demolition and removals of existing materials, equipment, or work necessary to install the work as shown on the drawings, specified and required to connect same with existing work in an approved manner. Demolition includes structural concrete, appurtenances, piping, electrical and mechanical equipment, wiring and conduits, paving, curbs, walks, fencing, and similar existing facilities.
3. Demolitions and removals which may be specified under other Sections shall conform to requirements of this Section.
4. Pay for all landfill disposal fees.

1.2 SUBMITTALS

- A. Schedule: Submit for approval proposed methods, equipment, and operating sequences. Include coordination for shut-off, capping, temporary services, continuation of utility services, and other applicable items to ensure no interruption of OWNER'S operations.

B. Shop Drawings: Submit for approval the following:

1. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the

specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

- C. CONTRACTOR shall submit electronic copies per Specification Section 01332, Shop Drawing Procedures. Text shall be in electronic ASCII format. Drawings and figures shall be in Version 2018 or newer AutoCAD ".dwg," or pdf ".pdf" format.

1.3 JOB CONDITIONS

A. Protection:

1. Perform all demolition and removal Work to prevent damage or injury to structures, occupants thereof and adjacent features which might result from falling debris or other causes, and so as not to interfere with the use, and free and safe passage to and from adjacent structures.
2. Closing or obstructing of roadways, sidewalks, and passageways adjacent to the Work by the placement or storage of materials will not be permitted, and all operations shall be conducted with a minimum interference to traffic on these ways.
3. Erect and maintain barriers, lights, sidewalk sheds, and other necessary protective devices.
4. Repair damage to facilities to remain, or to any property belonging to the OWNER or occupants of the facilities. Comply with requirements of Specification Section 02230, CLEARING.

B. Scheduling:

1. Carry out operations so as to avoid interference with OWNER'S operations and work in the existing facilities.

C. Notification:

1. At least 48 hours prior to commencement of a demolition or removal, notify ENGINEER, in writing, of proposed schedule therefore. OWNER will inspect the existing equipment and mark for identification those items which are to remain the property of the OWNER. Do not start removals without the permission of the ENGINEER.

D. Explosives:

1. Do not bring explosives on site.
2. Explosives are not approved.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION

3.1 GENERAL

- A. All materials and equipment removed from existing work, shall become the property of CONTRACTOR, except for those which are designated on the drawings to be “Salvaged”, “Reused”, or “Relocated” and those that the OWNER has identified and marked for their use. The OWNER may identify the materials and equipment that they want to keep during a site visit prior to the demolition work. All materials and equipment marked by the OWNER to remain the property of the OWNER shall be carefully removed by CONTRACTOR, so as not to be damaged, and shall be cleaned and stored on or adjacent to the site in a protected place specified by the ENGINEER or loaded onto trucks provided by the OWNER.
- B. Dispose of all demolition materials, equipment, debris, and all other items not marked by the OWNER to remain off the site and in conformance with all existing applicable laws and regulations.
- C. Pollution Controls: Use water sprinkling, temporary enclosures, and other suitable methods to limit the amount of dust and dirt rising and scattering in the air to the lowest practical level. Comply with governing regulations pertaining to environmental protection.
 - 1. Do not use water when it may create hazardous or objectionable conditions such as ice, flooding, and pollution.
 - 2. Clean adjacent structures, facilities, and improvements of dust, dirt, and debris caused by demolition operations. Return adjacent areas to conditions existing prior to the start of the Work.
- D. Pavement Demolition:
 - 1. All asphalt and concrete pavement demolition shall terminate at cut edges. All edges shall be linear and have a vertical cut face.

3.2 STRUCTURAL REMOVALS

- A. Remove structures to the lines and grades shown on the Drawings, unless otherwise directed by the ENGINEER. Where no limits are shown on the Drawings, the limits shall be 4-inches outside the item to be installed. The removal of masonry beyond these limits shall be at CONTRACTOR'S expense and these excess removals shall be reconstructed to the satisfaction of the ENGINEER, with no additional compensation to CONTRACTOR.
- B. All concrete, brick, tile, concrete block, roofing materials, reinforcement, structural or miscellaneous metals, plaster, wire mesh and other items contained in or upon the

structure shall be removed and taken from the site, unless otherwise approved by the ENGINEER. Demolished items shall not be used in backfill.

- C. After removal of parts or all of masonry walls, slabs and like work which tie into the Work or existing work, the point of junction shall be neatly repaired so as to leave only finished edges and surface exposed.
- D. Where new anchoring materials, including bolts, nuts, hangers, welds and reinforcing steel, are required to attach the Work to the existing work they shall be included under this Section, except where specified elsewhere.

3.3 MECHANICAL REMOVALS

- A. Mechanical removals shall consist of dismantling and removing of existing piping, equipment and other appurtenances as specified, shown, or required for the completion of the Work. Mechanical removals shall include cutting, capping, and plugging as required, except that the cutting of existing piping for the purpose of making connections thereto will be included under Division 15, Mechanical.
- B. Existing process, water, chemical, gas, fuel oil and other piping not required for the Work shall be removed where shown on the Drawings or where it will interfere with the Work. Piping not indicated to be removed or which does not interfere with the Work shall be removed to the nearest solid support, capped and left in place. Chemical and fuel lines and tanks shall be purged and made safe prior to removal or capping. Where piping that is to be removed passes through existing walls, it shall be cut off and properly capped on each side of the wall.
- C. When underground piping is to be altered or removed, the remaining piping shall be properly capped.
- D. Any changes to potable water piping and other plumbing and heating system work shall be made in conformance with all applicable codes and under the same requirements as other underground piping. All portions of the potable water system that have been altered or opened shall be pressure tested and disinfected in accordance with Section 15050, Piping Systems, and Section 15051, Buried Piping Installation, and local codes. Other plumbing piping and heating piping shall be pressure tested only.

3.4 ALTERATIONS AND CLOSURES

- A. Alterations shall conform with the Contract Documents, and the directions and approvals of the ENGINEER.
- B. Openings in existing concrete slabs, ceilings, masonry walls, floors and partitions shall be closed and sealed as shown on the Drawings or otherwise directed by the ENGINEER. The Work shall be keyed into the existing work in a manner approved

by the ENGINEER. In general, use the same or matching materials as the existing adjacent surface. The finished closure shall be a smooth, tight, sealed, permanent closure acceptable to the ENGINEER.

3.5 CLEAN-UP

- A. Properly remove from the site all debris resulting from the demolition operations as it accumulates. Upon completion of the Work, all materials, equipment, waste, and debris of every sort shall be removed and premises shall be left, clean, neat and orderly. Comply with requirements of Section 02315, Structural Excavation and Backfill, and Section 02230, Clearing.

++ END OF SECTION ++

SECTION 02230

CLEARING

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Scope:
 - 1. Provide all labor, materials, equipment and incidentals required to perform all clearing and grubbing as shown on the Drawings and specified.
- B. The Work covered by this Section consists of removing and disposing of all trees, stumps, bush, roots, shrubs, vegetation, logs, rubbish, and other objectionable material from the site, as required to perform the Work.

1.2 QUALITY ASSURANCE

- A. Codes and Standards: State and local laws and code requirements shall govern the hauling and disposal of trees, shrubs, stumps, roots, rubbish, debris and other matter.

1.3 JOB CONDITIONS

- A. Protection:
 - 1. Streets, roads, adjacent property and other works and structures shall be protected throughout the entire Project. Return to original condition, satisfactory to the ENGINEER, damaged facilities caused by CONTRACTOR'S operations.
 - 2. Trees, shrubs, grassed and landscaped areas, which are to remain, shall be protected by fences, barricades, wrapping or other methods as shown on the Drawings, specified or approved by the ENGINEER. Equipment, stockpiles, etc. shall not be permitted within tree branch spread. Trees shall not be removed without approval of the ENGINEER, unless shown or specified.
- B. Salvageable Improvements:
 - 1. Unless specified elsewhere, carefully remove items to be salvaged and store on premises in approved location, all in accordance with recommendations of specialists recognized in the Work involved.

1.4 GUARANTEE

- A. Guarantee that Work performed under this Section will not permanently damage trees, shrubs, turf or plants designated to remain, or other adjacent work or facilities. If damage resulting from CONTRACTOR'S operations appears during the period up

to 12 months after completion of the Project, replace damaged items, at no additional cost to OWNER.

1.5 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
1. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.
 2. CONTRACTOR shall submit hard copies and soft copy per Specification Section 01332, Shop Drawing Procedures. Soft copies shall be in CD format and shall include all information provided in hard copy. Text shall be in electronic ASCII format. Drawings and figures shall be in Version 2018 or newer AutoCAD ".dwg," or pdf ".pdf" format.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION

3.1 CLEARING AND GRUBBING

- A. Limits of clearing shall be all areas within the Contract limit lines, except as otherwise shown on the Drawings. Damage outside these limits caused by CONTRACTOR'S operations shall be corrected at CONTRACTOR'S expense.
- B. Remove from the site and properly dispose of all trees, shrubs, stumps, roots, brush, masonry, rubbish, scrap, debris, pavement, curbs, fences and miscellaneous other structures not covered under other Sections as shown on the Drawings, specified or otherwise required to permit construction of the Work. Comply with requirements of Section 02220, Demolition.

- C. No cleared or grubbed material may be used in backfills or structural embankments. Comply with requirements of Section 02315, Structural Excavation and Backfill.
- D. CONTRACTOR to remove existing trash and concrete pile-up from site and dispose them off to a safe location per the location per the local rules and regulations.
- E. Burning on the site will not be allowed.
- F. In order to avoid additional removal or damage, existing trees and shrubs shall be trimmed as required. Trimmed or damaged trees shall be treated and repaired by persons with experience in this specialty who are approved by ENGINEER. Trees and shrubs intended to remain, which are damaged beyond repair or removed, shall be replaced by CONTRACTOR at no additional cost to OWNER.
- G. Control air pollution caused by dust and dirt, and comply with governing regulations.

3.2 TOPSOIL REMOVAL

- A. Topsoil is defined as friable clay loam surface soil found in a depth of not less than 4- inches. Topsoil shall be substantially free of subsoil, clay lumps, stones, and other objects over 2-inches in diameter, and without weeds, roots, and other objectionable material.
- B. Strip topsoil which is satisfactory to whatever depths are encountered, and in such manner as to prevent intermingling with the underlying subsoil or other objectionable material. Remove heavy growths of grass from areas before stripping.
 - 1. Where trees are shown on the Drawings or directed by the ENGINEER to be left standing, stop topsoil stripping a sufficient distance from such trees to prevent damage to the main root system.
- C. Stockpile topsoil in storage piles in areas shown on the Drawings, or where otherwise approved by ENGINEER. Construct storage piles to freely drain surface water. Cover storage piles, if required, to prevent windblown dust.
- D. Dispose of excess topsoil as waste material off site.

++ END OF SECTION ++

SECTION 02315

STRUCTURAL EXCAVATION AND BACKFILL

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. Provide all labor, materials, equipment and incidentals required to perform all excavating, backfilling, filling and grading, and disposing of earth materials as shown on the Drawings, specified, and required for construction of manholes, vaults, cast-in-place structures, structure foundations, and other structures and facilities required to complete the Work in every respect.
2. All necessary preparation of subgrade for slabs, foundations and pavements is included.
3. All temporary means required to prevent discharge of sediment to water courses from dewatering systems or erosion are included.
4. On-site excavated material will be classified for use as backfill material. Excavation materials include all materials regardless of type, character, composition, moisture, or condition thereof.
5. Perform all earthwork as specified in this Section.

- B. General: Native soils are generally suitable for use in grading fills but shall be screened to remove cobbles 4-inches and larger, and shall have less than 50% passing the No. 200 sieve. See more detailed gradation requirements in the relevant sections.

1.2 QUALITY ASSURANCE

A. Testing Services:

1. General: Testing of materials, testing for moisture content during placement and compaction of fill materials, and of compaction requirements for compliance with technical requirements of these Specifications shall be performed by a testing laboratory as designated in Section 01451, Testing Laboratory Services Furnished by OWNER and Section 01452, Testing Laboratory Services Furnished by CONTRACTOR.
2. CONTRACTOR'S Testing Agency Scope:
 - a. Test CONTRACTOR'S proposed materials in the laboratory and/or field for compliance with these Specifications.
 - b. Conduct gradation and plasticity testing on excavated spoils proposed for backfill
 - c. Perform field moisture content and density tests to assure that the specified compaction of backfill materials has been obtained.
 - d. Report all test results to the ENGINEER.

3. Authority and Duties of CONTRACTOR'S Testing Agency: Technicians representing the testing laboratory shall inspect the materials in the field and perform tests and shall report their findings to the ENGINEER and CONTRACTOR. When the materials furnished or Work performed fails to fulfill Specification requirements, the technician will direct the attention of the ENGINEER and CONTRACTOR to such failure.
 - a. The technician shall not act as foreman or perform other duties for CONTRACTOR. Work will be checked as it progresses, but failure to detect any defective Work or materials shall not in any way prevent later rejection when such defect is discovered, nor shall it obligate the ENGINEER for final acceptance. Technicians are not authorized to revoke, alter, relax, enlarge, or release any requirements of the Contract Documents, nor to approve or accept any portion of the Work.
4. Responsibilities and Duties of CONTRACTOR:
 - a. The use of testing services shall in no way relieve CONTRACTOR of the responsibility to furnish materials and construction in full compliance with the Contract Documents.
 - b. To facilitate testing services:
 - 1) Secure and deliver to the ENGINEER or to the testing agency, without cost, preliminary representative samples of the materials he proposes to use and which are required to be tested for verification.
 - 2) Furnish such casual labor as is necessary to obtain and handle samples at the Work site or at other sources of material.
 - 3) Advise the ENGINEER'S testing agency at least two days in advance of any backfill operations to allow for completion of quality tests and for the assignment of personnel.
 - c. CONTRACTOR'S Testing Service shall inspect and approve subgrades and fill layers before further construction Work is performed thereon.
 - d. Responsibility belongs to CONTRACTOR to accomplish the specified compaction for backfill, fill, and other earthwork, and to control his operations by confirmation tests to verify and confirm that CONTRACTOR has complied, and is complying at all times, with the requirements of these Specifications concerning compaction, control, and testing.
 - e. The frequency of CONTRACTOR'S confirmation tests shall be not less than as follows; each test location for trenches shall include tests for each layer, type, or class of backfill from bedding to finish grade.
 - 1) Trenches for structures:
 - a) In Open Fields: Four locations every 200 linear feet, each lift.
 - b) Along Dirt or Gravel Roads or Off Traveled Right-of-way: Four locations every 200 linear feet, each lift.
 - c) Crossing Paved Roads: Two locations along each crossing.
 - d) Under Pavement Cuts or Within Two Feet of Pavement Edges: Two locations every 200 linear feet, each lift.

- e) Compaction test shall be taken at least every 1 foot of total lift and 20 feet lineally.
- 2) For structural backfill: On 10-foot intervals on all sides of the structure for every compacted lift but no less than one per lift on each side of the structure for structures less than 10-feet long on a side.
- 3) In embankment or fill: One per 250 square feet on every compacted lift.
- 4) Base material: Four per 100 square feet on every compacted lift.
- 5) Footing Subgrade: For each strata of soil on which footings will be placed, conduct at least one test to verify required design bearing capacities. Subsequent verification and approval of each footing subgrade may be based on a visual comparison of each subgrade with related tested strata, when acceptable to ENGINEER.
- 6) Paved Areas and Building Slab Subgrade: Make at least one field density test of subgrade for every 150 square feet of paved area or building slab, but in no case less than three tests. In each compacted fill layer, make one field density test for every 150 square feet of overlaying building slab or paved area, but in no case less than three tests.
- 7) Foundation Wall Backfill: Take at least six field density tests, at locations and elevations as directed by the ENGINEER.
- f. Copies of the test reports shall be submitted promptly to the ENGINEER. CONTRACTOR'S tests to be performed by a soils testing laboratory acceptable to the ENGINEER.
- g. Demonstrate the adequacy of compaction equipment and procedures before exceeding any of the following amounts of earthwork quantities:
 - 1) 50 linear feet of trench backfill.
 - 2) 5 cubic yards of structural backfill.
 - 3) 100 cubic yards of embankment work.
 - 4) 10 cubic yards of base material.
- h. Until the specified degree of compaction on the previously specified amounts of earthwork is achieved, no additional earthwork of the same kind shall be performed.
- i. Periodic compliance tests will be determined as needed by the ENGINEER to verify that compaction is conforming to the requirements previously specified, at no cost to OWNER. Remove the overburden above the level at which the ENGINEER wishes to test and shall backfill and recompact the excavation after the test is complete. CONTRACTOR to pay for third party testing as directed by the ENGINEER. Testing quantity shall be estimated at 20-percent of CONTRACTOR testing requirements.
- j. If compaction fails to conform to the specified requirements, remove and replace the backfill at proper density or shall bring the density up to specified level by other means acceptable to the ENGINEER. Subsequent tests required to confirm and verify that the reconstructed backfill has been brought up to specified density shall be paid by CONTRACTOR.

CONTRACTOR'S confirmation tests to be performed in a manner acceptable to the ENGINEER. Frequency of confirmation tests for remedial Work shall be double that amount specified for initial confirmation tests.

B. Permits and Regulations:

1. Obtain all necessary permits for Work in roads, rights-of-way, railroads, etc. Also, obtain permits as required by local, state and federal agencies for discharging water from excavations.
2. Perform excavation Work in compliance with applicable requirements of governing authorities having jurisdiction.

C. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown on the Drawings or specified. Where the requirements of more than one code or standard are applicable, the more restrictive will govern. The codes that will be in effect at the completion of the design phase of the project will be used.

1. ASTM A 36/A 36M, Specification for Carbon Structural Steel.
2. ASTM A 328/A 328M, Specification for Steel Sheet Piling.
3. ASTM D 422, Method for Particle-Size Analysis of Soils.
4. ASTM D 427, Test Methods for Shrinkage Factors of Soils by the Mercury Method.
5. ASTM D 698, Test Method for Laboratory Compaction Characteristics of Soil.
6. ASTM D 1556, Test Method for Density and Unit Weight of Soil in Place by the Sand-Cone Method.
7. ASTM D 2166, Test Method for Unconfined Compressive Strength of Cohesive Soils.
8. ASTM D 2922, Test Methods for Density of Soil and Soil-Aggregate in Place by Nuclear Methods (Shallow Depth).
9. ASTM D 4318, Test Method for Liquid Limit, Plastic Limit and Plasticity Index of Soils.
10. AISC Specifications for the Design, Fabrication, and Erection of Structural Steel for Buildings.
11. OSHA Standard, Title 29, Code of Federal Regulations, Part 1926, Section .650 (Subpart P - Excavations).
12. The Code of the City of Chandler, Arizona.
13. Uniform Standard Specifications for Public Work Construction by the Maricopa Association of Governments (MAG) as supplemented by the City of Chandler, Section 206, Structure Excavation and Backfill, Section 604, Placement of Controlled Low Strength Material, Section 702, Base Materials, Section 725, Portland Cement Concrete, Section 728, Controlled Low Strength Material. Where there is a conflict between MAG Standard Specifications as supplemented by the City of Chandler and this Specification, provisions of this Specification shall govern.

1.3 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
1. Excavation and Backfill Submittals:
 - a. Excavation Plan: Prior to start of excavation operations, a written plan shall be submitted to demonstrate compliance with OSHA Standard 29 CFR Part 1926.650. As a minimum, excavation plan shall include:
 - 1) Name of competent person.
 - 2) Excavation method(s) or protective system(s) to be used.
 - 3) Copies of "Manufacturer's Data" or other tabulated data if protective system(s) are designed on the basis of such data.
 - b. Excavation and backfill requirements detailing sheeting and bracing, or other protective system(s), dewatering systems, cofferdams, and underpinning.
 - c. Shop Drawings shall be prepared by a Registered Professional Engineer, licensed in the State of Arizona, recognized as an expert in the specialty involved. Drawings shall be submitted to ENGINEER for record purposes only. Calculations shall not be submitted. Drawing submittals will not be checked and will not imply approval by ENGINEER of the Work involved. Sole responsibility for designing, installing, operating and maintaining whatever system is necessary to satisfactorily accomplish all necessary sheeting, bracing, protection, underpinning and dewatering belongs to CONTRACTOR.
 - d. Samples of all materials, including select backfill, general backfill, crushed stone and sand shall be submitted to the ENGINEER and the testing service. Samples of the proposed material shall be submitted at least 14 days in advance of its anticipated use.
 2. Test Reports:
 - a. Testing laboratory shall submit copies of the following reports directly to ENGINEER, with copy to CONTRACTOR:
 - 1) Tests on borrow material.
 - 2) Tests on footing subgrade.
 - 3) Field density tests.
 - 4) Optimum moisture - maximum density curve for each soil used for backfill.
 - 5) Tests of actual unconfined compressive strength or bearing tests of each strata.
 - 6) Reports of observations for conformance of borrow material to the Project Geotechnical Report.
- B. CONTRACTOR shall submit electronic copies per Specification Section 01332, Shop Drawing Procedures.

1.4 JOB CONDITIONS

- A. Existing Structures: The Drawings show certain surface and underground structures adjacent to the Work. This information has been obtained from existing records. It is not guaranteed to be correct or complete and is shown on the Drawings for the convenience of CONTRACTOR. Explore ahead of the required excavation to determine the exact location of all existing structures. Structures shall be supported and protected from damage by CONTRACTOR. If they are broken or damaged, restore them immediately, at no additional cost to the OWNER.
- B. Existing Utilities: Locate existing underground utilities in the areas of the Work. If utilities are to remain in place, provide adequate means of protection during all operations.
 - 1. Should uncharted or incorrectly charted piping or other utilities be encountered during excavation, consult piping or utility owner and ENGINEER immediately for directions as to procedure. Cooperate with OWNER and utility owner in keeping services and facilities in operation. Repair damaged utilities to satisfaction of utility owner.
 - 2. Do not interrupt existing utilities serving facilities occupied and used by OWNER or others, except when permitted in writing by ENGINEER and then only after acceptable temporary utility services have been provided.
 - 3. Demolish and completely remove from site existing underground utilities indicated to be removed. Coordinate with utility companies for shut-off of services if lines are active.
- C. Use of Explosives:
 - 1. The use of explosives will not be permitted.
 - 2. Do not bring explosives onto site or use in the Work without prior written permission from authorities having jurisdiction. Provide copy of authorization to ENGINEER. Sole responsibility for handling, storage, and use of explosive materials when their use is permitted belongs to CONTRACTOR.
- D. Protection of Persons and Property: Barricade open excavations occurring as part of the Work and post with warning lights. Operate warning lights during hours from dusk to dawn each day and as otherwise required.
 - 1. Protect structures, utilities, sidewalks, pavements, and other facilities from damage caused by settlement, lateral movement, undermining, washout and other hazards created by earthwork operations.
- E. Dust Control: Conduct all operations meeting the requirements specified in Section 01414, Earthmoving and Dust Control.
- F. Roadways and Walks: Unless otherwise approved by ENGINEER, excavated material and materials of construction shall be so deposited, and the Work shall be so conducted, as to leave open and free for pedestrian traffic all crosswalks, and for

vehicular traffic a roadway not less than ten feet in width. All hydrants, valves, fire alarm boxes, letter boxes, and other facilities which may require access during construction shall be kept accessible for use. During the progress of the Work, maintain such crosswalks, sidewalks, and roadways in satisfactory condition and the Work shall at all times be so conducted as to cause a minimum of inconvenience to public travel, and to permit safe and convenient access to private and public property along the line of the Work.

PART 2 - PRODUCTS

2.1 SOIL MATERIALS

A. Select Backfill:

1. Select backfill for use beneath concrete slabs, cast-in-place concrete structures, manholes, asphaltic pavements and building foundations shall be crushed aggregate base course conforming to MAG Section 702 and meet the physical properties listed in Table 702-1, Aggregate Base Coarse. The depth of select backfill shall be a minimum of 12-inches unless shown otherwise on the Drawings.

B. Backfill and Fill Materials:

1. Materials acceptable for use as backfill against walls, manholes, foundations, and other structures shall be stockpiled native sandy clay or granular soils obtained from on-site excavations and which are uniformly mixed, contain no organic matter, nor contain rocks or fragments greater than 4-inches in size, nor have greater than 50-percent passing the 200 sieve. The maximum expansion of on-site materials shall be 1-percent as performed on a sample remolded to approximately 100-percent of the maximum dry density as determined in accordance with ASTM D 698 at two percent below optimum moisture content under a 100-psf surcharge pressure.
2. Backfill and fill materials from off-site sources shall consist of silty or clayey sandy soils which are uniformly mixed, contain no roots, branches, or other evident organic or deleterious matter and which have a Plasticity Index less than fifteen when tested in accordance with ASTM D 4318. The maximum particle size of imported soils or native fill shall be 4-inches with a grading required to provide compaction without nesting. The maximum expansion of off-site materials shall be 1-percent as performed on a sample remolded to approximately 100-percent of the maximum dry density as determined in accordance with ASTM D 698 at two percent below optimum moisture content under a 100-psf surcharge pressure.
3. All materials for use as backfill and fill material shall be tested by the laboratory and approved by the ENGINEER.
4. CONTRACTOR to take sample of excavated material and determine soil properties. If on-site material is determined to be unsuitable as determined by

the ENGINEER, select backfill or approved off-site backfill and fill materials shall be used.

5. Fill adjacent to structures is classified as backfill to a distance measured horizontally from the structure that is equal to the depth from the finished grade. Outside these limits the fill is classified as fill, unless otherwise specified.
6. Demolition spoils or materials shall not be used as or mixed with backfill material.

C.

C. Fill Material for Embankments.

1. Fill materials for use as embankments, and as miscellaneous landscaping materials exterior to plant facilities, shall consist of soils obtained from on-site excavations or off-site sources that are uniformly mixed, contain no organic material, rocks or fragments greater than 3-inches in size.
2. All materials for use as described above shall be tested by the laboratory and approved by the ENGINEER prior to use.

D. Pipe Bedding and Pipe Backfill:

1. Refer to Specification 15051.

PART 3 - EXECUTION

3.1 INSPECTION

- A. Provide ENGINEER with sufficient notice and with means to examine the areas and conditions under which excavating, filling, and grading are to be performed. ENGINEER will notify CONTRACTOR if conditions are found that may be detrimental to the proper and timely completion of the Work. Do not proceed with the Work until unsatisfactory conditions have been corrected in an acceptable manner.

3.2 SITE PREPARATION

- A. Clear all areas to be occupied by permanent construction or embankments of all trees, brush, roots, stumps, logs, wood and other materials and debris. Clean and strip subgrades for fills and embankments of vegetation, sod, topsoil and organic matter. All waste materials shall be removed from site and properly disposed of by CONTRACTOR. Burning is not to be permitted. Refer to and comply with the requirements of Section 02230, Clearing.

3.3 EXCAVATION

- A. Perform all excavation required to complete the Work as shown on the Drawings, specified and required. Excavations shall include earth, sand, clay, gravel, hardpan, boulders not requiring drilling and blasting for removal, decomposed rock, pavements, rubbish and all other materials within the excavation limits, except rock.

- B. Excavations for structures shall be open excavations. Provide excavation protection system(s) required by ordinances, codes, law and regulations to prevent injury to workmen and to prevent damage to new and existing structures or pipelines. Unless shown on the Drawings or specified otherwise, protection system(s) shall be utilized under the following conditions.
 - 1. Excavation Less Than Five Feet Deep: Excavations in stable rock or in soil conditions where there is no potential for a cave-in may be made with vertical sides. Under all other conditions, excavations shall be sloped and benched, shielded, or shored and braced.
 - 2. Excavations More Than Five Feet Deep: Excavations in stable rock where there is no potential for a cave-in may be made with vertical sides. Under all other conditions, excavations shall be sloped and benched, shielded or shored and braced.
 - 3. Excavation protection system(s) shall be installed and maintained in accordance with drawings submitted under Article 1.3, above.
- C. Where the structure is to be placed below the ground water table, well points, cofferdams or other acceptable methods shall be used to permit construction of said structure under dry conditions. Dry conditions shall prevail until concrete has reached sufficient strength to withstand earth and hydrostatic loads. In addition, protect excavation from flooding until all walls and floor framing up to and including grade level floors are in place and backfilling has begun. Water level shall be maintained below top of backfill at all times.
- D. Pumping of water from excavations shall be done in such a manner to prevent the carrying away of unsolidified concrete materials, and to prevent damage to the existing subgrade.
- E. The elevation of the bottom of footings shown on the Drawings shall be considered as approximate only and ENGINEER may order such changes in dimensions and elevations as may be required to secure a satisfactory footing. All structure excavations shall be hand-trimmed to permit the placing of full widths, and lengths of footings on horizontal beds. Rounded and undercut edges will not be permitted.
- F. When excavations are made below the required grades, without the written order of ENGINEER, they shall be backfilled with select backfill material, as directed by ENGINEER, at the expense of CONTRACTOR.
- G. Excavations shall be extended sufficiently on each side of structures, footings, etc., to permit setting of forms, installation of shoring or bracing or the safe sloping of banks.
- H. Subgrades for roadways and structures shall be firm, dense, and thoroughly compacted and consolidated; shall be free from mud, muck, and other soft or unsuitable materials; and shall remain firm and intact under all construction

operations. Subgrades which are otherwise solid, but which become soft or mucky on top due to construction operations, shall be reinforced with select backfill material. The finished elevation of stabilized subgrades shall not be above subgrade elevations shown on the Drawings. Proof roll all subgrades prior to placing of select fill and general fill material.

- I. Material Storage: Stockpile satisfactory excavated materials in approved areas, until required for backfill or fill. Place, grade and shape stockpiles for proper drainage.
 - 1. Locate and retain soil materials away from edge of excavations.
 - 2. Dispose of excess soil material and waste materials as specified hereinafter.
 - 3. Stockpiled excavated soils for use as subsequent fill shall be classified by laboratory as on-site granular or sandy clay soils. Use and placement of fill shall be performed as specified for each class.
 - 4. Excess soil from excavations shall be disposed of off-site. Disposal shall be in accordance with state and local regulatory requirements.
- J. Where ENGINEER considers the existing material beneath the bedding material unsuitable, CONTRACTOR shall remove same and replace it with select backfill.

3.4 UNAUTHORIZED EXCAVATION

- A. All excavation outside the lines and grades shown on the Drawings, and which is not approved by ENGINEER, together with the removal and disposal of the associated material shall be at CONTRACTOR'S expense. Unauthorized excavations shall be filled and compacted with select backfill by CONTRACTOR at their expense.

3.5 DRAINAGE, EROSION CONTROL AND DEWATERING

- A. Erosion Control:
 - 1. In general, the construction procedures outlined herein shall be implemented to assure minimum damage to the environment during construction. Take any and all additional measures required to conform to the requirements of applicable codes and regulations and the requirements specified in Section 01412, Stormwater Pollution Prevention Plan and Permit.
 - 2. Whenever possible, access and temporary roads shall be located and constructed to avoid environmental damage. Provisions shall be made to regulate drainage, avoid erosion and minimize damage to vegetation.
 - 3. Where areas must be cleared for storage of materials or temporary structures, provisions shall be made for regulating drainage and controlling erosion, subject to the ENGINEER'S approval.
 - 4. Temporary measures shall be applied to control erosion and to minimize the silting of the existing waterways, and natural ponding areas. Such measures shall include, but are not limited to, the use of berms, baled straw silt barriers, gravel or crushed stone, mulch, slope drains and other methods. These

temporary measures shall be applied to erodible materials exposed by any activities associated with the construction of this Work.

- a. Special care shall be taken to eliminate depressions that could serve as mosquito pools.
 - b. Temporary measures shall be coordinated with the construction of permanent drainage facilities and other Work to the extent practicable to assure economical, effective, and continuous erosion and silt control.
 - c. Provide special care in areas with steep slopes. Disturbance of vegetation shall be kept to a minimum to maintain stability.
5. Remove only those shrubs and grasses that must be removed for construction. Protect the remainder to preserve their erosion-control value.
 6. Install erosion and sediment control practices where shown on the Drawings and according to applicable standards, codes and specifications. The practices shall be maintained in effective working condition during construction and until the drainage area has been permanently stabilized.
 7. Mulching to be used for temporary stabilization.
 - a. Suitable Materials for Mulching:
 - 1) Unrotted straw or salt hay: 1-1/2 to 2 tons/acre.
 - 2) Asphalt emulsion or cutback asphalt: 600 to 1200 gal. /acre.
 - 3) Wood-fiber or paper-fiber (hydroseeding): 1500 lbs./ acre.
 - 4) Mulch netting (paper, jute, excelsior, cotton or plastic).
 - b. Straw or salt hay mulches should be immediately anchored using peg and twine netting or mulch anchoring tool or liquid mulch binders.
 8. After stabilization, remove all straw bale dikes, debris, etc., from the site.
 9. In the event of any temporary Work stoppage, take steps any temporary or environmental damage to the area undergoing construction.
 10. In the event CONTRACTOR repeatedly fails to satisfactorily control erosion and siltation, the OWNER reserves the right to employ outside assistance or to use its own forces to provide the corrective measures indicated. The cost of such work, plus engineering costs, will be deducted from monies due CONTRACTOR.
 11. Prevent blowing and movement of dust from exposed soil surfaces and access roads to reduce on and off-site damage and health hazards. Control may be achieved by irrigation in which the site shall be sprinkled with water until the surface is moist. The process shall be repeated as needed.

B. Drainage and Dewatering:

1. Provide and maintain adequate drainage and dewatering equipment to remove and dispose of all surface water and ground water entering excavations, or other parts of the Work. Each excavation shall be kept dry during subgrade preparation and continually thereafter until the structure to be built, therein is inspected by the ENGINEER and backfill operations have been completed and approved.
 - a. The different working areas on the site shall be kept free of surface water at all times. Install drainage ditches and dikes and shall perform all pumping

and other Work necessary to divert or remove rainfall and all other accumulations of surface water from the excavations and fill areas. The diversion and removal of surface water shall be performed in a manner that will prevent the accumulation of water behind temporary structures or at any other locations within the construction area where it may be detrimental.

- b. Water used for working or processing, resulting from dewatering operations, or containing oils or sediments that will reduce the quality of the water downstream of the point of discharge, shall not be directly discharged. Such waters shall be diverted through a settling basin or filter before being discharged.
 - c. Responsibility belongs to CONTRACTOR for the condition of any pipe, conduit or channel used for drainage purposes and all such pipes, conduits or channels shall be left clean and free of sediment.
 - d. Remove water from excavations as fast as it collects.
2. Provide, install and operate sufficient trenches, sumps, pumps, hose, piping, well points, deep wells, etc., necessary to depress and maintain the ground water level below the base of the excavations during all stages of construction operations. The ground water table shall be lowered in advance of excavation, for a sufficient period of time so as to permit dewatering of fine grain soils, and maintained two feet below the lowest subgrade excavation made until the structure has sufficient strength and weight to withstand horizontal and vertical soil and water pressures from natural ground water. The system shall be operated on a 24-hour basis and standby pumping facilities and personnel shall be provided to maintain the continued effectiveness of the system. If, in the opinion of the ENGINEER, the water levels are not being lowered or maintained as required by these Specifications, install additional or alternate dewatering devices as necessary, at no additional cost to the OWNER.
- a. Elements of the system shall be located so as to allow a continuous dewatering operation without interfering with the construction of the permanent Work. Where portions of the dewatering system are located in the area of permanent construction, submit details of the methods he proposes to construct the permanent Work in this location for the approval of the ENGINEER. Controls of ground water shall continue until the permanent construction provides sufficient dead load to withstand the hydrostatic uplift of the normal ground water, until concrete has attained sufficient strength to withstand earth and hydrostatic loads, and until all waterproofing Work has been completed. Dispose of all water removed from the excavation in such a manner so as not to endanger any portion of the Work under construction or completed. Convey water from the excavations in a closed conduit. Before discontinuing dewatering operations or permanently permitting the rise of the ground water level, computations shall be made to show that any structure affected by the water level rise is protected by backfill or other means to sustain uplift. Use a safety factor of 1.25 when making these computations.

- b. Dewatering operations shall not be discontinued without the prior authorization of the ENGINEER.
 - c. Design of dewatering system, including both drawings and calculations, shall be performed by a Registered Professional Engineer in the State of Arizona and shall be employed by CONTRACTOR. Dewatering system shall be designed so as to avoid settlement or damage to existing structures and utilities.
- C. Disposal of Water Removed by Dewatering System:
- 1. CONTRACTOR'S Dewatering System will discharge to on-site storm drain system in accordance with State and Federal regulations.
 - 2. Dispose of all water removed from the excavation in such a manner as not to endanger public health, property, or any portion of the Work under construction or completed.
 - 3. Dispose of water in such a manner as to cause no inconvenience to OWNER, ENGINEER, or others involved in Work about the site.
 - 4. Convey water from the construction site in a closed conduit. Do not use trench excavations as temporary drainage ditches
 - 5. Meter the quantity of water discharged in a manner acceptable to the Arizona Department of Water Resources.
 - 6. The discharged water from the Dewatering System shall be purchased from the Arizona Department of Water Resources at the prevailing rate.

3.6 SHEETING, SHORING AND BRACING FOR STRUCTURE EXCAVATIONS

- A. General:
- 1. Used material shall be in good condition, not damaged or excessively pitted. All steel or wood sheeting designated to remain in place shall be new. New or used sheeting may be used for temporary work.
 - 2. All timber used for breast boards (lagging) shall be new or used, meeting the requirements for Douglas Fir Dense Construction grade with a bending strength not less than 1500 psi or Southern Pine No. 2 Dense.
 - 3. All steel work for sheeting, shoring, bracing, cofferdams etc., shall be designed in accordance with the provisions of the "Specifications for the Design, Fabrication and Erection of Structural Steel for Buildings", of the AISC except that field welding will be permitted.
 - 4. Steel sheet piling shall be manufactured from steel conforming to ASTM A 328/A 328M. Steel for soldier piles, wales and braces shall be new or used and shall conform to ASTM A 36/A 36M.
 - 5. Maintain shoring and bracing in excavations regardless of time period excavations will be open. Carry down shoring and bracing as excavation progresses.
 - 6. Unless otherwise shown on the Drawings, specified, or ordered, all materials used for temporary construction shall be removed when Work is completed.

Such removal shall be made in a manner not injurious to the structure or its appearance or to adjacent Work.

7. Provide permanent steel sheet piling or pressure creosoted timber sheet piling wherever subsequent removal of sheet piling might permit lateral movement of soil under adjacent structures. Cut off tops as required, but at least three feet below grade, and leave permanently in place.
8. The clearances and types of the temporary structures, insofar as they affect the character of the finished Work, and the design of sheeting to be left in place, will be subject to the approval of ENGINEER; but responsibility for the adequacy of all sheeting, shoring, bracing, coffer-damming, etc., belongs to CONTRACTOR.
9. Safe and satisfactory sheeting, shoring and bracing shall be the entire responsibility of CONTRACTOR.
10. All municipal, County, State and Federal ordinances, codes, regulations and laws shall be observed.

B. Removal of Sheeting and Bracing:

1. Remove sheeting and bracing from excavations, unless otherwise directed in writing by ENGINEER. Removal shall be done so as to not cause injury to the Work. Removal shall be equal on both sides of excavation to ensure no unequal loads on pipe or structure.
2. Defer removal of sheeting and bracing, where removal may cause soil to come into contact with concrete, until the following conditions are satisfied:
 - a. Concrete has cured a minimum of seven days.
 - b. Wall and floor framing up to and including grade level floors are in place.

3.7 TRENCH SHIELDS

- A. Excavation of earth material below the bottom of a shield shall not exceed the limits established by ordinances, codes, laws and regulations.
- B. When using a shield for the installation of structures, the bottom of the shield shall not extend below the top of the bedding for the structures.
- C. When a shield is removed extreme care shall be taken to prevent damage to the structures or the disturbance of the bedding for structures. Structures that are disturbed shall be removed and reinstalled as specified.

3.8 PLACEMENT OF FILL AND BACKFILL

A. General:

1. All select backfill and backfill required for structures, provide the finished grades shown on the Drawings and as described herein shall be furnished, placed and compacted by CONTRACTOR.

2. Backfill excavations as promptly as Work permits, but not until completion of the following:
 - a. Acceptance by the ENGINEER of construction below finish grade, including damp proofing, waterproofing and perimeter insulation.
 - b. Inspection, testing, approval, and recording of locations of underground duct banks.
 - c. Removal of concrete formwork.
 - d. Removal of shoring and bracing, and backfilling of voids with satisfactory materials.
 - e. Removal of trash and debris.
 3. Fill containing organic materials or other unacceptable material shall be removed and replaced with approved fill material as specified.
- B. Placement of Select Backfill, Backfill and Fill:
1. Select backfill shall be placed to the grades shown on the Drawings. The lift thickness and compaction moisture content range given herein are approximate. These values shall be finally determined from the laboratory test results on the fill materials. Testing requirements shall be as specified in Paragraph 3.8.E., below.
 2. All select backfill shall be placed in horizontal loose lifts, not exceeding 8-inches in thickness, and shall be mixed and spread in a manner assuring uniform lift thickness after placing. Each lift shall be compacted by not less than two complete coverage's of the specified compactor. Select backfill shall be placed to the underside of all concrete slabs. The fill material shall extend a minimum of 16-inches outside the face of each structure and be a minimum thickness as specified in Paragraph 2.1.A. The maximum slope of select backfill to the subgrade shall be one vertical to one horizontal.
 3. Backfill and fill around and outside of structures and over select backfill shall be deposited in layers not to exceed 8-inches in uncompacted thickness and mechanically compacted, using platform type tampers. Compaction of structures backfilled by rolling will be permitted provided the desired compaction is obtained and damage to the structure is prevented. Compaction of select backfill and/or backfill by inundation with water will not be permitted. All materials shall be deposited as specified herein and as shown on the Drawings.
 4. The material shall be placed at a moisture content and density as specified under Paragraph 3.8.E., below. Provide equipment capable of adding measured amounts of water to the backfill and/or select backfill material to bring it to a condition within the range of the required moisture content. Provide equipment capable of discing, aerating, and mixing the soil to ensure reasonable uniformity of moisture content throughout the fill material and to reduce the moisture content of the borrow material by air drying, if necessary. If the subgrade or lift of earth material must be moisture conditioned before compaction, the fill material shall be sufficiently mixed or worked on the subgrade to ensure a uniform moisture content throughout the lift of material to be compacted.

Materials at moisture content in excess of the specified limit shall be dried by aeration or stockpiled for drying.

5. No backfill or fill material shall be placed when free water is standing on the surface of the area where the fill is to be placed. No compaction of fill will be permitted with free water on any portion of the fill to be compacted. No fill shall be placed or compacted in a frozen condition or on top of frozen material. Any fill containing organic materials or other unacceptable material previously described shall be removed and replaced with approved fill material prior to compaction.
6. Compaction shall be performed with equipment suitable for the type of fill material being placed. Select equipment that is capable of providing the minimum density required by these Specifications. Hand operated compacting equipment shall be used within a distance of ten feet from the wall of any completed below grade structure. Equipment shall be provided that is capable of compacting in restricted areas next to structures and around piping. The effectiveness of the equipment selected shall be tested at the commencement of compacted fill Work by construction of a small section of fill within the area where fill is to be placed. If tests on this section of fill show that the specified compaction is not obtained, increase the amount of coverages, decrease the lift thicknesses and/or obtain a different type of compactor.
7. Levels of backfill against concrete walls shall not differ by more than two feet on either side of walls, unless walls are adequately braced or all floor framing is in place up to and including grade level slabs. Particular care shall be taken to compact structure backfill, which will be beneath pipes, roads, or other surface construction or structures. In addition, wherever a trench passes through structure backfill, the structure backfill shall be placed and compacted to an elevation 12-inches above the top of the pipe before the trench is excavated. Compacted areas, in each case, shall be adequate to support the item to be constructed or placed thereon.
8. The compaction requirements specified are predicated on the use of normal materials and compaction equipment. In order to establish criteria for the placement of a controlled fill so that it will have compressibility and strength characteristics compatible with the proposed structural loadings, a series of laboratory compaction and/or compressive strength tests shall be performed on the samples of materials submitted by CONTRACTOR. From the results of the laboratory tests, the final values of the required percent compaction, the acceptable compaction moisture content range, and the maximum permissible lift thickness will be established for the fill material and construction equipment proposed.
9. Control the water content of fill material during placement within the range necessary to obtain the compaction specified. In general, the moisture content of the fill shall be within two percent of the optimum moisture content for compaction as determined by laboratory tests. Perform all necessary work to adjust the water content of the material to within the range necessary to permit the compaction specified. Do not place fill material when free water is standing

on the surface of the area where the fill is to be placed. No compaction of fill will be permitted with free water on any portion of the fill to be compacted.

10. Compact fill shall be compacted by at least two coverages of all portions of the surface of each lift by compaction equipment. One coverage is defined as the condition obtained when all portions of the surface of the fill material have been subjected to the direct contact of the compactor.
11. If the specified densities are not obtained because of improper control of placement or compaction procedures, or because of inadequate or improperly functioning compaction equipment, perform whatever Work is required to provide the required densities. This Work shall include complete removal of unacceptable fill areas, and replacement and recompaction until acceptable fill is provided.
12. If any settlement occurs, repairs will be at CONTRACTOR'S expense. Make all repairs and replacements necessary within 30 days after notice from ENGINEER or OWNER.

C. Compaction Density Requirements:

1. The degree of compaction required for all types of fills shall be as listed below. Material shall be moistened or aerated as necessary to provide the moisture content that will facilitate obtaining the specified compaction.

<u>Material</u>	<u>Required Minimum Density- Percent Compaction (ASTM D 698)</u>	<u>*Maximum Uncompacted Lift (inches)</u>
Subgrade and Subbase Fill:		
Below concrete slabs on grade	100	8
Below base of footings or mats, structural slabs	100	8
Below asphalt concrete paving	95	8
** Backfill and Fill:		
More than 5 feet below final grade	100	8
Less than 5 feet below grade	95	8
Aggregate Base Course (Select Backfill):		
Below concrete slabs or mats	95	8
Below asphalt paving	100	8
Landscape Areas	95	8

* Where applicable.

- ** Backfill shall not be used for support of facilities which are susceptible to damage from differential settlement of the fill section relative to walls.

All fill must be wetted and thoroughly mixed to achieve optimum moisture content, $\pm$ two percent, with the following exceptions: On site clayey soils optimum to plus two percent.

Natural undisturbed soils or compacted soil subsequently disturbed or removed by construction operations shall be replaced with materials compacted as specified above.

2. CONTRACTOR'S testing service shall perform tests necessary to provide data for selection of fill material and control of placement water content.
 3. Field density tests, to ensure that the specified density is being obtained, shall be performed by CONTRACTOR'S testing service during each day of compaction Work.
 4. If the tests indicate unsatisfactory compaction, provide the additional compaction necessary to obtain the specified degree of compaction. All additional compaction Work shall be performed by CONTRACTOR, at no additional cost to the OWNER, until the specified compaction is obtained. This Work shall include complete removal of unacceptable (as determined by the ENGINEER) fill areas and replacement and recompaction until acceptable fill is provided.
- D. Replacement of Unacceptable Excavated Materials: In cases where over-excavation for the replacement of unacceptable soil materials is required, the excavation shall be backfilled to the required subgrade with select backfill material and thoroughly compacted as specified in Paragraph 3.8.C., above. Sides of the excavation shall be sloped in accordance to the maximum inclinations specified for each structure location.

3.9 GRADING

- A. General: Uniformly grade areas within limits of grading under this Section, including adjacent transition areas. Smooth subgrade surfaces within specified tolerances, compact with uniform levels or slopes between points where elevations are shown on the Drawings, or between such points and existing grades.
- B. Grading Outside Building Lines: Grade areas adjacent to building lines to drain away from structures and to prevent ponding. Finish surfaces free from irregular surface changes, and as follows:
 1. Turfed Areas or Areas Covered with Gravel, Stone, Wood Chips, or Other Special Cover: Finish areas to receive topsoil or special cover to within not more than 1-inch above or below the required subgrade elevations.

2. Walks: Shape surface of areas under walks to line, grade and cross-section, with finish surface not more than 1-inch above or below the required subgrade elevation.
 3. Pavements: Shape surface of areas under pavement to line, grade and cross-section, with finish surface not more than 1/2-inch above or below the required subgrade elevation.
- C. Grading Surface of Fill Under Building Slabs: Grade smooth and even, free of voids, compacted as specified, and to required elevation. Provide final grades within a tolerance of 1/2-inch when tested with a ten foot straightedge.
- D. Compaction:
1. After grading, compact subgrade surfaces to the depth and percentage of maximum density for each area classification.

3.10 PAVEMENT SUBBASE COURSE

- A. General: Place subbase material, in layers of specified thickness, over ground surface to support pavement base course.
- B. Grade Control: During construction, maintain lines and grades including crown and cross-slope of subbase course.
- C. Shoulders: Place shoulders along edges of base course to prevent lateral movement. Construct shoulders of acceptable soil materials, placed in such quantity to compact to thickness of each base course layer. Compact and roll at least a 12-inch width of shoulder simultaneously with compacting and rolling of each layer of base course.
- D. Placing: Place base course material on prepared subgrade in layers of uniform thickness, conforming to indicated cross-section and thickness. Maintain optimum moisture content for compacting base material during placement operations.
1. When a compacted base course is shown on the Drawings to be 6-inches thick or less, place material in a single layer. When shown on the Drawings to be more than 6-inches thick, place material in equal layers, except no single layer more than 6-inches or less than 3-inches in thickness when compacted.

3.11 DISPOSAL OF EXCAVATED MATERIALS

- A. Material removed from the excavations which does not conform to the requirements for fill or is in excess of that required for backfill shall be hauled away from the project site by CONTRACTOR and disposed of in compliance with ordinances, codes, laws and regulations at no additional cost to the OWNER. Refer to and comply with the requirements of Section 02230, Clearing.

3.12 RESTORING AND RESURFACING EXISTING ROADWAYS AND FACILITIES

- A. Place 1-1/2-inches of temporary bituminous pavement immediately after backfilling trenches in paved roadways which are to be retained for permanent use. Maintain the surface of the paved area over the trench in good and safe condition during progress of the entire Work, and promptly fill all depressions over and adjacent to the trench caused by settlement of backfilling. The permanent replacement pavement shall be equal to that of the existing roadways, unless otherwise specified.
- B. Pavement, gutters, curbs, sidewalks or roadways disturbed or damaged by the CONTRACTOR'S operations, except areas designated "Pavement" or "Proposed Pavement", shall be restored by CONTRACTOR at their own expense to as good condition as they were previous to the commencement of the Work and in accordance with applicable local and state highway specifications.

3.13 TEMPORARY FENCING

- A. Furnish and install a temporary fence surrounding excavations and Work area, including the stockpile and storage areas. Fence shall have openings only at vehicular, equipment and worker access points.

3.14 ENVIRONMENTAL PROTECTION AND RESTORATION

- A. Refer and comply with the requirements of Section 01412, Stormwater Pollution Prevention Plan and Permit.

++ END OF SECTION ++

SECTION 02532

POLYMER CONCRETE INSERTS

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. The purpose of this specification is to provide requirements to furnish corrosion proof polymer concrete precast structural inserts for structural rehabilitation of existing manholes to eliminate existing corrosion, infiltration and inflow.

1.2 WORK OF THIS SECTION

- A. The requirements of this section includes providing a structural polymer concrete manhole insert system such that manholes become structurally sound and impervious to ground water, soil, debris, and are resistant to hydrogen sulfide corrosion. Manhole rehabilitation riser sections shall be constructed of precast sections comprised of resin, sand, aggregate, and FRP bar reinforcement.
- B. The CONTRACTOR to provide by-passing of flows around manholes if required per Specification Section 02145, Diversion of Sewage Flow and Dewatering.
- C. The CONTRACTOR to obtain confined space entry permit to perform the work.
- D. The CONTRACTOR is responsible for the complete installation of the manhole insert system including the removal of all old loose material, any pre-installation surface preparation and any structural manhole insert component.

1.3 QUALITY ASSURANCE

- A. Manhole rehabilitation riser sections shall conform to the following design criteria:
 1. ASTM C 478 Standard Specification for Precast Reinforced Concrete Manhole Sections
 2. ASTM C 579 Standard Test Methods for Compressive Strength of Chemical-Resistant Mortars, Grouts, Monolithic, Surfacing, and Polymer Concretes
 3. ASTM C 443 Standard Specification for Joints for Concrete Pipe and Manholes Using Rubber Gaskets
 4. ASTM C 580 Standard Test Method for Flexural Strength and Modulus of Elasticity of Chemical-Resistant Mortars, Grouts, Monolithic Surfacing, and Polymer Concretes
 5. ASTM C 857 Standard Practice for Minimum Structural Design Loading for Underground Utility Structures

6. ACI 350-06 Code Requirements for Environmental Engineering Concrete Structures & Commentary
7. ACI 440.1R-15 Guide for the Design and Construction of Structural Concrete Reinforced with Fiber-Reinforced Polymer (FRP) Bars
8. ACI 548.6R-96 Polymer Concrete-Structural Applications State-of-the-Art Report
9. ASTM D 648 Test Method for Deflection Temperature of Plastics Under Flexural Load in Edgewise Position
10. ASTM D 6783 Standard Specification for Polymer Concrete Pipe
11. ASTM D 2584 Test Method for Ignition Loss of Cured Reinforced Resins
12. ASTM C 923 Standard Specifications for Resilient Connectors between Concrete Manholes Structures and Pipe
13. ASTM C 990 Standard Specification for Joints for Concrete Pipe, Manholes and Precast Box Sections using Preformed Flexible Joint Sealants
14. ASTM C 497 Test Methods for Concrete Pipe, Manhole Sections, or Tile

1.4 SUBMITTALS

- A. The CONTRACTOR shall provide submittal information to the ENGINEER in accordance with Specification Section 01330, Submittals.
- B. Manufacturer's Qualifications:
 1. Manufacturers of polymer concrete insert shall have a minimum of ten years of experience producing structural inserts, and shall be able to show evidence of at least five installations in satisfactory operation in the last five years.
- C. Installer's Qualifications: The installer shall be able to show experience in the installation of the proposed structural manhole insert for more than five (5) years. References shall be provided for a minimum of five (5) jobs. Information provided shall include a description of the job, the location of the job, the value of the job, the client name, and the client contact person for the job including name, title, address, and phone number.
- D. Submit manufacturer's data and details of following items for approval:
 1. Shop drawings of manhole sections, base units and construction details, jointing methods, materials, and dimensions.
 2. Summary of criteria used in manhole design including, as minimum, material properties, loading criteria, and dimensions assumed. Include certification from manufacturer that polymer concrete manhole design meets or exceeds the load and strength requirements of ASTM C 478 and ASTM C 857, reinforced in accordance with ACI 440.1R-15. Include current ISO 9001:2015 certification.
 3. Materials to be used in fabricating pipe drop connections.
 4. Materials to be used for grade adjustments.
 5. Materials to be used for bench area finishing.
 6. Proof of independent Chemical Resistance testing conducted in accordance with the Standard Specifications for Public Works Construction.

- 7. Submitted sealed drawings by a registered Professional Engineer in the Arizona.
- E. Comply with the requirements of Specification Section 01332, Shop Drawing Procedures.
- F. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures.
- G. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

1.5 MANUFACTURER

- A. Approved structure manufacturers:
 - 1. Armorock Polymer Concrete; www.armorock.com 702-824-9702
 - 2. Or Equal

PART 2 - PRODUCTS

2.1 REQUIREMENTS

- A. Insert system shall contain sufficient structural strength to carry all live, dead and hydrostatic loads for the installation. The insert system will be capable of providing all necessary structural support and would installed on to existing rehabilitated manhole bench.
- B. Corrosion proof polymer concrete manhole insert riser, cone sections and related components shall conform to the structural intent of ASTM C – 478.
- C. Riser sections, eccentric cones and flat lids (with approval on an individual basis by city) are to be provided with flush edge configurations assembled with the appropriate alignment guides and butyl mastic to make a continuous and uniform insert structure manhole.

- D. Polymer concrete structural insert manhole riser and cone sections are to be provided in various lengths in combination to provide correct height with fewest joints.
- E. The polymer concrete structural insert will have a minimum wall thickness of 2-inches and an outside diameter clearance of 1.5-inches, assuming the existing structure is intact with original inside diameter. Additional wall thickness may be required for interior dimensions in excess of 60-inches.

2.2 DESIGN CRITERIA

- A. Polymer Concrete Structural Manhole risers and cones shall be designed by manufacturer to meet the intent of ASTM C 478 with allowable compositional and sizing differences as designed by the polymer concrete manufacturer.
 - 1. AASHTO HS-20 design or as required loading applied to manhole cover and transition and base slabs.
 - 2. Polymer manholes will be designed based upon live and dead load criteria in ASTM C 857 and ACI 350-06.
 - 3. Reinforcement – Shall use acid resistant reinforcement (FRP Bar) in accordance with ACI 440.1R-06 as applicable for polymer concrete design.
 - 4. Polymer Concrete Mix Design shall consist of thermosetting resin, sand, and aggregate. No Portland cement shall be allowed as part of the mix design matrix. All sand and aggregate shall be inert in an acidic environment.

PART 3 - INSTALLATION

- A. Manhole installation should strictly follow the manufactures recommended installation procedures.
 - 1. Excavate the area around the manhole as necessary to provide for removal of existing cone and castings while preventing soil and debris from falling into the manhole.
 - 2. Follow all requirements for safety and ventilation in accordance with all applicable federal, state and local requirements.
 - 3. Provide means, labor and equipment to prevent solid waste contamination during construction activities. If necessary, install and operate sewage diversion pumping equipment to maintain sewage flows without backup, overflow or spill as per specification 02145, Diversion of Sewage Flow and Dewatering.
 - 4. Make repairs necessary to existing manhole in order to receive inserts per contract drawings. Power -wash / clean any loose debris from wall areas. Remove dirt, grease, and debris in bench area to prepare bench surfaces for resurfacing. Remove all debris. Repair any active leaks. Repair and level the bench to prepare for polymer insert installation per contract drawings. Saw cut polymer riser section if necessary, to accommodate pipe entry and bench slopes.
 - 5. Depending on invert configuration the bench area should be prepared by building up the bench section with a Portland cement and sand mixture to provide a clean level

surface to receive the riser. This build up should allow for a level surface above the existing top of the highest pipe. If pipe inverts are severely staggered the initial polymer sleeve can be modified in the field to accommodate the difference in bench slopes. After grout has set, lower the first riser section, clean and wipe down the polymer riser wall above the bench to allow for application of the FRP per Specification Section 02720, Fiber Reinforced Polymer.

6. When the new bench configuration is sufficiently dry and clean, a shell coating of FRP will be built over the bench. The FRP shell should continue a minimum of 4-feet and 6-inches up the polymer riser face to allow for polymer interlinking or as shown per the Contract Drawings. No additional surface coating is required as the riser sections are structural and corrosive proof throughout.
7. All non-booted or flexible coupler pipe entry areas will be grouted by product as listed per the OWNER'S approved products list.
8. After the initial bottom seal has cured the additional remaining riser sections and cone shall be installed using the appropriate wall alignment guides, gaskets and/or mastic. The risers will be offloaded and lifted into place with approved manufacturer's lifting device. No other handling apparatus will be acceptable for handling material.
9. Fill the annular space between old and new structure with a half sack slurry mix / mixed fill, stabilized sand or stone (85-percent standard proctor) or a backfill material shall be subject to approval by ENGINEER and OWNER.
10. Install city provided grade rings and frame and cover, level to grade. Grout where required using product as listed per OWNER'S approved products list.
11. Finish the remainder of the backfill, remove any deleterious materials and debris from the manhole and replace pavement if required.
12. Upon completion of the installation process the CONTRACTOR shall remove all surplus materials, protective coverings, accumulated rubbish, and thoroughly clean all surfaces and repair any other installation related damage as required by OWNER. All external surface areas shall be repaired and returned to its original or required condition. All debris shall be collected and hauled off to a proper disposal site as required by local, state and federal regulations.

B. Certification / Warranty / Inspection

1. Provide certification to the OWNER for final acceptance of material and installation. The material supplier shall provide a copy of the appropriate test results that show that the polymer material meets the requirements listed in this specification and for any other qualified specification required by the OWNER for this project at time of bid.
2. Facility Quality Control should be maintained by adhering to ISO 9001:2018 for manufacturing. All manufacturers to be ISO 9001:2018 certified. All fabrication to take place in an all polymer concrete production facility.
3. Manufacturer to provide references of 5 previous projects in the last 5 years performed with both OWNER and CONTRACTOR for reference and review by OWNER. Polymer concrete shall be cast in a polymer only facility and shall not be manufactured in a cementitious concrete facility.

4. The installing CONTRACTOR shall demonstrate applicable experience and performance history by submitting references from a minimum of 3 other rehabilitation projects or shall have a minimum of five years' experience installing standard sanitary sewer manholes and a certification of training from the manufacturer on a structural manhole insert system.
5. The polymer concrete structural insert riser, cone and flat lid (with approval on an individual basis by city) sections shall be subject to inspection and approval by the OWNER or the ENGINEER. At the time of inspection all material will be examined for specification compliance and approved drawings. Any sections damaged after delivery and not deemed repairable by the Manufacturer's Representative and ENGINEER will be removed and replaced at CONTRACTOR's expense.
6. Manufacturer shall provide OWNER with a 50 year corrosion warranty on the structural inserts at close of project with date of installation and other project information clearly shown.
7. Project Warranty: The contractor shall warrant all material and workmanship performed under this contract for a period of **one (1) year** from the date of final acceptance or as stated in the project Contract Documents whichever is longer.
8. Material Warranty: The manhole inserts shall be free of defects that will affect the long-term line and operation of the manhole. The CONTRACTOR shall warrant in writing all materials provided and work performed under this contract for a period of **five (5) years** from the date of final acceptance.
9. Insert: Insert material(s) shall be tested for a design life of minimum for **fifty (50) years** against deterioration from H₂S corrosion and typical chemical found in municipal sanitary sewer.

++ END OF SECTION ++

SECTION 02720

FIBER REINFORCED POLYMER (FRP)

PART 1 - GENERAL

1.1 DESCRIPTION

- A. This specification is intended to provide guidelines and performance requirements for the rehabilitation of structures using internally and externally bonded fiber reinforced polymer (FRP) composite systems. The rehabilitation system described herein shall include a glass fiber reinforced polymer (GFRP) as the main reinforcement system. The FRP can also include layers of other materials to increase stiffness of the FRP composite system in a cost-effective manner. The additional materials used to increase the stiffness of the FRP system may be proprietary to the FRP material supplier. Any materials used for the FRP system shall be subject to the approval of the ENGINEER.

1. Scope:

- a. The work includes furnishing of all materials, labor, equipment, and services for the supply, installation, and finish of structural strengthening and/or rehabilitation using a FRP system.
- b. The CONTRACTOR shall be licensed in the State of Arizona and shall include all engineering, labor, materials, tools, equipment, appliances, and services required to design, deliver, furnish all items necessary for the proper execution and completion of the work as shown in the Contract Documents, as specified herein and/or as required by job conditions. A detailed design of the FRP system for the specific site conditions shall be included with the installation of the FRP system by the FRP installer selected by the CONTRACTOR. All items not shown or specified, but which are necessary for the proper execution and completion of the Work, shall be provided by the CONTRACTOR.
- c. The extent of the FRP composite system applied to shall be as defined herein and as shown on the Design Drawings.
- d. The CONTRACTOR shall cooperate and coordinate with all other trades in executing the work described in the Contract Documents.
- e. The CONTRACTOR shall deliver finished product(s) including all materials, labor, equipment, and services necessary for product installation, all quality controls and material samples required for performance tests, final inspection, and warranty work, all as specified in these Contract Documents and at the quantities of each component to complete the work.
- f. The prices submitted by the CONTRACTOR shall include all costs of permits, labor, equipment, and materials for the various items necessary for rehabilitation materials and processes in accordance with these Technical Specifications complete and in place, including all items of work not specifically identified herein which are required to make the product perform as intended. All CONTRACTOR costs

to deliver the final product shall be included in the respective lump sum and/or unit price bid items depicted in the Proposal.

- g. It shall be the responsibility of the CONTRACTOR to monitor the weather and make necessary adjustments in means and methods while work is being conducted.

1.2 SAFETY REQUIREMENTS:

- A. The CONTRACTOR shall conform to all work safety and health requirements of pertinent regulatory agencies and shall secure the site for working conditions in compliance with the same. The CONTRACTOR shall perform existing material testing as required by regulatory regulation requirements and provide/install/maintain such signs and/or other devices as are necessary for safety at the work site.
- B. Sewers and appurtenant structures are considered confined spaces and are subject to federal, state, and local regulations governing confined space entry and safety procedures. The CONTRACTOR shall obtain all necessary permits required to enter and work in confined spaces.

1.3 NOTIFICATION:

- A. It shall be the CONTRACTOR's responsibility to notify businesses and the public. The CONTRACTOR shall notify all utility companies involved whenever a utility is to be cut, tapped, moved, or in any way disturbed from its original placement. Sufficient notice shall be given to the utility company so that its users can be informed of any disruption of service. Such notice shall be given no less than 48 hours in advance.
- B. It is the CONTRACTOR's responsibility to inform the ENGINEER of construction activities which may lead to interruption in pedestrian or vehicular traffic or sewer service. The CONTRACTOR shall provide 72-hour notice prior to construction to verify all applicable businesses and residents have been alerted.

1.4 QUALITY CONTROL

- A. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown on the Drawings or specified.
 - 1. ACI 117-90, Specifications for Tolerances for Concrete Construction and Materials, and Commentary.
 - 2. ACI 224.1R-0, Causes, Evaluation, and Repair of Cracks in Concrete Structures.
 - 3. ACI 224R-01, Control of Cracking in Concrete Structures.
 - 4. ACI 440R-07, State-of-the-Art Report on Fiber Reinforced Plastic Reinforcement for Concrete Structures.
- B. ACI—American Concrete Institute
 - 1. 503R-93: Use of Epoxy Compounds with Concrete.
 - 2. 503.4-R92: Standard Specifications for Repairing Concrete with Epoxy Mortars.

3. 503.6R-97: Guide for the Application of Epoxy and Latex Adhesives for Bonding Freshly Mixed and Hardened Concretes.
 4. 546R-04: Concrete Repair Guide.
- C. ASTM- American Society for Testing and Materials
1. D3039: Test Method for Tensile Properties of Polymer Matrix Composite Materials
 2. D3418: Test Method for Transition Temperatures of Polymers by Differential Scanning Calorimetry.
 3. D5687: Guide for Preparation of Flat Composite Panels with Processing Guidelines for Specimen Preparation.
 4. D7565: Standard Test Method for Determining Tensile Properties of Fiber Reinforced Polymer Matrix Composites Used for Strengthening of Civil Structures.
 5. D7290: Standard Practice for Evaluating Material Property Characteristic Values for Polymeric Composites for Civil Engineering Structural Applications.
 6. D638 Standard Test Method for Tensile Properties of Plastics. D695 Standard Test Method for Compressive Properties of Rigid Plastics.
 7. D790: Standard Test Methods for Flexural Properties of Unreinforced and Reinforced Plastics and Electrical Insulating Materials.
 8. D543: Standard Practices for Evaluating the Resistance of Plastics to Chemical Reagents.
- D. NACE International
1. SSPC-SP12 / NACE 5: Surface Preparation and Cleaning of Steel and Other Hard Materials by High- and Ultra High- Pressure Water Jetting Prior to Recoating.
 2. SSPC-SP13 / NACE 6: Surface Preparation of Concrete.
- E. ICRI- International Concrete Repair Institute
1. No. 03732: Selecting and Specifying Concrete Surface Preparation for Sealers, Coatings, and Polymer Overlays.
 2. No. 03733: Guide for Selecting and Specifying Materials for Repairs of Concrete Surfaces.

1.5 CERTIFIED INSTALLER

- A. The CONTRACTOR must be trained and certified by the FRP material supplier and approved by the OWNER and ENGINEER for installation of the FRP system. A training outline for the certification program shall be submitted to the OWNER or ENGINEER for review. The CONTRACTOR shall provide evidence that the on-site Superintendent responsible for supervision of the FRP installation crew has received the FRP material supplier's training.
- B. The certified installer of the FRP system shall show experience in the installation of the proposed rehabilitation method for all jobs within the last five (5) years. Information provided shall include a description of the job, the location of the job, the value of the job,

the Owner name, and the Owner contact person for the job including name, title, address, and phone number.

- C. The CONTRACTOR shall appoint a Quality Assurance Manager who will be on-site full-time during FRP installation to take full responsibility for the quality of the work. The Quality Assurance Manager shall be fully certified by the FRP material supplier.

1.6 SURFACE PREPARATION

- A. The CONTRACTOR shall perform surface preparation per Technical Specification Section 02146, Sewer Pipe and Sewer Structure Cleaning

1.7 PERFORMANCE WORK STATEMENT (PWS)

- A. A Performance Work Statement (PWS) shall be submitted by the CONTRACTOR for the approval of OWNER and ENGINEER. The PWS shall include submittals consisting of Design Drawings, sketches, FRP material supplier's literature, catalog descriptions, or other descriptions in sufficient detail to allow a decision for submittal acceptance.

1.8 SUBMITTALS

- A. The CONTRACTOR shall submit the following information to the OWNER or ENGINEER in accordance with Technical Specification Section 01330, Submittals.
1. Performance work statement (PWS).
 2. Certifications of applicator with written consent from the FRP material supplier that the CONTRACTOR has been trained in proper application of FRP material supplier's systems.
 3. Physical and mechanical properties shall be provided of each proposed FRP system and other components, including test results conducted per the ASTM standards as applicable to the project. Physical properties shall, at the minimum include:
 - a. Laminate density, Laminate Shore D hardness (ASTM D2240).
 - b. Corrosion resistance (ASTM D543).
 - c. Fully cured resin water absorption (ASTM D570).
 - d. Glass transition temperature (ASTM E1640).
 - e. Fabric ply thickness, resin cure times.
 - f. Volatile organic compound (VOC) of laminate materials (ASTM D2369).
 - g. FRP fabric roll dimensions range.
 4. Mechanical properties shall, at the minimum include:
 - a. Minimum ultimate and characteristic design tensile strength.
 - b. Elongation percent.
 - c. Tensile modulus of the laminate in fiber direction (per ASTM D3039, D7290, respectively).
 - d. Minimum design laminate thickness per layer.
 5. FRP material supplier's Safety Data Sheets (SDS) for all materials brought on-site.
 6. Product Data Sheets (PDS) for the components of the FRP system.

7. Safety Plan in compliance with local, state, and federal requirements.
8. Detailed installation plan describing all work items.
9. FRP material supplier test data.
10. FRP system design ENGINEER's qualifications and the criteria incorporated into the design. Design calculations submitted shall be sealed by a professional Structural ENGINEER registered in the State of Arizona.
11. Product installation procedure.
12. QA/QC plan including field sampling and testing.
13. FRP Design Drawings and Technical Specifications.
14. Documentation of CONTRACTOR's Experience: CONTRACTOR shall provide references for all jobs within the last five (5) years that were either completed or under construction using the proposed rehabilitation method and materials.
15. Qualifications, certifications / training received of installer including experience on similar projects.
16. Submit FRP material supplier's published product and installation literature and published quality control testing criteria data for the proposed FRP system.
17. Submit independent test report showing that the physical properties of the proposed materials meet the requirements of these Technical Specifications and will meet or exceed the physical properties given in the FRP material supplier's published literature submitted as required by Part 2 – PRODUCTS.
18. Submit a report of the proposed installation process and procedures as directed by the FRP system to be used. The report shall include information specific to the installation instructions and any unique site condition(s) that must be taken into consideration including surface preparation, environmental limitations such as humidity, surface temperature, set times, access coordination, installation timing and identification and any coordination requirements/requests from the OWNER or ENGINEER.
19. Shop Drawings: The CONTRACTOR shall provide shop drawings of proposed materials for approval prior to ordering. Shop drawings shall depict:
 - a. All measurements made by the CONTRACTOR to verify critical dimensions.
 - b. Material(s) proposed for construction.
 - c. Critical dimensions, jointing, overlap, and connections, necessity for fasteners, and anchors with sizes, spacing, and location if proposed, connections, attachments, and openings.
 - d. Color(s).
20. Documentation of the control checks for materials used in the fabrications and each part of the fabrication materials including resin and fabric(s).
21. Warranty: CONTRACTOR shall submit warranty documents signed by an officer of the company and notarized stating the terms and conditions of the warranty identified in Paragraph 1.11 – WARRANTY.
22. Any item the CONTRACTOR proposes to utilize as an "or equal" to that proposed in the Contract Documents.

1.9 QUALITY ASSURANCE/QUALITY CONTROL

- A. Examination: Each component part shall be examined by the CONTRACTOR for dimensional requirements, soundness, and workmanship in accordance with ASTM C478.
- B. Composition Control: The FRP installer shall control and document the fiber to resin ratio content for each part of the installation. Documentation of the control checks shall be maintained by the CONTRACTOR and FRP installer. Proper content shall be shown by fiber to resin ratio usage rate checks, in accordance with the FRP material supplier's recommended material composition.
- C. Testing Records: Records of all testing shall be submitted in accordance with Technical Specification Section 01330, Submittals.

1.10 INSPECTION POINTS

- A. The CONTRACTOR shall keep the OWNER or ENGINEER informed of milestone progress for the following items:
 - 1. Completion of removal of unsound material and pressure wash cleaning.
 - 2. Completion of leveling/seal material placement and surface preparation prior to FRP placement.
 - 3. Completion of FRP placement.
 - 4. Completion of FRP protective coating.

1.11 WARRANTY

- A. The CONTRACTOR shall replace promptly and at no expense to the OWNER, any FRP system materials which fail during the warranty period. A failure is defined as blistering, cracking, embrittlement, softening, peeling, pitting, or adhesion failure to the substrate. Warranty work shall include all ancillary items including, but not limited to permits, traffic control, exposure, flow control, labor, equipment, bypass pumping, third party quality control inspection and all other work required to complete the repair/replacement work. Any testing performed during construction, including but not limited to, spark testing and adhesion testing, shall not in any way modify this Warranty or relieve the CONTRACTOR from its responsibility to repair or replace a failed FRP system. Mechanical damage due to maintenance operations or ancillary work on the FRP system by others is excluded from this Warranty. Warranty and durations are described below:
 - 1. Project Warranty: The CONTRACTOR shall warrant all materials and workmanship performed under this contract for a period of **five (5) years** from the date of final acceptance of the project and Letter of Acceptance by the OWNER or as stated in the project Contract Documents whichever is longer.
 - 2. Materials Warranty: The FRP system installed shall be free of defects that will affect the long-term life and operation of the system. The CONTRACTOR shall warrant in writing all materials provided under this contract for a period of **five (5) years** from the date of final acceptance of the project and Letter of Acceptance by the OWNER.

3. Design Life: Material(s) shall provide a design life of a minimum of **fifty (50) years** against deterioration from H₂S corrosion and typical chemicals found in municipal sanitary sewage.

PART 2 - PRODUCTS

2.1 ACCEPTABLE FRP MATERIAL SUPPLIERS & COMPOSITE STRENGTHENING SYSTEM

A. General

1. The Materials for the FRP system must be suitably qualified, compliant with industry standards and properties provided herein, and supplied by a reputable FRP material supplier approved by the OWNER or ENGINEER. Before the materials are ordered or work is performed, the CONTRACTOR must identify which materials are proposed to be utilized and from which FRP material supplier(s) CONTRACTOR proposes to obtain the materials.
2. Quality Control Testing and Certification: FRP material supplier shall certify that all material(s) provided conforms to the project specification or provide a proposed substitution per paragraph 1.9. The quality control test results for all proposed materials shall be submitted per Technical Specification Section 01330, Submittals.
3. Labeling, packaging, and storage shall include any health hazard warnings, precautions for handling and recommended first aid procedures in case of contact.
4. All materials used shall conform to the project Technical Specifications and other specifications referenced within and subject to the approval of the OWNER or ENGINEER.

B. Design Loads

1. The FRP system shall be designed for the following loads as they apply for the specific site conditions:
 - a. Earth load (a long-term loading).
 - b. External pressure from ground water to ground surface elevation. (a long-term loading).
 - c. Live load (a short-term loading).
 - d. Internal working pressure (a long-term loading, where applicable)
 - e. Water weight (a long-term loading)
 - f. Host pipe weight (a long-term loading)
 - g. Wind loads (if applicable)
 - h. Seismic (if applicable)

2.2 GLASS FABRIC

- A. Glass fabric layers may be used where loading conditions allow.

- B. A glass fabric layer shall be used to create a dielectric barrier if the host pipe is conducive to electricity (steel, iron, etc.). The minimum dielectric constant (κ) shall be 4 (ASTM D150) for the dielectric barrier. Glass fabric shall have the minimum physical and mechanical properties shown in the table:

Longitudinal (0°) Direction:			
Weight (Fabric Only)	20 oz/yd ² (670 g/m ²)	27 oz/yd ² (904 g/m ²)	20 oz/yd ² (670 g/m ²)
Fiber Orientation	Uniaxial	Uniaxial	Biaxial
Tensile Strength	63.7 ksi (439 MPa)	85.2 ksi (587 MPa)	20 ksi (138 MPa)
Tensile Modulus	2,940 ksi (20,270 MPa)	3,980 ksi (27,440 MPa)	1,500 ksi (10,342 MPa)
Ultimate Elongation	2.20%	2.30%	2.10%
Breaking Force	2,365 lb./in. (4,140 N/cm)	3,490 lb./in. (6,110 N/cm)	1,120 lb./in. (1,961 N/cm)
Transverse (90°) Direction:			
Tensile Strength	N/A	N/A	20 ksi (138 MPa)
Tensile Modulus	N/A	N/A	1,500 ksi (10,342 MPa)
Ultimate Elongation	N/A	N/A	2.10%
Breaking Force	N/A	N/A	1,120 lb./in. (1,961 N/cm)
Ply Thickness	.037 in	N/A	0.040 in. (1.0 mm)

2.3 ALTERNATIVE FABRICS

- A. Alternative fabrics can be used for improved ring stiffness. Such materials can be proprietary and are subject to approval. Proposed alternative fabrics shall be submitted per Technical Specification Section 01330, Submittals. Although revealing the chemical composition of alternative systems is at the discretion of the FRP material supplier, the physical and mechanical properties of such materials and the overall system are subject to the review and approval process.

2.4 SATURATING RESIN – POLYMER

- A. Fabrics shall be saturated with a two component, high strength, and low viscosity structural epoxy or other resin system as listed in Section 2.5. The resin shall have low viscosity and long pot life, with a fast cure time designed for high volume saturation of heavy reinforcement fabrics using a saturation machine or hand tools as per project requirements to saturate the fabric thoroughly and uniformly.

1. The resin system shall be resistant to service environment conditions, including but not limited to moisture, corrosive gases, elevated temperature, and chemicals in the fluid flowing inside the municipal sanitary sewer host pipe.
 2. The resin system shall not be diluted with any organic solvents such as a thinner.
 3. The resin system shall not be used outside of the FRP material supplier's specified pot life.
 4. The resin system shall not be applied on cold or frozen surfaces.
 5. Only moisture tolerant resins shall be allowed to be applied on wet surfaces.
 6. The temperature and moisture range for application shall be within the FRP material supplier's specified values.
- B. Primer: For systems requiring a primer, the primer shall have sufficiently low viscosity to penetrate the concrete substrate and provide an adhesive bond for the thickened resin applied.
- C. Thickened Resin: A thickened resin system, which consists of the saturating resin and fumed silica (or similar), or the specified tack coat, both supplied by the FRP material supplier as premixed or mixed at the site according to the FRP material supplier's recommended procedure, shall be used to provide a smooth surface for the application of the FRP laminate. The thickened resin system shall be used to fill in surface voids and even out the concrete substrate. It is permissible to use a thin coat of thickened epoxy between laminae to enhance adhesion.
- D. Saturating Resins: Saturating resins shall be 100% solids formulation with low odor conforming to the minimum physical and mechanical properties as shown in the table below.

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Epoxy Resin Properties	
Color – Part “A” is pigmented syrup, Part “B” is amber liquid.	
Viscosity Mixed at 770 F (250 C)	1,500 – 1,600 cps
Pot Life at 770 F (250 C)	3 – 4 h (thin film set time)
Cure time (>55 °F)	24 h
Density at 392 °F (200 °C)	Part A: 70.5 lb./ft ³ (1.13 kg/L) Part B: 62.4 lb./ ft ³ (1.00 kg/L)
Tensile Strength (ASTM D638) Tensile Modulus (ASTM D638)	7,150 psi (49 MPa) 289,000 psi (1,992 MPa)
Flexural Strength (ASTM D790) Flexural Modulus (ASTM D790)	11,140 psi (77 MPa) 252,400 psi (1,740 MPa)
Compressive Strength (ASTM D695) Compressive Modulus (ASTM D695)	13,000 psi (90 MPa) 350,000 psi (240 MPa)
Water Absorption (% gain) in 24 hours	< 1%
Expansion Coefficient [-37.40 – 40.10 °C] Expansion Coefficient [1,200 – 2,220 °C]	78 ×10 ⁻⁶ m/m °C 151.8 ×10 ⁻⁶ m/m °C

2.5 OTHER MATERIALS

- A. The CONTRACTOR shall provide compatible primer, filler and other materials recommended by the FRP material supplier as needed for the proper installation of the surface bonded FRP system. Vinyl ester resins can be used to saturate the FRP fabrics as an alternative to epoxy. The vinyl ester (or epoxy) resins shall have the minimum physical and mechanical properties as provided in the Table below. It should be noted that use of vinyl ester resins will require additional safety measures when applied in confined space due to styrene or other monomer emissions. Cure times of vinyl ester resins vary in a wide range, and if the project requires faster cure times than those provided by the FRP material supplier in ambient temperature, then heating of resin during application should be considered commensurate with the FRP material supplier’s curing schedule.

Sample Epoxy or Vinyl Ester Resin Properties Table	
Viscosity (ISO 2555)	200 – 250 cps
Gel Time	30 min
Density	62.4 lb./ ft ³ (1.00 kg/L)
Tensile Strength (ASTM D638) Tensile Modulus (ASTM D638)	12,800 psi (88 MPa) 460,000 psi (3.2 GPa)
Flexural Strength (ASTM D790) Flexural Modulus (ASTM D790)	22,000 psi (153 MPa) 500,000 psi (3.5 GPa)
Compressive Strength (ASTM D695)	17,600 psi (121 MPa)
Water Absorption (% gain) in 24 hours	< 1%
Expansion Coefficient [-37.40 – 40.10 °C] Expansion Coefficient [1,200 – 2,220 °C]	78×10^{-6} m/m °C 151.8×10^{-6} m/m °C

PART 3 - EXECUTION

3.1 GENERAL PROCEDURES

- A. The design of the FRP reinforcement shall conform to the design provisions given for the project and applicable standards.

3.2 INSTALLATION BY THE WET LAYUP METHOD

- A. The CONTRACTOR may propose installation techniques and design improvements to meet the project goals and design requirements outlined in the Design Drawings and Technical Specifications. Such proposed changes shall be submitted as part of the Performance Work Statement (PWS) per paragraph 1.7.
- B. Surface Preparation
1. The CONTRACTOR shall prepare the surface receiving FRP per Technical Specification Section 02146, Sewer Pipe and Sewer Structure Cleaning including the following:
 - a. The CONTRACTOR shall expose and inspect the pipe and manholes to be rehabilitated with the FRP system. Inspection shall verify the existing conditions of the concrete.
 - b. All proposed repairs and restoration technique submittals shall be approved prior to surface preparation.

- c. The CONTRACTOR shall examine the existing conditions to identify potential obstructions and constraints, shall verify dimensions, geometry, and access locations.
 - d. An adhesive bond with adequate strength shall be provided between the first layer of the FRP system and substrate. Surface preparation shall also promote continuous intimate contact between the FRP system and substrate by providing a clean, and smooth surface.
- C. Surface Grinding/Blasting: All irregularities, unevenness, and sharp protrusions in the surface profile shall be mechanically ground away to a smooth surface with CSP 3/CSP 2 surface profile for substrate. Disk grinders or other similar devices shall be used to remove previously applied coatings, stain, paint, or any other surface substance that may affect the bond. Voids or depressions with diameters larger than 1 ½ in. or depths greater than 1/8 in., when measured from a 12-in. straight edge placed on the surface, shall be filled according to Section 3.2.E.
- D. Crack Injection: In case of host pipe having deep cracks and fractures to an extent that it imposes a risk of failure of the substrate during FRP application, these cracks and fractures shall be filled, and the host pipe shall be pre-strengthened by crack injection. Where crack injection is necessary, internal, or external cracks at a width specified by the ENGINEER shall be filled using pressure injection of epoxy. The FRP system shall be installed no earlier than 12 hours after crack injection. Any surface roughness caused by injection shall be removed as per Section 3.2.C.
- E. Surface Profiling: After surface grinding, if necessary, any remaining unevenness in the surface greater than that specified in Section 3.2.C, fins, protrusions, voids, bug holes, depressions, eroded or uneven surfaces shall be filled and smoothed over by using putty made of epoxy resin mortar or polymer cement mortar.
- F. Surface Cleaning: Substrate concrete and finished surface of concrete shall be cleaned prior to application of the FRP system. Cleaning shall remove any dust, laitance, grease, oil, curing compounds, wax, stains, coatings (as necessary), surface lubricants, foreign particles, weathered or deteriorated concrete layers, or any other bond-inhibiting material. If power wash is used, the surface shall be allowed to dry thoroughly before installing the FRP system. If grit blast media is used, the surface shall be cleaned of expended media and all loose materials before installing the FRP system. The clean surface shall be protected against redeposition of any bond-inhibiting materials.
- G. Installation
 - 1. This section specifies general installation procedures for the wet lay-up of the FRP system composed of FRP laminae. The CONTRACTOR shall submit a site-specific installation procedure in the PWS, which shall, at the minimum, include the following measures.
 - a. Environmental Conditions: Environmental conditions for installation shall be examined before and during installation of the FRP system to ensure conformity to

the Contract Documents and FRP Material Supplier's recommendations. Primers, putty, saturating resins, or adhesives shall not be applied on cold, frozen, damp, or wet surfaces, unless the resin is moisture tolerant. Ambient, concrete, and steel surface temperatures shall be within 45-95 °F (7-35 °C). Moisture level on all contact surfaces shall be less than 10 percent at the time of installation of the FRP system, as evaluated according to ACI 503R-93. Moisture restrictions may be waived for resins that have been formulated for wet applications. Work areas identified as confined space require an entry permit in accordance with OSHA regulations. The CONTRACTOR is responsible for the training of all personnel, air testing, safety equipment and complying with the requirements of OSHA. A detailed Safety Plan shall be submitted by the CONTRACTOR as part of the Technical Specification Section 01330, Submittals.

- b. Moisture Vapor Transmission: Application of bonded FRP systems shall not proceed if any moisture vapor transmission is present. Concrete dryness is necessary when using elevated temperature cure. Any bubble that develops from moisture vapor transmission can effectively be injected with the same adhesive material used for the FRP system following the procedure specified in Section 3.2.D.
- c. Applications in Inclement Weather: When inclement weather does not allow installation of the FRP system, as specified in Section 3.2.G.1.a, auxiliary measures may be employed to correct the conditions. An auxiliary heat source may be used in cold weather to raise the ambient and concrete surface temperatures to acceptable levels during product application and cure duration, as recommended by the FRP material supplier, but not higher than the glass transition temperature (T_g) of the primer or saturating resin. Pressurized air, with an effective oil separator, may be used to dry the surface dampness.
- d. Equipment: The CONTRACTOR shall provide all necessary equipment in sufficient quantities and clean operating conditions for continuous uninterrupted FRP system installation.
- e. Mixing of Resin Components: All resin components, including the main agent and hardener, shall be mixed at the proper temperature using the appropriate weight ratio and for a duration specified by the FRP material supplier until thorough mixing with uniform color and consistency is achieved. The resins shall not be diluted with any organic solvents such as a thinner. Electrically powered mixing blades can be used for mixing. Resin shall be mixed in quantities sufficiently small to ensure that it can be used within its pot life. Any mixed resin that exceeds its pot life or begins to generate heat or shows signs of increased viscosity shall not be used and shall be disposed of according to the SDS. The mixing of some resins may be accompanied by noxious fumes. Precautions must be taken regarding the resin's impact on the environment, including emission of volatile organic compounds. If flammable resins are used, explosion proof electrical motors or air driven motors shall be used for mixing and installation of the materials.
- f. Primer and Putty: When necessary, apply one (1) or two (2) coats of primer on the substrate surface to penetrate its open pores. Putty, if used, shall be applied as soon

as the primer becomes tack free or is not sticky to the fingers. Putty shall be applied within seven (7) days after primer application; otherwise, the primer-coated surface shall be roughened with sandpaper or a similar tool. The resulting surface shall be cleaned according to Section 3.2.F before applying the putty. Apply a minimum of 40-mil (1 mm) thick coat of putty in one (1) layer, and smooth over the surface to fill in any small voids, cracks, or uneven areas. Any swelling on the surface after applying the putty shall be corrected to meet surface profile as specified in Section 3.2.E. The surfaces of primer and putty shall be protected from dust, moisture, and any other contaminants before applying the FRP system.

- g. Saturated: Saturated is the term for the engineered resin used to impregnate FRP fabrics and turn them into laminae upon cure. The saturant used in a FRP system shall have sufficiently low viscosity to ensure full impregnation of the fiber sheets prior to curing. To maintain proper viscosity of the saturant, the ambient and substrate surface temperatures must be within the range specified in Section 3.2.G.1.a. Any mixed saturant that exceeds its pot life shall be disposed of according to Section 3.5.
- h. Applying Fiber Sheet and Saturant: The fabric shall be uniformly saturated with the saturant using the saturating machine or hand saturated, where site conditions are not favorable for using the machine. The fabric shall be cut to the length specified in the Contract Documents and shall be installed in place and gently pressed onto the wet putty. Any entrapped air between the fiber sheet and the surface shall be released or rolled along the sheet in the direction parallel to the fibers while allowing the resin to impregnate the fibers and achieve intimate contact with the substrate. Rolling perpendicular to the fiber direction is not permitted. In bi-directional fabrics, rolling shall be initially in the fill (transverse) direction end to end and then in the warp (longitudinal) direction.
- i. Multiple-Fiber Plies: Subsequent layers of fabric (if called for in the design), shall be properly saturated with saturant and applied on top of the first sheet of fabric with no interruption. The amount of resin overcoat for intermediate plies shall factor in overcoat for the applied ply and undercoat for the next ply. Follow the PWS and approved shop and Design Drawings for the fiber orientation and ply stacking sequence. Each ply and core fabric shall be applied before the onset of complete gelation of the previous layer. The number of plies that can be applied in a single day shall be based on the FRP material supplier's recommendation. When previous layers are cured, interlayer surface preparation, such as light sanding and filling with putty, may be required, as specified in Section 3.2.G.1.f.
- j. Overlapping: Lap joints shall be constructed over a minimum of one (1) CFRP or GFRP laminate. The lap joint layer shall not be the top laminate. The length of the lap splice shall be as specified by the Contract Documents but must be a minimum of 6-inches (152 mm) long. Staggering of lap splices on multiple plies and adjacent strips shall be required unless permitted by Contract Documents.
- k. Alignment of FRP Materials: The fiber plies shall be aligned on the host pipe according to the Contract Documents. Any deviation in the alignment more than 5 degrees (approximately 1 in./ft) is not acceptable and shall be grounds for rejection. Once installed, the fibers shall be free of kinks, folds, and waviness.

- l. Termination Points: The termination points of the FRP system shall be installed such that water is not allowed to seep in between the surface and the FRP system. The FRP system shall be sealed properly at the termination points by applying epoxy sealant. In locations where flow velocities are high or in pressurized conduits, elastomeric internal joint seals with compressed steel bands shall be used at the termination points, with the appropriate approved details included in the Design Drawings.
- m. Curing: The FRP system shall be allowed to cure for the minimum amount of time specified per the FRP material supplier's requirements. Field modification of resin chemistry for rapid curing shall not be allowed. An elevated cure temperature may be used if rapid curing is necessary. The cure of installed plies shall be monitored before placing subsequent plies. In case of any curing irregularity, installation of subsequent plies shall be halted. Protect the FRP system while curing.
- n. Protective Coating and Finishing: It is permissible to use a protective coating for abrasion resistance and improved flow conditions. The protective coating shall be either thickened epoxy, with or without a pigment added to facilitate inspection, or another resin as agreed between the OWNER or ENGINEER and CONTRACTOR. The protective coating shall be compatible with exposure conditions including corrosive gases, moisture, abrasive chemicals in sewage, salt water, and physical abrasion due to debris and silt in sewers. Applicability of the topcoat to the FRP system in abrasive environments shall be verified by testing (ASTM D4060).
- o. Protective Coating Field Application: Surface preparation shall be as recommended by the FRP material supplier. Solvent wipes shall not be used to clean the FRP surface unless approved by the FRP material supplier. If abrasive cleaning is necessary, air pressure shall be limited to avoid any damage to fibers. Ambient and surface temperatures shall be within the range specified in Section 3.2.G.1.a, prior to applying the protective coating.

3.3 INSPECTION & QUALITY ASSURANCE

- A. The CONTRACTOR shall coordinate with and accommodate the needs of the OWNER or ENGINEER Quality Assurance inspector. The CONTRACTOR shall provide a specific quality assurance (QA) plan for the tests identified. The CONTRACTOR shall provide its own onsite inspector for Quality Control (QC).
- B. The project specific QA/QC plan shall be included in the PWS submittal that includes:
 1. Inspection of Materials: The FRP material supplier's certifications for all delivered and stored FRP components shall be inspected for conformity to the Contract Documents before starting the project. Materials testing shall be conducted on samples as detailed in the PWS. Any material that does not meet the requirements of the Contract Documents or PWS will be rejected. Additional inspection measures may be taken during the installation process at the discretion of the OWNER or ENGINEER.
 2. Daily Inspection: Daily inspection shall include date and time of repair; relative humidity; general weather conditions; surface dryness per ACI 503.4-92; surface

preparation and surface profile using ICRI surface profile chips; qualitative description of surface cleanliness; type of auxiliary heat source, if any; widths of cracks not injected with epoxy; fiber or procured laminate batch numbers and their locations in the structure; batch numbers, mixture ratios, mixing times, and qualitative descriptions of the appearance of all mixed resins, primers, putties, saturants, adhesives, and coatings; observations of the progress of the cure of resins; conformance with installation procedures; adhesion test results of bond strength, failure mode, and location; FRP properties from tests of field sample panels or witness panels, if required; location and size of any delamination's or air voids; lot number, manufacturer and expiration date of applied materials, and the general progress of work.

3. Inspection for Fiber Orientation: Fiber or ply orientation, fiber kinks, and waviness shall be examined by visual inspection for conformity to the Contract Documents. Tolerances will follow Sections 3.2 and 3.3. Any nonconforming FRP repair area shall be removed and repaired as per Section 3.4 – Repair of Defective Work.
4. Inspection for Debonding: After at least 24 hours from initial installation of the resin, a visual inspection of the surface shall be performed for any swelling, bubbles, voids, or delamination. If an air pocket is suspected, an acoustic tap test shall be carried out with a hard object to identify delaminated areas by sound, with at least one strike per one (1) foot². Delamination defects smaller than 0.5-inch in diameter shall require no corrective action, unless as specified in Section 3.4.A. Delamination defects larger than 0.5-inch but smaller than 2-inch in diameter will be repaired as per Section 3.4.B. Delamination defects larger than 2-inch but smaller than 6- inch in diameter, and with a frequency of less than five (5) per 10 foot² of surface area, shall be repaired as per Section 3.4.C. Larger delamination defects shall be repaired as per Section 3.4.D.
5. Inspection for Cure of Resin: If specified in the Contract Documents, the relative cure of resin in FRP systems shall be examined by visual inspection or by third party laboratory testing of resin-cup samples using ASTM D3418. Follow recommendations of the resin FRP material supplier for acceptance criteria. If the cure of resin is found unacceptable, the entire area shall be marked and repaired as per Section 3.4.
6. Tests: Specimens from the installed FRP system or samples representative thereof, shall be tested at the request of the OWNER or ENGINEER, for tensile, compressive, and adhesive strengths. A minimum of 2 witness panels per day shall be obtained on the days the FRP system is installed at the project site.
7. Adhesion test: Refer to ASTM D7234 for adhesion to concrete. To validate the adequacy of the surface preparation and the adhesion strength of the FRP system, the CONTRACTOR shall perform random adhesion tests on the prepared concrete substrate of pipe segments adjacent to repair pipes as directed by the ENGINEER and witnessed by the ENGINEER. The ENGINEER should designate the areas for trial adhesion tests prior to the surface preparation activities. These areas shall be cleaned, prepared, and covered with two-ply FRP system test patches with minimum dimensions of 1-foot x 1-foot for adhesion test to concrete substrate. The patch shall consist of two orthogonal plies of FRP laminae. One (1) adhesion test shall be performed on each test patch. The remaining adhesion test patches shall be finish coated and remain in place

- for future testing purposes as needed. The CONTRACTOR shall log the location of the adhesion test and report the test results to the OWNER or ENGINEER.
8. Tensile strength test (ASTMD3039): Depending on the composition of the FRP system used for a particular application, test panels shall be field fabricated using two (2) FRP system layers of glass fiber fabric, any core (3D) fabric, resins and saturation equipment used in the application. Tensile test panels shall be approximately 12-inch by 12-inch. The test panel shall be prepared on a smooth flat surface overlaid with plastic (polyethylene or vinyl) sheeting. Saturating resin shall be used to prime the surface, followed by the saturated system, and finally topped with more saturating resin. A cover of plastic sheeting shall be placed over the panel and the panel squeegeed to remove any bubbles and other surface irregularities to ensure a smooth flat surface. The panel shall be labeled with time, date, and sample panel number, fabric lot numbers, and resin batch numbers, and stored in an environment representative of the surface being repaired condition to cure until collected for lab testing.
 - a. The test lab shall perform a minimum of five (5) tensile tests with the fibers oriented in the strong direction for each tensile test panel in accordance with ASTM D3039, and report certified tests results for tensile strength, tensile modulus, related specimen dimensions, and percent elongation. For projects where multiple test panels with the same fabric batch are prepared, it is permissible to perform three (3) tension tests per panel instead of five (5) tests per panel, subject to review and approval by the ENGINEER.
 - b. The FRP system will be unacceptable if the average tensile strength per unit width is below that specified in the Contract Documents.
 9. Holiday "Spark" Testing: The CONTRACTOR shall furnish services of a trained operator in holiday detection devices until final acceptance of the FRP system. Devices shall be tested and operated in the presence of the OWNER or ENGINEER. The CONTRACTOR shall perform holiday testing on all FRP surfaces in the presence of the OWNER or ENGINEER. The CONTRACTOR shall be responsible to repair defective areas and retest the FRP system as necessary at no additional cost to the OWNER.

3.4 REPAIR OF DEFECTS

- A. This section specifies the conditions and types of defects that require repair and the acceptable methods of repair. Defects are of different types and may be generally classified as aesthetic, short-term critical, or long-term critical. Repair procedure depends on the type, size, and extent of defects. Repair procedures indicated below are exemplary, and the actual procedure specific to each project shall be submitted as a part of the PWS.
- B. Repair of Protective Coating:
 1. Defects in protective coating can be of three (3) types: small hairline cracks, blistering, and peeling. In all cases, the moisture content of the substrate should be below 0.05% before applying a new coating. Prior to any repair of protective coating, the FRP system shall be examined visually or otherwise to ensure that no defect exists within or on the surface.

2. Defects in FRP, if found, shall be repaired as described below. If protective coating appears to show small areas with cracks, the local surface shall be lightly sanded. Then a new coating with the appropriate primer shall be applied according to the FRP material supplier's recommendations. At the minimum, the coating shall be applied over an area extending 1-inch on either side of the defect. If the protective coating shows signs of blistering, the entire area of blisters as well as the surrounding area to a distance of at least 12-inches shall be carefully scraped clean. In no case should a blistered surface be recoated without complete removal of the existing coating. The area shall be wiped clean and dried thoroughly. Once dry, the area can be recoated after application of the primer coat if required by the FRP material supplier. If the surface shows signs of excessive peeling, the entire coating shall be scraped off and the surface lightly sanded, wiped clean, and thoroughly dried before applying a new coat according to the FRP material supplier's recommendations.
- C. Epoxy Injection of Small Defects:
1. Small entrapped voids or surface discontinuities no larger than 0.5-inch in diameter shall not be considered defects and require no corrective action unless they occur next to edges or when there are more than five (5) such defects in an area of ten (10) foot². Small defects of size between 0.5-inch and 2-inches in diameter shall be repaired using low-pressure epoxy injection if the defect is local and does not extend through the complete thickness of the FRP system.
- D. Patching of Minor Damage:
1. Minor defects are those with diameters between 2-inches and 6-inches and a frequency of less than five (5) per unit surface area of 10 foot². The area surrounding the defects to an extent of at least 1-inch on all sides shall be carefully removed. The area shall be wiped clean and thoroughly dried. The area shall then be patched by adding an FRP patch extending at least 1-inch on all sides of the removed area.
- E. Replacement of Large Defects:
1. Defects larger than 6-inches in diameter shall be carefully marked and scarfed out extending to a minimum of 1-inch on all sides. Scarfing shall be progressing through the layers of the FRP system until past the defective area. The substrate shall be appropriately prepared, and primer reapplied after ensuring that the surface is clean and dry. Application of an FRP patch system shall extend a minimum of 6-inches on all sides of the scarfed area. Once cured, the protective coating shall be applied over the entire area.

3.5 STORAGE, HANDLING, AND DISPOSAL

- A. Storage Requirements: All components of the FRP system must be delivered and stored in the original factory-sealed, unopened packaging or in containers with proper labels identifying the FRP material supplier, brand name, system identification number, manufacture date, and expiration date. Catalysts and initiators should be stored separately. All components must be protected from dust, moisture, chemicals, direct sunlight, physical

damage, fire, and temperatures outside the range specified in the system data sheets. Any component that has been stored in a condition different from that stated above must be disposed of, as specified in Section 3.5.G.

- B. Shelf Life: All components of the FRP system, especially resins and adhesives, that have been stored longer than the shelf life specified on the system data sheet shall not be used and must be disposed of, as specified in Section 3.5.G.
- C. Handling: All components of the FRP system, especially fiber sheets, must be handled with care according to the FRP material supplier recommendations to protect them from damage and to avoid misalignment or breakage of the fibers by pulling, separating, or wrinkling them or by folding the sheets. After cutting, sheets shall be either stacked dry with separators or at a radius no tighter than 12-inches or as recommended by the FRP material supplier.
- D. Safety Hazards: All components of the FRP system, especially resins and adhesives, must be handled with care to avoid safety hazards, including but not limited to skin irritation and sensitization, and breathing vapors and dust. Mixing resins shall be monitored to avoid fuming and inflammable vapors, fire hazards, or violent boiling. The CONTRACTOR is responsible for ensuring that all components of the FRP system at all stages of work conform to the local, state, and federal environmental and worker's safety laws and regulations. The CONTRACTOR is advised that a forced ventilator system may be required inside enclosed sections and that provision for ventilation, if any, shall be included in the cost of the work. Refer to the Safety Plan as specified in Section 3.2.G for details.
- E. Safety Data Sheets: The SDS for all components of the FRP system shall be accessible to all at the project site. Specific handling hazards and disposal instructions shall be specified in the SDS section entitled Personnel and Workplace Protection. The CONTRACTOR is responsible for providing the proper means of protection for safety of the personnel and the workplace.
 - 1. The CONTRACTOR shall inform the personnel of the dangers of inhaling fumes of primer, putty, or resin and shall take all necessary precautions against injury to personnel. The resin mixing area shall be well vented to the outside. Refer to the Safety Plan as specified in Section 3.2.G for details.
- F. Clean-Up: The CONTRACTOR is responsible for the cleanup of the equipment and the project site from hazardous and aesthetically undesirable FRP components using appropriate solvents, as stated in the safety data sheet.
- G. Disposal: Any component of the FRP system that has exceeded its shelf life or pot life or has not been properly stored, and any unused or excess material that is deemed waste shall be disposed of in a manner amiable to the protection of the environment and consistent with the SDS.

3.6 PROJECT CLOSE-OUT AND MAINTENANCE

- A. Documentation of the work: The OWNER requires documentation of the work to be prepared and submitted to the OWNER or ENGINEER by the CONTRACTOR on completion of the work according to Technical Specification Section 01330, Submittals. The documentation shall include the following:
 - 1. Information on the products and installation method used.
 - 2. Pre- and post-inspection results.
 - 3. Test results.
 - 4. Any changes or deviations from the Contract Documents and possible corrective work.
- B. Verification/Basis for Rejection: Materials not complying with the requirements of this Technical Specification shall be rejected at the discretion of the OWNER or ENGINEER. Repairs, replacements, and retesting shall be accomplished in accordance with the Contract Documents at no additional cost to the OWNER.
- C. Affidavit of Compliance: The OWNER requires an affidavit from the FRP material supplier or CONTRACTOR, as applicable, that the material provided, or the installation performed complies with the applicable requirements of the Technical Specifications provided herein and delivered to the OWNER or ENGINEER according to Technical Specification Section 01330, Submittals.

3.7 PROTECTION OF SITE:

- A. Except as otherwise provided herein, the CONTRACTOR shall protect from damage all fences, structures, sidewalks, utilities, trees, shrubbery, lawns, etc., during the progress of this work. All debris and unused materials shall be removed immediately upon completion of the work. The site shall be restored as nearly as possible to its original condition, including the replacement, at the CONTRACTOR's sole expense, of any facility or landscaping which has been destroyed or damaged beyond restoration.

3.8 UTILITIES:

- A. The CONTRACTOR shall arrange for and provide any required utilities at the CONTRACTOR's sole cost and expense. This includes but is not limited to water, power for operating any equipment along with personnel sanitation facilities.

3.9 CLEANING AND PREPARATION

- A. The CONTRACTOR shall be responsible for all cleaning and surface preparation. The CONTRACTOR shall notify the ENGINEER of any irregularity, which may interfere with the proper installation.

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PROJECT NAME: Loop 101 and 202 Freeway 66-inch Redundant Sewer Project
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3.10 ACCEPTANCE

- A. As a condition precedent to Final Acceptance of the work, CONTRACTOR shall certify the proper installation and testing of material provided under this section. Certification shall warrant that materials and installation are free of defects and/or workmanship and suitable for trouble-free operation under the conditions set forth in these Technical Specifications.

++ END OF SECTION ++

SECTION 03100

CONCRETE FORMWORK

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Scope:
1. Provide all labor, materials, equipment and incidentals as shown, specified and required to furnish and install concrete formwork. The Work also includes:
 - a. Providing openings in formwork to accommodate the Work under this and other Sections and building into the formwork all items such as sleeves, anchor bolts, inserts and all other items to be embedded in concrete for which placement is not specifically provided under other Sections.
- B. Coordination:
1. Review installation procedures under other Sections and coordinate the installation of items that must be installed with the formwork.
 2. Coordinate formwork specifications herein with the requirements for finished surfaces specified in Section 03300, Cast-In-Place Concrete, and Section 03200, Concrete Reinforcement.

1.2 QUALITY ASSURANCE

- A. Referenced Standards:
1. American Concrete Institute (ACI):
 - a. CT-13, Concrete Terminology.
 - b. 117, Specification for Tolerances for Concrete Construction and Materials.
 - c. 347R, Guide to Formwork for Concrete.
- B. Qualifications:
1. Formwork, shoring and reshoring to be designed by a licensed professional engineer currently registered or having a minimum of three years of experience in this type of design work.
 - a. Above qualifications apply to the following:
 - 1) Slabs and beams not cast on the ground
 - 2) Wall and column placements over 15-feet in height.

C. Miscellaneous:

1. Design and engineering of formwork, shoring and reshoring as well as its construction is the responsibility of the Contractor.
2. Design requirements:
 - a. Design formwork for loads, lateral pressures and allowable stresses outlined in ACI 347R and for design considerations, wind loads, allowable stresses and other applicable requirements of the controlling local building code.
 - 1) Where conflicts occur between the above two standards, the more stringent requirements shall govern.
 - b. Design formwork to limit maximum deflection of form facing materials reflected in concrete surfaces exposed to view to 1/240 of span between structural members.
3. For slabs and beams not cast on the ground, develop a procedure and schedule for removal of shores and installation of reshores and for calculating the loads transferred to the structure during this process in accordance with ACI 347R.
 - a. Perform structural calculations as required to prove that all portions of the structure in combination with remaining forming and shoring system has sufficient strength to safely support its own weight plus the loads placed thereon. Calculations shall be performed by a licensed professional engineer.
 - b. When developing procedure, schedule and structural calculations, consider the following at each stage of construction:
 - 1) The structural system that exists.
 - 2) Effects of all loads during construction.
 - 3) Strength of concrete.
 - 4) Modulus of Elasticity (Young's Modulus) of the concrete at the time of shoring and form removal.
 - 5) The influence of deformations of the structure and shoring system on the distribution of dead loads and construction loads.
 - 6) The strength and spacing of shores or shoring systems used, as well as the method of shoring, bracing, shore removal, and reshoring including the minimum time intervals between the various operations.
 - 7) Any other loading or condition that affects the safety or serviceability of the structure during construction.

1.3 DEFINITIONS

- A. Words and terms used in these Specifications are defined in ACI CT-13.

1.4 SUBMITTALS

- A. Shop Drawings:
 - 1. Comply with the requirements of Section 01332, Shop Drawing Procedures.
 - 2. Product technical data including:
 - a. Manufacturer and type of proposed form ties.
 - 3. Submit for information purposes the following:
 - a. Copies of manufacturer's data and installation instructions for proprietary materials, including form coatings, manufactured form systems, ties and accessories.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Forms for Surfaces Exposed to View:
 - 1. Wood forms:
 - a. 5/8 or 3/4-inch 5-ply structural plywood of concrete form grade.
 - b. Built-in-place or prefabricated type panel.
 - 2. Metal forms:
 - a. Metal forms may be used except for aluminum in contact with concrete.
 - b. Forms to be tight to prevent leakage, free of rust and straight without dents to provide members of uniform thickness.
- B. Forms for Surfaces Not Exposed to View:
 - 1. Wood or metal sufficiently tight to prevent leakage.
 - 2. Do not use aluminum forms.

2.2 ACCESSORIES

- A. Form Ties:
 - 1. Commercially fabricated for use in form construction.
 - a. Field fabricated ties are unacceptable.
 - 2. Constructed so that ends or end fasteners can be removed without causing spalling at surfaces of the concrete.
 - 3. Embedded portion of ties to be not less than 1-1/2-inch from face of concrete after ends have been removed.
 - 4. Cone size:
 - a. 3/4 IN minimum diameter cones on both ends.
 - b. Depth of cone not to exceed the concrete reinforcing cover.
 - 5. Provide ties with built-in waterstops in all walls that will be in contact with process liquid during plant operation and below grade soil.

6. Through-wall ties that are designed to be entirely removed may be used for walls that will be in contact with process liquid during plant operation provided:
 - a. Tie hole is plugged with:
 - 1) X-Plug, by Greenstreak, Inc.
 - 2) Or Equal.
 - b. Tie hole plugs are to be installed in strict accordance with the manufacturer's printed installation instructions.
 - c. Contractor provides and follows quality control plan for verifying proper installation.

PART 3 - EXECUTION

3.1 PREPARATION

- A. Form Surface Treatment:
 1. Before placing of reinforcing steel or concrete, cover surfaces of forms with an approved release material that will effectively prevent absorption of moisture and prevent bond with concrete, will not stain concrete or prevent bonding of future finishes.
 - a. A field applied form release agent or sealer of approved type or a factory applied nonabsorptive liner may be used.
 2. Do not allow excess form release material to stand in puddles in forms nor in contact with hardened concrete against which fresh concrete is to be placed.
- B. Provide temporary openings at base of column and wall forms and at other points where necessary to facilitate cleaning and observation immediately before concrete is placed, and to limit height of free fall of concrete to prevent aggregate segregation.
 1. Temporary openings to limit height of free fall of concrete shall be spaced no more than 8-feet apart.
- C. Clean surfaces of forms, reinforcing steel and other embedded materials of any accumulated mortar or grout from previous concreting and of all other foreign material before concrete is placed.

3.2 ERECTION

- A. Install products in accordance with manufacturer's instructions.
- B. Tolerances:
 1. Conform to ACI 117.
 2. Variation from plumb:
 - a. In lines and surfaces of columns, piers, walls, and in risers.

- 1) Maximum in any 10-feet of height: $\frac{1}{4}$ -inch.
 - 2) Maximum for entire height: $\frac{1}{2}$ -inch.
 - b. For exposed corner columns, control-joint grooves, and other exposed to view lines:
 - 1) Maximum in any 20-feet length: $\frac{1}{4}$ -inch.
 - 2) Maximum for entire length: $\frac{1}{2}$ -inch.
3. Variation from level or from grades specified:
 - a. In slab soffits, ceilings, beam soffits and in arises, measured before removal of supporting shores.
 - 1) Maximum in any 10-feet of length: $\frac{1}{4}$ -inch.
 - 2) Maximum in any bay or in any 20-feet length: $\frac{3}{8}$ -inch.
 - 3) Maximum for entire length: $\frac{3}{4}$ -inch.
 - b. In exposed lintels, sills, parapets, horizontal grooves, and other exposed to view lines:
 - 1) Maximum in any bay or in 20-feet length: $\frac{1}{4}$ -inch.
 - 2) Maximum for entire length: $\frac{1}{2}$ -inch.
4. Variation of linear structure lines from established position in plan and related position of columns, walls, and partitions:
 - a. Maximum in any bay: $\frac{1}{2}$ -inch.
 - b. Maximum in any 20-feet of length: $\frac{1}{2}$ -inch.
 - c. Maximum for entire length: 1-inch.
5. Variation in sizes and location of sleeves, floor openings, and wall openings:
 - a. Maximum of $+\frac{1}{2}$ -inch.
6. Variation in horizontal plan location of beam, column and wall centerlines from required location: Maximum of $+\frac{1}{2}$ -inch.
7. Variation in cross sectional dimensions of columns and beams and in thickness of slabs and walls: Maximum of $-\frac{1}{4}$ -inch, $+\frac{1}{2}$ -inch.
8. Footings and foundations:
 - a. Variations in concrete dimensions in plan: $-\frac{1}{2}$ -inch, $+2$ -inch.
 - b. Misplacement or eccentricity:
 - 1) 2-percent of footing width in direction of misplacement but not more than 2-inch.
 - c. Thickness:
 - 1) Decrease in specified thickness: 5-percent.
 - 2) Increase in specified thickness: No limit except that which may interfere with other construction.
9. Variation in steps:
 - a. In a flight of stairs:
 - 1) Rise: $+\frac{1}{16}$ -inch.
 - 2) Tread: $+\frac{1}{4}$ -inch.
 - b. In consecutive steps:
 - 1) Rise: $+\frac{1}{16}$ -inch.
 - 2) Tread: $+\frac{1}{8}$ -inch.

10. Establish and maintain in an undisturbed condition and until final completion and acceptance of Project, sufficient control points and bench marks to be used for reference purposes to check tolerances.
 11. Regardless of tolerances listed allow no portion of structure to extend beyond legal boundary of Project.
 12. To maintain specified tolerances, camber formwork to compensate for anticipated deflections in formwork prior to hardening of concrete.
- C. Make forms sufficiently tight to prevent loss of mortar from concrete.
- D. Place 3/4-inch chamfer strips in exposed to view corners of forms to produce 3/4-inch wide beveled edges.
- E. At construction joints, overlap contact surface of form sheathing for flush surfaces exposed to view over hardened concrete in previous placement by at least 1-inch.
1. Hold forms against hardened concrete to prevent offsets or loss of mortar at construction joint and to maintain a true surface.
 2. Where possible, locate juncture of built-in-place wood or metal forms at architectural lines, control joints or at construction joints.
- F. Where circular walls are to be formed and forms made up of straight sections are proposed for use, provide straight lengths not exceeding 2-feet wide.
1. Brace and tie formwork to maintain correct position and shape of members.
- G. Construct wood forms for wall openings to facilitate loosening, if necessary, to counteract swelling.
- H. Anchor formwork to shores or other supporting surfaces or members so that movement of any part of formwork system is prevented during concrete placement.
- I. Provide runways for moving equipment with struts or legs, supported directly on formwork or structural member without resting on reinforcing steel.
- J. Provide positive means of adjustment (wedges or jacks) of shores and struts and take up all settlement during concrete placing operation.
1. Securely brace forms against lateral deflection.
 2. Fasten wedges used for final adjustment of forms prior to concrete placement in position after final check.

3.3 REMOVAL OF FORMS

- A. No construction loads shall be supported on, nor any shoring removed from, any part of the structure under construction except when that portion of the structure

- in combination with remaining forming and shoring system has sufficient strength to safely support its weight and loads placed thereon.
- B. When required for concrete curing in hot weather, required for repair of surface defects or when finishing is required at an early age, remove forms as soon as concrete has hardened sufficiently to resist damage from removal operations or lack of support.
 - C. Remove top forms on sloping surfaces of concrete as soon as concrete has attained sufficient stiffness to prevent sagging.
 - 1. Perform any needed repairs or treatment required on such sloping surfaces at once, followed by curing specified in Specification Section 03300, Cast-In-Place Concrete.
 - D. Loosen wood forms for wall openings as soon as this can be accomplished without damage to concrete.
 - E. Formwork for columns, walls, sides of beams, and other parts not supporting weight of concrete may be removed as soon as concrete has hardened sufficiently to resist damage from removal.
 - a. For walls of water containing structures, leave forms in place for a minimum of 48-hours.
 - F. Where no reshoring is planned, leave forms and shoring used to support weight of concrete in place until concrete has attained its specified 28-day compressive strength and the concrete Modulus of Elasticity has reached a value of $(57,000 \text{ times the square root of } f'c)$ unless otherwise approved in writing by the ENGINEER.
 - 1. Where a reshoring procedure is planned, supporting formwork may be removed when concrete has reached the concrete strength required by the formwork designer's structural calculations.
 - G. When shores and other vertical supports are so arranged that non-load-carrying form facing material may be removed without loosening or disturbing shores and supports, facing material may be removed when concrete has sufficiently hardened to resist damage from removal.

3.4 RESHORING

- A. No construction loads shall be supported on, nor any shoring removed from, any part of the structure under construction except when that portion of the structure in combination with remaining forming and shoring system has sufficient strength to safely support its weight and loads placed thereon.

- B. While reshoring is underway, no superimposed dead or live loads shall be permitted on the new construction.
- C. During reshoring do not subject concrete in structural members to combined dead and construction loads in excess of loads that structural members can adequately support.
- D. Place reshores as soon as practicable after stripping operations are complete but in no case later than end of working day on which stripping occurs.
- E. Tighten reshores to carry their required loads without overstressing.
- F. Shoring, reshoring and supporting formwork may be removed when concrete has reached the concrete strength required by the formwork designer's structural calculations.
- G. For floors supporting shores under newly placed concrete leave original supporting shores in place or reshore.
 - 1. Reshoring system shall have a capacity sufficient to resist anticipated loads.
 - 2. Locate reshores directly under a shore position above.
- H. In multi-story buildings, extend reshoring over a sufficient number of stories to distribute weight of newly placed concrete, forms, and construction live loads in such a manner that design superimposed loads of floors supporting shores are not exceeded.

3.5 FIELD QUALITY CONTROL

- A. Special Inspection:
 - 1. See Specification Section 01416, Special Inspections.
 - 2. Special Inspection is required for:
 - a. Shape, location, and dimensions.
 - 1) Inspect in accordance with dimensions and details on Drawings
 - 2) Frequency: Inspect periodically, prior to each concrete pour.

++ END OF SECTION ++

SECTION 03200

CONCRETE REINFORCEMENT

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Scope:
1. Provide all labor, materials, equipment and incidentals as shown, specified and required to furnish and install concrete reinforcement.
 2. The extent of concrete reinforcement is shown.
 3. The Work includes fabrication and placement of reinforcement including bars, ties and supports, and welded wire fabric for concrete, encasements and fireproofing.

1.2 QUALITY ASSURANCE

- A. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown or specified:
1. ASTM A82, Specification for Steel Wire, Plain, for Concrete Reinforcement.
 2. ASTM A184, Specification for Fabricated Deformed Steel Bar Mats for Concrete Reinforcement.
 3. ASTM A185, Specification for Steel Welded Wire Fabric, Plain, for Concrete Reinforcement.
 4. ASTM A496, Specification for Steel Wire, Deformed, for Concrete Reinforcement.
 5. ASTM A497, Specification for Steel Welded Wire Fabric, Deformed, for Concrete Reinforcement.
 6. ASTM A615, Specification for Deformed and Plain Billet-Steel Bars for Concrete Reinforcement.
 7. ASTM A706, Specification for Low-Alloy Steel Deformed Bars for Concrete Reinforcement.
 8. ASTM A775, Specification for Epoxy-Coated Reinforcing Steel Bars.
 9. ASTM A1022, Standard Specification for Deformed and Plain Stainless Steel Wire and Welded Wire for Concrete Reinforcement.
 10. ASTM A1064, Standard Specification for Steel Wire and Welded Wire Reinforcement, Plain and Deformed, for Concrete.
 11. ACI 315, Manual of Standard Practice for Detailing Reinforced Concrete Structures.
 12. ACI 318, Building Code Requirements for Structural Concrete.
 13. ACI 350, Code Requirements for Environmental Engineering Concrete Structures.

14. ACI SP66, Detailing Manual.
15. ANSI/AWS D1.4, Structural Welding Code - Reinforcing Steel.
16. CRSI 1MSP, Concrete Reinforcing Steel Institute (CRSI) Manual of Standard Practice.

- B. Allowable Placing Tolerances: Comply with ACI 350, Chapter 7 - Details of Reinforcement except as specified below and as shown per the Contract Drawings:
1. Concrete surfaces which are in contact with liquids: 2-inches minimum coverage.

1.3 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
1. Comply with the requirements of Section 01332, Shop Drawing Procedures.
 2. Manufacturer's specifications and installation instructions for all materials and reinforcement accessories.
 3. Drawings for fabrication, bending, and placement of concrete reinforcement. Comply with ACI 315, Parts A and B. For walls, show elevations to a minimum scale of 1/4-inch to one foot. For slabs, show top and bottom reinforcing on separate plan views. Show bar schedules, stirrup spacing, diagrams of bent bars, arrangements and assemblies, as required for the fabrication and placement of concrete reinforcement unless otherwise noted. Splices shall be kept to a minimum. Splices in regions of maximum tension stresses shall be avoided whenever possible.
 4. Shop Drawings shall be in sufficient detail to permit installation of reinforcing without reference to Contract Drawings.
 - a. Shop Drawings shall not be prepared by reproducing the plans and details indicated on the Contract Drawings but shall consist of completely redrawn plans and details as necessary to indicate complete fabrication and installation of all reinforcing steel.
 - b. Where multiple types of supports for reinforcing steel (such as chairs, runners, bolsters, and other types of supports) will be used in the Work, clearly indicate on the Shop Drawings the support types and materials of supports.
 5. Drawings detailing the location of all construction and expansion joints as required under Section 03251, Concrete Joints, shall be submitted and approved before Shop Drawings for reinforcing steel are submitted.
 6. Description of reinforcing weld locations and weld procedures.
 7. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR,

each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

B. Certificates:

1. Submit one copy of steel producer's certificates of mill analysis, tensile and bend tests for reinforcing steel.
2. Submit certification of welders and weld procedures for splices in accordance with ANSI/AWS D1.4 requirements.

1.4 DELIVERY, HANDLING AND STORAGE

- A. Deliver concrete reinforcement materials to the site bundled, tagged and marked. Use metal tags indicating bar size, lengths, and other information corresponding to markings shown on placement diagrams.
- B. Store concrete reinforcement material at the site to prevent damage and accumulation of dirt or excessive rust. Store on heavy wood blocking so that no part of it will come in contact with the ground.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Reinforcing Bars: ASTM A 615, and as follows:
 1. Provide Grade 60, deformed, for all bars, unless indicated otherwise.
- B. Reinforcing Bars to be Welded: ASTM A706, Grade 60, deformed.
- C. Mechanical Couplers: Reinforcement bars may be spliced with a mechanical connection. This connection shall be a full mechanical connection which shall develop in tension or compression, as required, at least 125 percent of specified yield strength (fy) of the bar being spliced in accordance with ACI 318.
 1. Acceptable Manufacturers:
 - a. Lenton Rebar Splicing by Erico, Inc.

- b. Richmond dowel bar splicer system by Richmond Screw and Anchor Co., Inc.
 - c. Bar-Grip Systems by Barsplice Products, Inc.
- D. Steel Wire: ASTM A 82.
- E. Welded Wire Reinforcement: ASTM A1064 or ASTM A1022 where noted on Drawings.
 - 1. Furnish in flat sheets, not rolls.
- F. Supports for Reinforcement: Bolsters, chairs, spacers and other devices for spacing, supporting and fastening reinforcement in place.
 - 1. Use wire bar type supports complying with CRSI "Manual of Standard Practice" recommendations, except as specified below. Do not use wood, brick, or other unacceptable materials.
 - 2. For slabs on grade, use precast concrete blocks, 4-inch square in plan, with embedded tie wire as specified by CRSI, "Manual of Standard Practice". The precast concrete blocks shall have the same or higher compressive strength as specified for the concrete in which they are located.
 - 3. For all concrete surfaces, where legs of supports are in contact with forms, provide supports complying with CRSI "Manual of Standard Practice" as follows:
 - a. At formed surfaces in contact with soil, weather, or liquid or located above liquid, supports shall be CRSI Class 1 for maximum protection. The plastic coating on the legs shall extend at least 1/2-inch upward from the form surface.
 - b. At interior dry surfaces (not located above liquid), supports shall be either Class 1 or Class 2 for moderate protection.
 - c. At formed surfaces with an architectural finish, use stainless steel protected legs (Type B).
 - 4. Over waterproof membranes, use precast concrete chairs.
- G. Drilled Dowels
 - 1. Drilled dowels shall consist of reinforcing bars anchored with an adhesive system into hardened concrete or grout filled masonry. The adhesive system shall use a two-component adhesive mix and shall be injected with a static mixing nozzle following manufacturer's written instructions. The embedment depth of the reinforcing bar shall provide minimum allowable bond strength no less than the allowable tensile capacity of the reinforcing bar, unless shown otherwise on the Drawings.
 - 2. Drilled dowel anchoring systems shall have ultimate capacity no less than 125 percent of the yield strength of the reinforcing steel at an embedment of 12 bar diameters.
 - 3. Product and Manufacturer: Provide one of the following:

- a. HIT-RE 500-V3 Epoxy Adhesive Anchor System, by Hilti Inc.
- b. SET-3G Structural Epoxy-Tie Anchor System by Simpson Strong-Tie.
- c. Or equal.

2.2 FABRICATION

- A. General: Fabricate reinforcing bars to conform to required shapes and dimensions, with fabrication tolerances complying with CRSI, "Manual of Standard Practice". In case of fabricating errors, do not re-bend or straighten reinforcement in a manner that will injure or weaken the material.
- B. Unacceptable Materials: Reinforcement with any of the following defects will not be permitted in the Work:
 1. Bar lengths, bends, and other dimensions exceeding specified fabrication tolerances.
 2. Bends or kinks not shown on approved Shop Drawings.
 3. Bars with reduced cross-section due to excessive rusting or other cause.
- C. Ship reinforcing to jobsite with attached plastic or metal tags.
 1. Place on each tag the mark number of the reinforcing corresponding to the mark number indicated on the Shop Drawing.
 2. Mark numbers on tags to be so placed that the numbers cannot be removed.

PART 3 - EXECUTION

3.1 INSPECTION

- A. Examine the substrate and the conditions under which concrete reinforcement is to be placed, and notify ENGINEER, in writing, of unsatisfactory conditions. Do not proceed with the Work until unsatisfactory conditions have been corrected in a manner acceptable to ENGINEER.

3.2 INSTALLATION

- A. Comply with the applicable recommendations of specified codes and standards, and CRSI, "Manual of Standard Practice", for details and methods of reinforcement placement and supports.
- B. Clean reinforcement to remove loose rust and mill scale, earth, ice, and other materials which reduce or destroy bond with concrete.
- C. Position, support, and secure reinforcement against displacement during formwork construction or concrete placement operations. Locate and support reinforcing by metal chairs, runners, bolsters, spacers and hangers, as required.

1. Place reinforcement to obtain the minimum concrete coverages as shown and as specified in ACI 350. Arrange, space, and securely tie bars and bar supports together with 16 gage wire to hold reinforcement accurately in position during concrete placement operations. Set wire ties so that twisted ends are directed away from exposed concrete surfaces.
 2. Prior to placement of concrete, demonstrate to ENGINEER that the specified cover of reinforcement has been attained, by using a surveying level or string line.
 3. Reinforcing steel shall not be secured to forms with wire, nails or other ferrous metal. Metal supports subject to corrosion shall not touch formed or exposed concrete surfaces.
- D. Install welded wire fabric in as long lengths as practical. Lap adjoining pieces at least one full mesh and lace splices with 16-gage wire. Do not make end laps midway between supporting beams, or directly over beams of continuous structures. Offset end laps in adjacent widths to prevent continuous laps.
- E. Provide sufficient numbers of supports of strength required to carry reinforcement. Do not place reinforcing bars more than 2-inches beyond the last leg of any continuous bar support. Do not use supports as bases for runways for concrete conveying equipment and similar construction loads.
- F. Lap Splices:
1. Provide standard reinforcement splices by lapping ends, placing bars in contact, and tying tightly with wire. Comply with requirements shown for minimum lap of spliced bars, shall be as shown on the Drawings
- G. Mechanical Couplers:
1. Mechanical butt splices shall be in accordance with the recommendation of the manufacturer of the mechanical splicing device. Butt splices shall develop 125 percent of the specified minimum yield tensile strength of the spliced bars or of the smaller bar in transition splices. Bars shall be flame dried before butt splicing. Adequate jigs and clamps or other devices shall be provided to support, align, and hold the longitudinal centerline of the bars to be butt spliced in a straight line.
- H. Welded Splices
1. When permitted by the ENGINEER, in writing, all welding of reinforcing bars shall conform to ANSI/AWS D1.4. Preheating and rate of cooling requirements shall be based on bar steel chemistry and ANSI/AWS D1.4 requirements. Welded splices shall be sized and constructed to transfer a minimum of 125 percent of the specified minimum yield tensile strength of the spliced bars or of the smaller bar in transition splices. Unless otherwise permitted by the

ENGINEER, welding of crossing bars (tack welding) for assembly of reinforcement is prohibited.

2. Welding of wire to wire, and of wire or welded wire fabric to reinforcing bars or structural steels, shall conform to applicable provisions of ANSI/AWS D1.4 and any supplementary requirements by the ENGINEER for the particular application.
3. After completion of welding on coated reinforcing bars, coating damage shall be repaired as specified herein. All welds and all steel splice members when used to splice bars shall be coated with the same material used for repair of coating damage.

I. Drilled Dowels

1. Drilled dowels shall be reinforcing dowels set in a resin adhesive in a hole drilled into hardened concrete.
2. Holes shall be drilled to the adhesive anchor system manufacturer's recommended diameter and depth to develop the required pullout resistance but shall not be greater in diameter than 1/4-inch more than the nominal bar diameter nor less than 12 times the nominal bar diameter in depth.
3. The hole shall be drilled by methods which do not interfere with the proper bonding of the resin. Only masonry type drill bits shall be used.
4. Existing reinforcing steel in the vicinity of proposed holes shall be located prior to drilling. The location of holes to be drilled shall be adjusted to avoid drilling through or nicking any existing reinforcing bars only after approval by the ENGINEER.
5. The hole shall be brushed (non-metallic bristle brush only) and blown clean with clean, dry compressed air to remove all dust and loose particles.
6. Resin shall be injected into the hole through the injection system-mixing nozzle (and any necessary extension tubes) placed to the bottom of the hole. The discharge end shall be withdrawn as resin is placed but kept immersed to prevent formation of air pockets. The hole shall be filled to a depth that ensures that excess material is expelled from the hole during dowel placement.
7. Dowels shall be twisted during insertion into the partially filled hole so as to guarantee full wetting of the bar surface with resin. The bar shall be inserted slowly enough to avoid developing air pockets.

3.3 INSPECTION OF REINFORCEMENT

A. Reinforcement Congestion and Interferences:

1. Notify ENGINEER whenever the specified clearances between bars cannot be met.
2. Do not place any concrete until the ENGINEER submits a solution to reinforcing congestion problem.
3. Reinforcing may be moved as necessary to avoid interference with other reinforcing steel, conduits, or embedded items.

4. If bars are moved more than one bar diameter, obtain ENGINEER'S approval of resulting arrangement of reinforcing.
 5. No cutting of reinforcing shall be done without written approval of ENGINEER.
- B. Concrete shall not be placed until the reinforcing steel is inspected and permission for placing concrete is granted by ENGINEER. All concrete placed in violation of this provision will be rejected.
- C. Formwork for walls and other vertical members will not be closed up until the reinforcing steel is inspected and permission for placing concrete is granted by ENGINEER. All concrete placed in violation of this provision will be rejected.
- D. Testing of Drilled Dowels:
1. Employ the services of an independent testing laboratory to perform field quality testing of installed drilled dowels. A minimum of 10 percent (but no less than 15 dowels) shall be chosen at random by the testing laboratory and confirmed by the ENGINEER. Unless otherwise directed by the ENGINEER, drilled dowels shall be tensioned to twice the manufacturer's rated capacity of the anchor. Where dowels are located less than eight dowel diameters from the edge of concrete the ENGINEER will determine the tensile load required for the test.
 2. Provide access for the testing agency to places where Work is being produced so that required inspection and testing can be accomplished.
 3. CONTRACTOR shall be responsible for the failure of any drilled dowel. If any dowel fails, the testing shall be increased to 100 percent.
 4. Correct improper workmanship, remove and replace, or correct as directed by the ENGINEER, all drilled dowels found unacceptable or deficient, at no additional cost to the OWNER.
 5. Pay for all corrections and subsequent tests required to confirm the integrity of the drilled dowels.
 6. The independent testing and inspection agency shall complete a report for each area. The report shall summarize the observations made by the Inspector and be submitted to the ENGINEER.
- E. Inspection of Welded Splices: Employ a testing agency to perform field quality control testing of the welded splices. All welded splices shall be visually inspected. A minimum of five percent of butt splice welds shall be radiographically tested. Any weld which is deficient in any way shall be repaired to be completely sound at the CONTRACTOR'S expense.
- F. Special Inspection:
1. See Section 01416.
 2. Special Inspection is required for:
 - a. Reinforcing size, spacing, lap length and concrete cover.
 - 1) Inspect in accordance with Drawings and Specification.

- 2) Frequency: Inspect prior to each concrete pour.
- b. Drilled dowel adhesive anchoring system:
 - 1) Inspect in accordance with ICC-ES report.
 - 2) Frequency:
 - a) Inspect all adhesive anchors for the first 4 HRs of installation.
 - b) Inspect approximately 25 PCT of adhesive anchors thereafter.
 - c) Additional inspection will be required for different installer or if the quality of installation appears to vary.
- c. Mechanical splices.
 - 1) Inspect in accordance with ICC-ES report.
 - 2) Frequency:
 - a) Inspect all mechanical splices prior to placing concrete.
 - b) Inspect approximately 25 PCT of mechanical splices thereafter.
 - c) Additional inspection will be required for different installer or if the quality of installation appears to vary.

++ END OF SECTION ++

SECTION 03251

CONCRETE JOINTS

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Scope:
 - 1. Provide all labor, materials, equipment and incidentals as shown, specified and required to furnish and install concrete joints.
 - 2. The types of concrete joints required include the following:
 - a. Construction joints.
 - b. Expansion joints.
 - c. Control joints.
 - d. Isolation joints.
 - e. Waterstops.
- B. General: All joints subject to hydrostatic pressure or in contact with soil, except non-water bearing slabs-on-grade, shall be provided with continuous waterstop.

1.2 QUALITY ASSURANCE

- A. Standard Specifications Details:
 - 1. Conform all applicable requirements of Sections No. 505 and 729 of the Uniform Standard Specifications for Public Works Construction by the Maricopa Association of Governments (MAG) as supplemented by the City of Chandler. Where there is a conflict between MAG Standard Specifications as supplemented by the City of Chandler and this Specification, provisions of this Specification shall govern.
- B. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown or specified:
 - 1. ACI 301, Standard Specifications for Structural Concrete.
 - 2. ASTM C 920, Standard Specification for Elastomeric Joint Sealants.
 - 3. ASTM D 412, Test Methods for Vulcanized Rubber and Thermoplastic Rubbers and Thermoplastic Elastomers-Tension.
 - 4. ASTM D 471, Standard Test Method for Rubber Property—Effect of Liquids.
 - 5. ASTM D 624, Test Method for Tear Strength of Conventional Vulcanized Rubber and Thermoplastic Elastomers.
 - 6. ASTM D 994, Standard Specification for Preformed Expansion Joint Filler for Concrete (Bituminous Type).
 - 7. ASTM D 1056, Standard Specification for Flexible Cellular Materials-Sponge or Expanded Rubber.

8. ASTM D 1751, Standard Specification for Preformed Expansion Joint Filler for Concrete Paving and Structural Construction (Nonextruding and Resilient Bituminous Types).
9. ASTM D 1752, Standard Specification for Preformed Sponge Rubber and Cork Expansion Joint Fillers for Concrete Paving and Structural Construction.
10. ASTM D 2240, Test Method for Rubber Property – Durometer Hardness.
11. CRD-C572, U.S. Army Corps of Engineers Specifications for Polyvinyl-Chloride Waterstop.

C. All manufactured items shall be installed in accordance with manufacturer's instructions.

1.3 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
1. Comply with the requirements of Section 01332, Shop Drawing Procedures.
 2. Manufacturer's specifications and installation instructions for all materials required.
 3. Layout of all construction and expansion joint locations prior to the submittal of steel reinforcement Shop Drawings.
 4. Drawing indicating location, size and type of prefabricated waterstop joints.
 5. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

1.4 PRODUCT DELIVERY, STORAGE AND HANDLING

- A. All materials used for joints in concrete shall be stored on platforms or in enclosures and covered to prevent contact with the ground and exposure to the weather and

direct sunlight. Storage and handling requirements of the manufacturer shall also be followed.

PART 2 - PRODUCTS

2.1 GENERAL

- A. Subject to compliance with the Contract Documents, the manufacturers listed in this article are acceptable.

2.2 WATERSTOPS, PVC TYPE

- A. Acceptable manufacturers:
1. Sika Greenstreak Plastic Products.
 2. W.R Meadows.
 3. Vinylex Corporation.
 4. Bometals, Inc.
 5. Or equal.
- B. Materials:
1. Virgin polyvinyl chloride compound not containing scrap or reclaimed materials or pigment.
 2. Cast-in-place type: USACE CRD-572.
 3. Approved profiles as listed.
 - a. Construction joints:
 - 1) Ribbed: 6 inches wide by 3/8 inch.
 - 2) Sika Greenstreak Plastic Products Style #679, or equal.
 - b. Control joints:
 - 1) 6-inch wide by 3/8 inch thick with ribs and center bulb.
 - 2) Sika Greenstreak Plastic Products Style #705, or equal.
 - c. Expansion joint:
 - 1) 9 inches wide by 3/8 inch thick center bulb two inch outside diameter.
 - 2) Sika Greenstreak Plastic Products Style #700, or equal.
 4. Provide factory-made waterstop fabrications at all changes in direction, intersections and transitions, leaving only straight butt splices for the field. Butt welds to be a minimum six inches clear of the intersection.
 5. Factory prepunched (less than 1.5 feet on centers, each edge, staggered) for wire supports.
 - a. Provide hog rings or grommets at all punched holes along the length of the waterstop.
 6. See Drawings for application and other requirements.
- C. Waterstops, TPE/TPV:

1. Acceptable manufactures:
 - a. JP Specialties, Inc.
 - b. Westec.
 - c. Or equal.
 2. Materials:
 - a. Synthetic rubber with high resistance to oils solvents and aggressive chemicals.
 - b. Material shall allow not more than 35 percent maximum change in material properties after seven day exposure to selected chemicals, in accordance with ASTM D471.
 - c. Waterstop shall be resistant to the following chemicals:
 - 1) Diesel fuel.
- D. Waterstops, Preformed Strip Type:
1. Acceptable manufacturers:
 - a. Sika Greenstreak Plastics, Inc. (Hydrotite).
 - b. Adeka Ultra Seal USA (MC-2010MN).
 - c. DeNeef (Swellseal 2010).
 - d. Or equal.
 2. Hydrophilic, non-bentonite composition.
 3. Manufactured solely for the purpose of preventing water from traveling through construction joints.
 4. Volumetric expansion limited to three times maximum.
 5. See the Drawings for locations, applications, and other requirements.
- E. Water Swelling Sealant:
1. Required adhesive for use with strip-type waterstop.
 2. Compatible with strip-type waterstop.
 3. Single component, gun applied.
 4. Moisture cured.
 5. Minimum 70 percent volumetric expansion swelling capability.

2.3 CONCRETE CONSTRUCTION JOINT ROUGHENER

- A. Provide a water-soluble non-flammable, surface-retardant roughener.
- B. Product and Manufacturer: Provide one of the following:
1. Rugasol-S, as manufactured by Sika Corporation for horizontal joints only.
 2. Concrete Surface Retarder-Formula S, as manufactured by Euclid Chemical Company, for horizontal joints only.
 3. Concrete Surface Retarder-Formula F, as manufactured by Euclid Chemical Company, for vertical joints only.
 4. Or equal.

2.4 EPOXY BONDING AGENT

- A. Provide a two-component epoxy-resin bonding agent.
- B. Product and Manufacturer: Provide one of the following:
 - 1. Sikadur 32 Hi-Mod LPL, as manufactured by Sika Corporation.
 - 2. Eucopoxy LPL, as manufactured by the Euclid Chemical Company.
 - 3. Or equal.

2.5 EPOXY-CEMENT BONDING AGENT

- A. Provide a three component epoxy resin-cement blended formulated as a bonding agent.
- B. Product and Manufacturer: Provide one of the following:
 - 1. Sika Armatec 110 EpoCem, as manufactured by Sika Corporation.
 - 2. Corr-Bond, as manufactured by the Euclid Chemical Company.
 - 3. Or equal.

2.6 RUBBER BONDING AGENT

- A. Product and Manufacturer: Provide one of the following:
 - 1. Scotch-Grip 1300 Rubber Adhesive, as manufactured by 3M Company.
 - 2. Or equal.

2.7 ASPHALT EXPANSION JOINT FILLERS

- A. Acceptable manufacturers:
 - 1. W.R Meadows.
 - 2. J and P Petroleum Products.
 - 3. Or equal.

- B. Materials: ASTM D994.

2.8 FIBER EXPANSION JOINT FILLERS

- A. Materials: ASTM D1751.

2.9 PREFORMED EXPANSION JOINT FILLER

- A. Materials: ASTM D 1752, Type I (sponge rubber) or Type II (cork).

2.10 NEOPRENE EXPANSION JOINT FILLERS

- A. Acceptable manufacturers:
 - 1. Permaglaze.

2. Rubatex.
3. Williams Products.
4. Or equal.

B. Materials:

1. Closed cell neoprene.
2. ASTM D1056, Type 2, Class A or C.
3. Grade: Compression deflection as required to limit deflection to 25 percent of joint thickness under pressure from concrete pour height.

2.11 JOINT SEALANT

- A. Sealant used in expansion joints and other locations where it is shown and which will be subject to being submerged by water for any period of time shall be a two part polyurethane type sealant meeting the requirements of ASTM C 920, Type M, Class 25. The sealant shall be specially formulated for continuous submerged conditions. The manufacturer's recommended primer must be used with the sealant.
- B. The sealant shall meet the following requirements (measured at 73 degrees F and 50 percent RH):
1. Ultimate hardness (ASTM D 2240, Type A, Shore): 20 to 45.
 2. Tensile strength (ASTM D 412): 200 psi, minimum.
 3. Ultimate elongation (ASTM D 412): 400 percent, minimum.
 4. Tear strength (ASTM D 624, die C): 75 pounds per inch of thickness, minimum.
 5. Color: light gray.
- C. Product and Manufacturer: Provide one of the following:
1. Permapol RC-270 Reservoir Sealant, as manufactured by Products Research and Chemical Corporation.
 2. Sikaflex-2c, as manufactured by Sika Corporation.
 3. Or equal.

2.12 SEALANT ACCESSORIES

- A. Backer Rod: Backer rod shall be an extruded closed-cell polyethylene foam rod. The material shall be compatible with the sealant material used and shall have a tensile strength of not less than 40 psi and a compression deflection of approximately 25 percent at 8 psi. The rod shall be 1/8-inch larger in diameter than the joint width at joints less than 3/4-inch wide and 1/4-inch larger in diameter at joints 3/4-inch and wider.
- B. Bond Breaker Tape: Bond breaker shall be polyethylene or TFE-fluorocarbon self adhesive tape, as recommended by the manufacturer.

PART 3 - EXECUTION

3.1 INSPECTION

- A. Examine the substrate and the conditions under which Work is to be performed and notify ENGINEER, in writing, of unsatisfactory conditions. Do not proceed with the Work until unsatisfactory conditions have been corrected in a manner acceptable to ENGINEER.

3.2 JOINTS AND EMBEDDED ITEMS

- A. Construction Joints - General:
1. Provide joints at locations as shown or indicated on the Drawings or as shown on approved Shop Drawings approved by ENGINEER or in an appropriate Contract modification.
 - a. Where construction joint spacing shown on the Drawings exceeds the joint spacing indicated in Paragraph B. below, submit proposed construction joint location in conformance with this Section.
 2. Unplanned construction joints are unacceptable.
 - a. If concrete cannot be completely placed between planned construction joints, then it must be removed.
 3. In general, locate joints near middle of spans of slabs, beams and girders unless a beam intersects a girder at this point, in which case, offset joint in girder a distance equal to twice the width of the beam.
 4. Locate joints in walls and columns at underside of floors, slabs, beams, or girders, and at tops of foundations or floor slabs, unless shown otherwise.
 - a. At Contractor's option, beam pockets may be formed into concrete walls.
 - b. Size pockets to allow beam reinforcing to be placed as detailed on the Drawings.
 5. Place beams, girders, column capitals and drop panels at same time as slabs.
 6. Make joints perpendicular to main reinforcement with all reinforcement continuous across joints.
 7. Provide the following joints unless shown or indicated otherwise on the Drawings:
 - a. Roughen joints: horizontal and vertical construction joints.
 8. Roughen construction joints:
 - a. Clean the previously hardened concrete interface and remove all laitance.
 - b. Intentionally roughen the interface to a full amplitude of 1/4 inch.
 - c. Provide recessed flat surface as required to install strip type waterstops.
 9. Minimum time before placement of adjoining concrete construction:
 - a. All concrete: 60 hours, unless otherwise noted.
- B. Construction Joints - Spacing Unless Otherwise Specified:
1. Structures not intended to contain liquid:

- a. Wall vertical construction joints:
 - 1) 50 feet maximum on centers.
 - 2) At wall intersections, 4 feet minimum from corner.
- b. Base slab, floor, and roof slab construction joints:
 - 1) Placements to be approximately square and not to exceed 4,000 square feet.
 - 2) Maximum side dimension of a slab pour shall be 90 feet.
- 2. Water retaining structures:
 - a. Wall vertical construction joints:
 - 1) 30 feet maximum on centers.
 - 2) At wall intersections, 10 feet minimum from corner.
 - b. Wall horizontal construction joints: 18 feet on centers.
 - c. Walls that are thicker than 18 inches may be poured up to 24 feet tall in one pour provided the following requirements are satisfied:
 - 1) A test wall of similar size, height and thickness will be poured to demonstrate the quality of the concrete work.
 - 2) The test wall will be located as a portion of a non-water retaining wall.
 - a) The test wall will include a waterstop at the bottom of the wall.
 - b) Alternatively, a separate test wall, not part of the final Work, may be constructed.
 - 3) The concrete placement and concrete quality of the test wall will be observed by the ENGINEER.
 - a) Concrete will be judged on the following:
 - i) Ability to keep bottom of the pour clean and free from trash and debris.
 - ii) Ability to protect the waterstop from folding over due to the force of falling concrete.
 - iii) Ability to properly consolidate all concrete in the wall pour, including below formed openings.
 - 4) ENGINEER will evaluate the Contractor's work and may accept taller concrete wall pours if concrete quality is acceptable.
 - 5) Preparation for all subsequent wall pours over 18 feet tall must be the same as the preparation of the test wall.
 - 6) Should the quality of concrete work on subsequent wall pours be judged inadequate, when compared to the original accepted test wall, the Contractor may be directed to limit wall pour heights to 18 feet as originally specified.
 - 7) Should the quality of concrete work on the test wall be judged inadequate, additional test walls will not be observed and judged to allow pour heights greater than 18 feet unless approved by ENGINEER.
 - d. Floor slab, construction joints:

- 1) Placements to be approximately square and not to exceed 2,000 square feet.
 - 2) Maximum side dimension of a slab pour to be less than:
 - a) Twice the length of the short side.
 - b) 60 feet.
 - e. Elevated slab construction joints:
 - 1) Placements to be approximately square and not to exceed 4,000 square feet.
 - 2) Maximum side dimension of a slab pour to be less than:
 - a) Twice the length of the short side.
 - b) 70 feet.
- C. Construction Joints - Bonding:
1. Obtain bond between concrete pours at construction joints by thoroughly cleaning and removing all laitance from construction joints.
 2. Before new concrete is placed, all construction joints shall be coated with cement grout, or dampened, as indicated below:
 3. Roughen construction joints:
 - a. Roughen the surface of the concrete to expose the coarse aggregate uniformly with 1/4 inch minimum amplitude.
 - 1) Remove laitance, loosened particles of aggregate or damaged concrete at the surface.
 - b. Cover the hardened concrete of horizontal joints with a coat of cement grout of similar proportions to the concrete, except provide fine aggregate in lieu of coarse aggregate.
 - c. Provide 2 inches to 5 inches layer of grout in bottoms of walls with waterstops. Place grout immediately before placing concrete.
 - d. Place fresh concrete before the grout has attained its initial set.
 - 1) Vibrate grout and first layer of concrete simultaneously to commingle the materials.
 - e. At Contractor's option, 1/4 inch minimum amplitude may be created by utilizing a chemical retarder placed on the formed surface of the concrete in conjunction with power washing.
 - 1) See Paragraph 2.3 for product information.
- D. Slab On Grade Joints:
1. Construction joints:
 - a. Locate construction and control joints in slabs on grade as shown or indicated on the Drawings.
 - b. Construction joints act as control joints.
 2. Control joints:
 - a. Provide contraction (control) joints as indicated.
 - b. If not shown, provide along column centerlines.

- c. Where column centerline spacing or spacing between column centerlines and walls exceeds 20 feet, provide an intermediate joint at intervals not exceeding 20 feet.
- d. Locate joints to produce panels that are as square as possible with length not exceeding 1.5 times width.
- e. Also provide joints where change in slab width occurs, such as at block-outs, pits, etc.
- f. If saw cut joints are required or permitted, time cutting properly with set of concrete by one of the following methods:
 - 1) A conventional saw, cutting as soon as possible after final finishing when cutting action will not tear, ravel, abrade, or otherwise damage surface and before concrete develops random contraction cracks. Cut to a depth of 1/4 slab thickness or 1/3 slab thickness if steel-fiber or structural synthetic macro fiber reinforcement is used.
 - 2) An early entry saw beginning immediately after final finishing when cutting action will not tear, ravel, abrade, or otherwise damage surface and before concrete develops random contraction cracks.
 - a) Remove debris in path of cut and under skid plate before cutting. Skid plate must remain flat on surface.
 - b) Use early entry saw blades and skid plates.
 - c) Install early entry joint protector at saw-cut intersection prior to cross-cut.
 - d) Remove dry powder without disturbing finish.
 - e) Avoid traffic across saw cut until sufficient strength is gained to protect joint edges.
 - 3) Complete before shrinkage stresses become sufficient to produce cracking.

E. Isolation joints:

- 1. Wherever a sidewalk or other slab on grade abuts a concrete structure and is not shown doweled into that structure, an isolation joint shall be provided. Such joint shall be formed by a 1/2-inch joint filler with the upper 1/2-inch of the joint filled with sealant unless otherwise indicated on the Drawings.

F. Expansion Joints:

- 1. Do not allow reinforcement or other embedded metal items bonded to concrete (except smooth dowels bonded on only one side of joint) to extend continuously through an expansion joint.
- 2. Use neoprene expansion joint fillers, unless shown or indicated otherwise on the Drawings.
- 3. Locate and install expansion joints as shown. Install joint filler in accordance with manufacturer's instructions. Sealants shall be installed as specified herein.

G. Waterstops - General:

1. Waterstop shall be continuous with splices in accordance with manufacturer's instructions and create water tight joints.
 2. Do not mix different types of waterstop materials in the same structure without approval from the ENGINEER unless shown or indicated on the Drawings.
 3. Preformed strip type:
 - a. Locate waterstop at center of wall, unless shown or indicated otherwise on the Drawings.
 - 1) Maintain at least three inches from edge of concrete or as recommended by manufacturer.
 - b. Install in a bed of swelling sealant on smooth surface of hardened concrete by use of nails, adhesive or other means as recommended by manufacturer to prevent movement of waterstop during placement of concrete.
 - c. Roughened joints shall be specially prepared during concrete placement to provide smooth surface for proper water stop installation.
 - d. Use in joints against existing concrete where shown or indicated on the Drawings.
 4. PVC/ TPE/TPV waterstops:
 - a. Pre-position waterstop accurately in joints, with adequate clearance from all reinforcing. Do not push waterstop into wet concrete.
 - b. Secure waterstops in correct position using hog rings or grommets spaced no more than 1.5 feet maximum staggered along each edge full length and passed through the edge of the waterstop.
 - 1) Tie wire to adjacent reinforcing.
 - c. Provide retrofit waterstops with batten bars, anchors and adhesive gel as specified by manufacturer.
 - d. Hold horizontal waterstops in place with continuous supports.
 - e. Install according to manufacturer's instructions.
 - 1) Do not displace reinforcement from required location.
 - f. Splice ends and intersections with perpendicular butt splice using electrical splicing iron in accordance with manufacturer's instructions.
 - 1) Use factory fabricated "T" and corner intersection fittings.
 - 2) Field splice straight runs of material.
 - g. Unless otherwise noted, provide for construction joints in new construction for all structures indicated on the Drawings.
- H. Other Embedded Items:
1. Place sleeves, inserts, anchors, and embedded items required for adjoining work or for its support, prior to initiating concreting.
 2. Do not route electrical conduit, drains, or pipes in concrete slabs, walls, columns, foundations, beams or other structural members unless approved by ENGINEER.
- I. Placing Embedded Items:
1. Support against displacement.

2. Fill voids in sleeves, inserts and anchor slots temporarily with readily removable material to prevent entry of concrete into voids.
3. Provide adequate means for anchoring waterstop in concrete.
 - a. Provide means to prevent waterstops in the forms from being folded over by the concrete as it is placed.

3.3 BONDING AGENT

- A. Use epoxy bonding agent for bonding of fresh concrete to concrete that has been in place for at least 60 days or to existing concrete.
- B. Use epoxy-cement bonding agent for the following:
 1. Bonding toppings and concrete fill to concrete that has been in place for at least 60 days or to existing concrete.
 2. For all locations where bonding agent is required and concrete cannot be placed within the open time period of epoxy bonding agent.
 3. Bonding of horizontal construction joints where these are required by the Drawings or approved by ENGINEER for foundation mats that are five feet thick or greater.
- C. Use a cement-water slurry as a bonding agent for toppings and concrete fill to new concrete. The cement water slurry shall be worked into the surface with a stiff bristle broom and concrete shall be placed before the cement-water slurry dries.
- D. Handle and store bonding agent in compliance with the manufacturer's printed instructions, including safety precautions.
- E. Mix the bonding agent in complete accordance with the instructions of the manufacturer.
- F. Before placing fresh concrete, thoroughly roughen and clean hardened concrete surfaces and coat with bonding agent not less than 1/16-inch thick. Place fresh concrete while the bonding agent is still tacky (within its open time), without removing the in-place bonding agent coat, and as directed by the manufacturer.

3.4 SEALANT INSTALLATION

- A. Sealants shall be installed according to the manufacturer's recommendations for sealant which is to be subjected to continuous submerged conditions and the following requirements. Prior to sealant installation, arrange to have a representative of the sealant manufacturer instruct the crew doing the Work as to the proper methods of surface preparation, mixing, and application of the sealant.
- B. Surfaces to receive sealant shall be cleaned of all materials which could interfere with proper bonding. Concrete surfaces shall have all fins or other defects removed or

- repaired and shall receive a light abrasive blasting prior to priming and sealant application. All surfaces to receive sealant shall be completely dry.
- C. Spaces to receive sealant shall be filled with joint filler as shown. Where not shown, the space shall be filled with joint filler or a backer rod so that the depth of sealant does not exceed the width of the space. Where the bottom of the space to receive sealant is formed by a material other than backer rod, a bond breaker tape shall be placed. The maximum sealant depth, at middle of the joint width, shall be 1/2-inch.
 - D. The primer and sealant used shall be supplied by the same manufacturer. No sealant shall be placed without the use of a primer.
 - E. Self-leveling sealants shall only be used in joints with a slope less than 0.5 percent and where maximum and minimum sealant depths can be maintained. Non-sag sealant shall be used at all other locations and may be used instead of self-leveling sealant. All non-sag sealant shall be tooled to a uniform concave surface before skinning and curing begins.
 - F. Sealant material shall be conditioned to be within the optimum temperature range recommended by the manufacturer for installation for a minimum of 16 hours prior to installation. Installation shall proceed only when the substrate is at a temperature recommended by the manufacturer. Sealant shall not be placed if there is a threat of imminent rainfall. Submit a letter certifying that the applied sealants were installed in accordance with the manufacturer's recommendations, including temperature, relative humidity, etc.
 - G. All joints to receive sealant shall be inspected by the ENGINEER prior to sealant placement.
 - H. All sealant shall achieve final cure at least seven days before the structure is filled with water.
 - I. Any sealant which, after the manufacturer's recommended curing time for the job conditions, fails to fully and properly cure shall be completely removed. The surfaces to receive sealant shall be completely cleaned of all traces of the improperly cured sealant and primer. The specified sealant shall then be reinstalled. All costs of such removal, surface treatment, and reinstallation shall be at the expense of CONTRACTOR.

3.5 FIELD QUALITY CONTROL

- A. Special Inspection:
 - 1. See Section 01416.
 - 2. Special Inspection (non-code required) is required for:
 - a. Concrete Joints:

- 1) Inspect concrete joints for proper joint type, dimensions, reinforcing, dowel alignment, surface preparation and location.
- 2) Frequency: Prior to each concrete pour.
- b. Waterstops:
 - 1) Visually inspect waterstops for proper location, continuity, installation to prevent displacement, cleanliness and damage to waterstop.
 - 2) Frequency:
 - a) Prior to each concrete pour
 - 3) Testing PVC/TPE/TPV Waterstop Joints:
 - a) Samples of field fabricated waterstop butt joints may be selected at random by the ENGINEER for testing by a laboratory at the CONTRACTOR'S expense.
 - i) When tested, they shall have a tensile strength across the joints equal to at least 600 psi.
- B. Supplier's Onsite Services:
 1. Waterstop manufacturer's representative shall provide on-site training of waterstop installation, field splicing, welding and inspection procedures prior to construction.

++ END OF SECTION ++

SECTION 03252

ANCHORAGE IN CONCRETE

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Scope:
 - 1. Provide all labor, materials, equipment and incidentals as shown on the Drawings, specified, and required to furnish and install anchor bolts, concrete anchors (adhesive and expansion anchors), and concrete inserts.
- B. Coordination: This Section includes all bolts, anchors and inserts required for the Work but not specified under other Sections.
- C. The types of work using anchor bolts and anchors drilled into concrete includes, but is not limited to, the following:
 - 1. Structural members and accessories.
 - 2. Metal, wood, and plastic fabrications.
 - 3. Equipment.
 - 4. Sluice and slide gates.
 - 5. Stop log frames
 - 6. Piping.
 - 7. Grating and floor plate.

1.2 QUALITY ASSURANCE

- A. Reference Standards: Comply with the applicable provisions and recommendations of the following, except as otherwise shown and specified.
 - 1. ACI 318, Building Code Requirements for Structural Concrete and Commentary.
 - 2. ACI 355.2, Seismic Testing of Post-Installed Concrete and Masonry Anchors in Cracked Concrete.
 - 3. ACI 355.4, Qualification of Post-Installed Adhesive Anchors in Concrete.
 - 4. American Concrete Institute/Concrete Reinforcing Steel Institute (ACI-CRSI):
 - a. Adhesive Anchor Installation Certification Program: Adhesive Anchor Installer.
 - 5. ASTM A 36, Specification for Structural Steel.
 - 6. ASTM A 108, Standard Specification for Steel Bar, Carbon and Alloy, Cold-Finished.
 - 7. ASTM A 123, Specification for Zinc (Hot-Dip Galvanized) Coatings on Iron and Steel Products.

8. ASTM A 153, Specification for Zinc Coating (Hot-Dip) on Iron and Steel Hardware.
 9. ASTM A 307, Specification for Carbon Steel Bolts and Studs, 60,000 psi Tensile Strength.
 10. ASTM A 484/A 484M, Specification for General Requirements for Stainless and Heat-Resisting Steel Bars, Billets and Forgings.
 11. ASTM A 525, Specification for General Requirements for Steel Sheet, Zinc-Coated (Galvanized) by the Hot-Dip Process.
 12. ASTM A 536, Specification for Ductile Iron Castings.
 13. ASTM A 570/A 570M, Specification for Structural Steel, Sheet and Strip, Carbon, Hot-Rolled.
 14. ASTM B 633, Specification for Electrodeposited Coatings of Zinc on Iron and Steel.
 15. ASTM F 593, Specification for Stainless Steel Bolts; Hex Cap Screws, and Studs.
 16. ASTM F 1554, Standard Specification for Anchor Bolts, Steel, 36, 55, and 105-ksi Yield Strength.
 17. ASTM F 2329, Standard Specification for Zinc Coating, Hot-Dip, Requirements for Application to Carbon and Alloy Steel Bolts, Screws, Washers, Nuts, and Special Threaded Fasteners
 18. Federal Specification, FF-S-325 for Concrete Expansion Anchors.
 19. Federal Specifications, WW-H-171E for Malleable Iron.
 20. ICC Evaluation Service (ICC-ES):
 - a. AC193, Acceptance Criteria for Mechanical Anchors in Concrete Elements.
 - b. AC308, Acceptance Criteria for Post-Installed Adhesive Anchors in Concrete Elements.
- B. Expansion anchors and inserts shall be ICBO, UL, or FM approved.
- C. Qualifications:
1. Anchor designer for Contractor-designed post-installed anchors and cast in place anchorage shall be a professional civil engineer licensed in the State of Arizona.
 2. Installer for post-installed anchors shall be trained by the manufacturer or certified by a training program approved by the ENGINEER.
 3. Installer for adhesive anchors installed in horizontal, upward incline, or overhead applications shall be certified by ACI-CRSI Adhesive Anchor Installation Certification Program.
- D. Post-installed anchors and related materials shall be listed by the following agencies:
1. ICC-ES.
 2. Engineer approved equivalent.

1.3 DEFINITIONS

- A. Adhesive Anchors:
 - 1. Post-installed anchors developing their strength primarily from chemical bond between the concrete and the anchor.
 - 2. Includes anchors using acrylics, epoxy and other similar adhesives.
- B. Anchor Bolt: Any cast-in-place anchorage that is made of a headed (i.e. bolt) material.
- C. Anchor Rod: Any cast-in-place or post-installed anchorage made from unheaded, threaded, rod or deformed bar material.
- D. Concrete Anchor: Generic term for either an anchor bolt or an anchor rod.
- E. Galvanizing: Hot-dip galvanizing per ASTM A123, ASTM A153 or ASTM F2329 with minimum coating of 2.0 oz of zinc per square foot of metal (average of specimens) unless noted otherwise or dictated by standard.
- F. Hardware: As defined in ASTM A153 and ASTM F2329.
- G. Installer or Applicator:
 - 1. Installer or applicator is the person actually installing or applying the product in the field at the Project site.
 - 2. Installer and applicator are synonymous.
- H. MPII: Manufacturer's printed installation instructions.
- I. Mechanical Anchors:
 - 1. Post-installed anchors developing their strength from attachment other than thru adhesives or chemical bond to concrete.
 - 2. Includes expansion anchors, expansion sleeve, screw anchors, undercut anchors, specialty inserts and other similar types of anchorages.
 - 3. Drop-in anchors and other similar non-ICC ES approved anchors are not allowed.
- J. Post-Installed Anchor: Any adhesive or mechanical anchor installed into previously placed and adequately cured concrete.

1.4 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
 - 1. Comply with the requirements of Specification Section 01332, Shop Drawing Procedures.
 - 2. Product technical data including:

- a. Acknowledgement that submitted products meet requirements of referenced standards.
- b. Manufacturer material data sheet for each anchor.
 - 1) Clearly indicate which products on the data sheet are proposed for use on the Project.
- c. Manufacturer's printed installation instructions.
- d. Current ICC-ES report for each post-installed anchor system indicating the following:
 - 1) Certification that anchors meet all requirements indicated in this Specification.
 - 2) Performance data showing that anchor is approved for use in cracked concrete.
 - 3) Seismic design categories for which anchor system has been approved.
 - 4) Required installation procedures.
 - 5) Special inspection requirements for installation.
- e. Contractor Designed Post-Installed Anchors:
 - 1) Show diameter and embedment depth of each anchor.
 - 2) Indicate compliance with ACI 318, Chapter 17.
 - 3) Design tension and shear loads used for anchor design.
 - 4) Engineering design calculations:
 - a) Indicate design load to each anchor.
 - b) Develop anchor forces based on Design Criteria listed herein and/or manufacturer's design information.
 - c) Sealed and signed by contractor's professional engineer.
 - d) Calculations will be submitted for information purposes only.
 - 5) Provide torque values where pipe sleeves anchors are used.
- f. Anchorage layout drawings and details:
 - 1) Indicate anchor diameter, embedment, length, anchor type, material and finish.
 - 2) Drawings showing location, configuration, spacing and edge distance.
 - 3) Type of post-installed anchor system used.
 - a) Provide manufacturer's ICC-ES report for the following:
 - i) Mechanical anchorage per ICC-ES AC193.
 - ii) Adhesive anchorage per ICC-ES AC308.

B. Samples:

1. Representative samples of concrete anchors may be requested by ENGINEER. Review will be for type and finish only. Compliance with all other requirements is exclusively the responsibility of the Contractor.

C. Informational Submittals:

1. Certification of qualifications for each installer of post-installed anchors.
2. Indicate successful completion or certification for each type of approved post-installed anchor as required by the Contract Documents.

3. Provide one of the following for each type of anchor, as required by this specification section:
 - a. Letter from manufacturer documenting successful training completion [for mechanical anchors only].
 - b. Certification of completion for Engineer approved program.
- D. Shop Drawings: Submit for approval the following:
 1. Refer to and comply with the requirements of Section 01332, Shop Drawing Procedures.
 2. Setting drawings and templates for location and installation of anchorage devices.
 3. Copies of manufacturer's specifications, load tables, dimension diagrams and installation instructions for the anchorage devices.
 4. Copies of Building Code Approval Agency's Evaluation Report, UL, or FM reports certifying design values for load carrying capacities along with installation requirements for the anchorage devices.
 5. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures.
 6. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

1.5 DELIVERY, STORAGE, AND HANDLING

- A. Deliver products to job site in manufacturer's or distributor's packaging undamaged and complete with installation instructions.
- B. Store above ground on skids or other supports to keep items free of dirt and other foreign debris and to protect against corrosion.

- C. Protect and handle materials in accordance with manufacturer's recommendations to prevent damage or deterioration.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Cast-in-place Concrete Anchors:
1. Building and nonbuilding structures, unless otherwise specified:
 - a. ASTM F1554, Grade 36 or Grade 55 with weldability supplement S1 for galvanized or non-galvanized threaded rods.
 - b. ASTM A307, Grade A for galvanized headed bolts.
 - c. For Submerged or used in enclosed tankage, ASTM F593, Type 316.
 2. All other cast-in-place concrete anchors for equipment:
 - a. Stainless steel with matching nut and washer.
 - b. Submerged application: ASTM F593, Type 316.
 - c. Non-submerged application: ASTM F593, Type 304 or Type 316 anchors and washers
 - d. Nuts for non-submerged applications, ASTM563 and coated after torqued.
- B. Post-Installed Mechanical and Adhesive Concrete Anchors:
1. Stainless steel with matching nut and washer.
 2. Submerged application: ASTM F593, Type 316.
 3. Non-submerged application: ASTM F593, Type 304 or Type 316, with ASTM A563 nuts coated after torquing.
- C. Reinforcement: See Specification Section 03200, Concrete Reinforcement.
- D. Headed Studs: ASTM A108 with a minimum yield strength of 50,000 psi and a minimum tensile strength of 60,000 psi.
- E. Deformed Bar Anchors: ASTM A496 with minimum yield strength of 70,000 psi and a minimum tensile strength of 80,000 psi.
- F. Washers:
1. ASTM F436 unless noted otherwise, finish to match bolt.
 2. If stainless steel anchorage is being used for cast-in-place anchorage, furnish washers of the same material and alloy as in the accompanying anchorage.
 3. Plate washers: Minimum 1/4 inches thick fabricated ASTM A36 square plates as specified or required.
 4. Follow manufacturer's requirements for all post-installed anchorage.
- G. Nuts:
1. ASTM A563 for all cast-in-place anchorage.

2. If stainless steel anchorage is being used for cast-in-place anchorage in submerged or enclosed tankage, nuts shall meet ASTM F594 and be the matching material and alloy as in the accompanying anchorage.
3. Follow manufacturer's requirements if using post-installed anchorage.

H. Galvanizing Repair Paint:

1. High zinc dust content paint for regalvanizing welds and abrasions.
2. ASTM A780.
3. Zinc content: Minimum 92% in dry film.
4. ZRC "ZRC Cold Galvanizing" or Clearco "High Performance Zinc Spray."

2.2 CONTRACTOR DESIGNED ANCHORAGE

A. Manufacturers:

1. Post-installed anchor systems for the listed manufacturers will be considered only if a current ICC-ES evaluation report is submitted in accordance with the SUBMITTALS Article in PART 1 of this Specification Section and if the anchor system is approved by the ENGINEER.
 - a. Hilti.
 - b. Dewalt.
 - c. Simpson Strong-Tie.

B. Design the anchorage when any of the following occur:

1. Design load for concrete anchorage is shown on the Drawings.
2. When specifically required by the Contract Documents.
3. When an anchorage is required but not specified in the Drawings.

C. Anchorage Design Loads:

1. Determine all of the design loads, including wind and seismic loads, per the building code. Additionally, specialty engineer is required to incorporate all static and dynamic loading forces established by the equipment manufacturer, where required for equipment.
 - a. Anchorage of equipment and non-structural components: Use the actual dead and operating loads provided by the manufacturer.
2. When Contract Drawings indicate an anchor diameter or length, design shall incorporate these as "minimums." Design these anchors and provide all necessary documentation as required herein.

D. Cast-in-Place Concrete Anchors:

1. Provide the material, nominal diameter, embedment length, spacing, edge distance and design capacity to resist the calculated load based on the requirements given in the building code including ACI 318, Chapter 17.
2. Design assuming cracked concrete.

E. Post-installed Concrete Anchors:

1. Provide the manufacturer's system name/type, nominal diameter, embedment depth, spacing, minimum edge distance, cover, and design capacity to resist the specified or calculated load based on requirements given in the building code, ACI 318, Chapter 17 and current ICC-ES report, for the anchor to be used.
2. Design assuming cracked concrete.

2.3 ENGINEER DESIGNED ANCHORAGE

A. When the size, length and details of anchorages are shown on Contract Structural Drawings, Contractor design of anchorage is not required unless otherwise indicated.

B. Manufacturers:

1. Additional newer post-installed anchor systems for the listed manufacturers will be considered only if a current evaluation agency report is submitted in accordance with the SUBMITTALS Article in PART 1 of this Specification Section, the anchor system is certified by ICC-ES for cracked concrete conditions, and if approved by the ENGINEER.
2. Mechanical Anchors in Concrete:
 - a. Hilti:
 - 1) Kwik Bolt TZ2 (ICC-ES ESR-4266).
3. Adhesive Anchors in Concrete:
 - a. Hilti:
 - 1) HIT RE 500 V3 (ICC ESR-3814).
4. Screw Anchors in Concrete:
 - a. Hilti:
 - 1) KH-EZ Screw (ICC-ES ESR-3027).
5. Substitution request to indicate the proposed anchor has at least the same tension and shear strength as the specified anchor installed as indicated in the Contract Drawings.
 - a. Calculations to be stamped by a Professional Engineer registered in the State of Arizona.

PART 3 - EXECUTION

3.1 GENERAL

A. Cast-in-Place Anchorage:

1. Use where anchor rods or bolts are indicated on the Drawings, unless another anchor type is approved by the ENGINEER.
2. Provide concrete anchorage as shown on the Drawings or as required to secure components to concrete.

- B. Adhesive Anchorage:
 - 1. Use only where specifically indicated on the Drawings or when approved for use by the ENGINEER.
 - 2. May be used where subjected to vibration or where buried or submerged.
 - 3. Do not use in overhead applications or sustained tension loading conditions such as utility hangers.
 - 4. Contact ENGINEER for approval when anchors will not be installed in compliance with MPII
- C. Mechanical Anchorage:
 - 1. Use only where specifically indicated on the Drawings or when approved for use by the ENGINEER.
 - 2. Do not use where subjected to vibration.
 - 3. May be used in overhead applications.
 - 4. Contact ENGINEER for clarification when anchors will not be installed in compliance with manufacturer's printed installation requirements.
- D. Do not use powder actuated fasteners and other types of bolts and fasteners not specified herein for structural applications unless approved by the ENGINEER or specified in Contract Documents.

3.2 PREPARATION

- A. Provide adequate time to allow for proper installation and inspection prior to placing concrete for cast-in-place concrete anchorage.
- B. Prior to installation, inspect and verify areas and conditions under which concrete anchorage is to be installed.
 - 1. Notify ENGINEER of conditions detrimental to proper and timely completion of work.
 - 2. Do not proceed with work until unsatisfactory conditions have been corrected in a manner acceptable to the ENGINEER.
- C. Special Inspection is required in accordance with the building code for all concrete anchorage.
 - 1. Notify the Special Inspector that an inspection is required prior to concrete placement (or during post-installed anchorage installation).
 - 2. See the FIELD QUALITY CONTROL Article in PART 3 of this Specification Section for additional requirements.
- D. Post-installed anchor manufacturer's representative shall demonstrate and observe the proper installation procedures for the post-installed anchors at no additional expense to the OWNER.
 - 1. Follow such procedures to assure acceptable installation.

2. Adhesive anchors must be installed in concrete aged a minimum of 21 days.

3.3 INSTALLATION

- A. Tie cast-in-place anchorage in position to embedded reinforcing steel using wire.
 1. Tack welding of anchorage is prohibited.
 2. Chase threads as required and coat the projected portion of carbon steel anchors and nut threads with a heavy coat of clean grease after concrete has cured.
 3. Anchorage location tolerance shall be in accordance with AISC 303.
 4. Provide steel or durable wood templates for all column and equipment anchorage.
 - a. Templates to be placed above top of concrete and not impede proper concrete placement and consolidation.
- B. Unless noted or specified otherwise:
 1. Connect aluminum and steel members to concrete using stainless steel cast-in-place anchorage unless shown otherwise.
 2. Provide washers for all anchorage.
 3. Where exposed, extend threaded anchorage a maximum of 3/4 inches and a minimum of 1/2 inches above the top of the fully engaged nut.
 - a. If anchorage is cut off to the required maximum height, threads must be dressed to allow nuts to be removed without damage to the nuts.
- C. Do the following after nuts are snug-tightened down:
 1. If using post-installed anchorage, follow MPII.
 2. Upset threads of anchorage to prevent nuts from backing off.
 - a. Provide double nut or lock nut in lieu of upset threads for items that may require removal in the future.
 3. For all other cast-in-place anchorage material, tighten nuts down an additional 1/8 turn beyond snug tight to prevent nuts from backing off.
 4. If two nuts are used per concrete anchor above the base plate, tighten the top nut an additional 1/8 turn to "lock" the two nuts together.
 5. If using post-installed anchorage, follow MPII.
- D. Assure that embedded items are protected from damage and are not filled in with concrete.
- E. Secure architectural components such that it will not be aesthetically distorted and fasteners will not be overstressed from expansion, contraction, or installation.
- F. Repair damaged galvanized surfaces in accordance with ASTM A780.
 1. Prepare damaged surfaces by abrasive blasting or power sanding.
 2. Apply galvanizing repair paint to minimum 6 mils DFT in accordance with manufacturer's instructions and ASTM A780.

- G. For post-installed anchors, comply with the MPII on the hole diameter and depth required to fully develop the tensile strength of the anchor or reinforcing bar.
 - 1. Use hammer drills to create holes.
 - 2. Properly clean out the hole per the ICC-ES reports utilizing a non-metallic fiber bristle brush and compressed air or as otherwise required to remove all loose material from the hole prior to installing the anchor in the presence of the Special Inspector.

3.4 FIELD QUALITY CONTROL

- A. Special Inspection:
 - 1. See Specification Section 01416, Special Inspections.
- B. Field Inspection and Testing
 - 1. CONTRACTOR to employ a testing laboratory to perform field quality testing of all installed anchors. Such testing shall conform to requirements of ACI 355.2 and/or ACI 355.4 as applicable.
 - a. OWNER reserves the right to inspect and test completed anchorage at a minimum of 10% (for large quantity) to 25% (smaller quantity) to 100% (very small project quantity). . ENGINEER is to determine the level of testing which is required for the various types of adhesive anchors and anchor bolts.
 - b. Such testing shall conform to requirements of ACI 355.2 and/or ACI 355.4 as applicable.
 - 2. If failure of any of the tested anchors or reinforcing bars occurs, the OWNER reserves the right to inspect and test all remaining anchors of the same type.
 - 3. The independent testing and inspection agency shall complete a report on each area of the Work where concrete anchors are installed. The report shall summarize the observations made by the inspector and be submitted to ENGINEER.
 - 4. Failed anchors shall be satisfactorily replaced at no cost to OWNER.

3.5 CLEANING

- A. After concrete has been placed, remove protection and clean all anchorage of all concrete, dirt, and other foreign matter.

++ END OF SECTION ++

SECTION 03300

CAST-IN-PLACE CONCRETE

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. Provide all labor, materials, equipment and incidentals as shown on the Drawings, specified and required to furnish and install cast-in-place concrete.
2. The Work includes providing concrete consisting of portland cement, fine and coarse aggregate, water, and approved admixtures; combined, mixed, transported, placed, finished and cured. The Work also includes:
 - a. Providing openings in concrete to accommodate the Work under this and other Sections and building into the concrete all items such as sleeves, frames, anchor bolts, inserts and all other items to be embedded.

B. Coordination:

1. Review installation procedures under other Sections and coordinate the installation of items that must be installed in the concrete.

C. Classifications of Concrete:

1. Type "1" concrete shall be steel reinforced and includes the following:
 - a. All cast-in-place concrete, unless indicated otherwise.
2. Type "2" concrete shall be placed without forms or with simple forms, with little or no reinforcing, and includes the following:
 - a. Concrete fill within structures greater than or equal to 4-inches thick. Where less than 4-inches thick, provide Grout Fill topping grout in conformance with Section 03600 Grout.
 - b. Duct banks.
 - c. Unreinforced encasements.
 - d. Curbs and gutters.
 - e. Sidewalks.
 - f. Thrust blocks.
3. Type "3" concrete shall be steel reinforced and includes the following:
 - a. All precast concrete, unless indicated otherwise.
4. Type "4" concrete shall be unreinforced and used where required as concrete fill under foundations, filling abandoned piping and wherever "lean" concrete is required on the Drawings. This concrete shall be one-sack Controlled Low Strength Material (CLSM) conforming to MAG Specification Section No. 728.

1.2 QUALITY ASSURANCE

A. Standard Specifications and Details:

1. Conform all applicable requirements of Sections Nos. 505, 725, 726 and 728 of the Uniform Standard Specifications for Public Works Construction by the Maricopa Association of Governments (MAG). Where there is a conflict between MAG Standard Specifications and this Specification, provisions of this Specification shall govern.
- B. Reference Standards: Comply with the applicable provisions and recommendations of the following, except as otherwise shown or specified.
1. ACI 214, Recommended Practice for Evaluation of Strength Test Results of Concrete.
 2. ACI 301, Specifications for Structural Concrete (includes ASTM Standards referred to herein).
 3. ACI 304, Guide for Measuring, Mixing, Transporting and Placing Concrete.
 4. ACI 305, Hot Weather Concreting.
 5. ACI 306, Cold Weather Concreting.
 6. ACI 309, Guide for Consolidation of Concrete.
 7. ACI 311, Guide for Concrete Inspection.
 8. ACI 318, Building Code Requirements for Structural Concrete.
 9. ACI 350, Code Requirements for Environmental Engineering Concrete Structures.
 10. ANSI/NSF 61, Drinking Water System Components-Health Effects.
 11. AASHTO M 182, Burlap Cloth Made From Jute or Kenaf.
 12. AASHTO TP 23, Proposed Standard Method of Test for Water Content of Freshly Mixed Concrete Using Microwave Oven Drying.
 13. ASTM C 31, Practice for Making and Curing Concrete Test Specimens in the Field.
 14. ASTM C 33, Specification for Concrete Aggregates.
 15. ASTM C 39, Test Method for Compressive Strength of Cylindrical Concrete Specimens.
 16. ASTM C 42, Test Method for Obtaining and Testing Drilled Cores and Sawed Beams of Concrete
 17. ASTM C 94, Specification for Ready-Mixed Concrete.
 18. ASTM C109, Test Method for Compressive Strength of Hydraulic Cement Mortars.
 19. ASTM C 143, Test Method for Slump of Hydraulic- Cement Concrete.
 20. ASTM C 150, Specification for Portland Cement.
 21. ASTM C 157, Test Method for Length Change of Hardened Hydraulic-Cement Mortar and Concrete.
 22. ASTM C 171, Specification for Sheet Materials for Curing Concrete.
 23. ASTM C 172, Practice for Sampling Freshly Mixed Concrete.
 24. ASTM C 231, Test Method for Air Content of Freshly Mixed Concrete by the Pressure Method.
 25. ASTM C 260, Specification for Air-Entraining Admixtures for Concrete.
 26. ASTM C 309, Specification for Liquid Membrane-Forming Compounds for Curing Concrete.

27. ASTM C 330, Specification for Lightweight Aggregates for Structural Concrete.
28. ASTM C 494, Specification for Chemical Admixtures for Concrete.
29. ASTM C 618, Specification for Fly Ash and Raw or Calcined Natural Pozzolan for Use as a Mineral Admixture in Portland Cement Concrete.
30. ASTM C 882, Test Method for Bond Strength of Epoxy-Resin Systems Used with Concrete by Slant Shear.
31. ASTM C 1240, Specification for Silica Fume for Use as a Mineral Admixture in Hydraulic-Cement Concrete, Mortar, and Grout.
32. ASTM C 1778, Standard Guide for Reducing Risk of Deleterious Alkali-Aggregate Reaction in Concrete.
33. ASTM E 154, Test Methods for Water Vapor Retarders Used in Contact with Earth Under Concrete Slabs, on Walls, or as Ground Cover.
34. ASTM E 329, Specification for Agencies Engaged in the Testing and/or Inspection of Materials Used for Construction.

C. Concrete Testing Service:

1. Field testing services for concrete will be provided by CONTRACTOR. Inspection services for concrete placement will be provided by ENGINEER.
2. The CONTRACTOR shall provide a qualified Engineering Technician who will be provided to sample the concrete. The concrete will be tested for slump, air content, and temperature at the time of placement. Sets of four (4) cylinders will be molded at each sampling. Samples shall be taken per 10 cubic yards, with a minimum of one sample per class of concrete per day. Samples will be field cured at the site and returned to selected laboratory for moist curing prior to compressive strength testing. Compressive strength testing will be conducted at 7-day (1 cylinder) and 28-day (3 cylinders) intervals. CONTRACTOR to provide concrete Sample reports to ENGINEER electronically.
 - 1) Testing agency shall meet the requirements of ASTM E 329.
 - 2) Selection of a testing laboratory is subject to ENGINEER'S approval.
 - 3) Submit a written description of the proposed concrete testing laboratory giving qualifications of personnel, laboratory facilities and equipment, and other information that may be requested by ENGINEER.
3. Materials and installed Work may require testing and retesting, as directed by ENGINEER, at any time during the progress of the Work. Allow free access to material stockpiles and facilities at all times. Tests not specifically indicated to be done at OWNER'S expense, including the retesting of rejected materials and installed Work, shall be done at CONTRACTOR'S expense.

D. Qualifications of Water-Reducing Admixture Manufacturer:

1. Water-reducing admixtures shall be manufactured under strict quality control in facilities operated under a quality assurance program. Furnish copy of manufacturer's quality assurance handbook to document the existence of the program. Manufacturer shall maintain a concrete testing laboratory that has

been approved by the Cement and Concrete Reference Laboratory at the Bureau of Standards, Washington, D.C.

2. Provide a qualified concrete technician employed by the admixture manufacturer to assist in proportioning the concrete for optimum use of the admixture. The concrete technician shall advise on proper addition of the admixture to the concrete and on adjustment of the concrete mix proportions to meet changing jobsite conditions.

E. Laboratory Trial Batch:

1. Each concrete mix design specified shall be verified by a laboratory trial batch, unless indicated otherwise.
2. Each trial batch shall include the following testing:
 - a. Aggregate gradation for fine and coarse aggregates.
 - b. Fly ash testing to verify meeting specified properties, unless the fly ash supplier provides certification by an independent testing laboratory.
 - c. Slump.
 - d. Air content.
 - e. Compressive strength based on three cylinders each tested at seven days and at 28 days.
 - f. Shrinkage test as specified herein for Type "1" concrete mix design.
3. Each trial batch shall provide the following information:
 - a. Project identification name and number.
 - b. Date of report.
 - c. Complete identification of aggregate source of supply.
 - d. Tests of aggregates for compliance with specified requirements.
 - e. Scale weight of each aggregate.
 - f. Absorbed water in each aggregate.
 - g. Brand, type and composition of cement.
 - h. Brand, type and amount of each admixture.
 - i. Amounts of water used in trial mixes.
 - j. Proportions of each material per cubic yard.
 - k. Gross weight and yield per cubic yard of trial mixtures.
 - l. Measured slump.
 - m. Measured air content.
 - n. Compressive strength developed at seven days and 28 days, from not less than three test cylinders cast for each seven day and 28 day test, and for each design mix.
 - o. Shrinkage test results where required and as specified herein.
4. The requirement for a trial batch may be waived if the required test information has been provided in a previous laboratory trial batch run on the identical mix design within the previous two years. The same brand, type, and source of all materials must have been used.

F. Shrinkage Testing:

1. Shrinkage tests are required for mix design approval and for approval of concrete in the field.
 - a. Mix design shrinkage test:
 - 1) The submitted mix design proportions must be the same proportions used for the laboratory mix design shrinkage test and shall indicate concrete meets the specified shrinkage limit.
 - 2) Compressive strength test specimens shall be taken from each concrete sample used to prepare the shrinkage specimens. These compression test specimens shall be considered as part of the normal requirements for tests on this project.
 - 3) Drying shrinkage for laboratory cast specimens for mix design approval shall conform to ASTM C 157, modified as follows:
 - a) Curing of specimens:
 - i) Remove specimens from molds at an age of $23 \pm 1/2$ hours after the addition of water to the cement during the mixing operation.
 - ii) Upon removal of the specimens from the molds, immerse them in lime-saturated water maintained at 73 ± 1 degrees F for a minimum of 30 minutes.
 - iii) At an age of $24 \pm 1/2$ hours after the addition of water to the cement during the mixing operation, remove the specimens from water storage one at a time, wipe with a damp cloth, and immediately take the original length (not to be confused with “base length”).
 - iv) After measuring the original length, store the specimens in lime-saturated water at 73 ± 3 degrees F until they have reached an age of 7 days, including the period in the molds.
 - b) At the end of the curing period, take a second measurement after the specimens have been brought to a more closely controlled temperature as was done prior to the first measurement. This measurement shall be the base length for shrinkage calculations (“zero” days drying age) and shall be expressed as a percentage of original length (expansion).
 - c) Immediately store specimens in a temperature- and humidity-controlled room maintained at 73 ± 3 degrees F and 50 ± 4 percent relative humidity, for the remainder of the test. Specimens to have a clearance of at least 1 inch on all sides.
 - d) Measure to determine shrinkage, expressed as percentage of base length. Compute the drying shrinkage deformation of each specimen as the difference between the base length (at “zero” days drying age) and the length after drying at each test age. Compute the average drying shrinkage deformation of the specimens to the nearest 0.0001 inch at each test age. If the drying shrinkage of any specimen departs from the average of

- that test age by more than 0.0004 inch, disregard the results obtained from that specimen. Report results of shrinkage tests to the nearest 0.001 percent of shrinkage;
- e) Make and report shrinkage measurements separately for 7, 14, 21, and 28 days of drying after the 7-day curing period.
- b. Field shrinkage tests:
- 1) Field test shrinkage specimens shall be taken on the first concrete placement of each class of concrete.
 - 2) Compressive strength test specimens shall be taken from each concrete sample used to prepare the shrinkage specimens. These compression test specimens shall be considered as part of the normal requirements for tests on this project.
 - 3) If excessive cracking or other symptoms that are potentially related to shrinkage occur during construction, ENGINEER may require up to two (2) additional field shrinkage tests during construction to verify compliance with these Specifications.
 - 4) The initial field shrinkage testing will establish a baseline for the shrinkage of the specified mixes that represents the concrete as placed and cured in the field.
 - 5) The field cast specimens shall be transported to a facility that can provide temperature and humidity control prior to removing them from the molds.
 - 6) Drying shrinkage for specimens cast in the field shall conform to ASTM C 157, modified as follows:
 - a) Curing of specimens:
 - i) Remove specimens from molds at an age of $23 \pm 1/2$ hours after the addition of water to the cement during the mixing operation.
 - ii) Upon removal of the specimens from the molds, immerse them in lime-saturated water maintained at 73 ± 1 degrees F for a minimum of 30 minutes.
 - iii) At an age of $24 \pm 1/2$ hours after the addition of water to the cement during the mixing operation, remove the specimens from water storage one at a time, wipe with a damp cloth, and immediately take the original length (not to be confused with “base length”).
 - iv) After measuring the original length, continue curing the specimens, with the same curing methods used in the field, until they have reached an age of 14 days, including the period in the molds.
 - b) At the end of the curing period, store the specimens in a temperature-and humidity-controlled room maintained at 73 ± 3 degrees F and 50 ± 4 percent relative humidity, for the remainder of the test. Specimens to have a clearance of at least 1 inch on all sides.

- c) Take a second measurement immediately after the specimen's temperature has stabilized. This measurement shall be the base length for shrinkage calculations ("zero" days drying age) and shall be expressed as a percentage of original length (expansion).
 - d) Measure to determine shrinkage, expressed as percentage of base length. Compute the drying shrinkage deformation of each specimen as the difference between the base length (at "zero" days drying age) and the length after drying at each test age. Compute the average drying shrinkage deformation of the specimens to the nearest 0.0001 inch at each test age. If the drying shrinkage of any specimen departs from the average of that test age by more than 0.0004 inch, disregard the results obtained from that specimen. Report results of shrinkage tests to the nearest 0.001 percent of shrinkage;
 - e) Make and report shrinkage measurements separately for 7, 14, 21, and 28 days of drying after the 14-day curing period.
- 2. Shrinkage specimens shall be fabricated, cured, dried, and measured in accordance with ASTM C 157 and as modified herein. All specimens shall 11-1/4 inch $\pm$ 1/8 inch for an effective gauge length of 10 inches.
 - a. Shrinkage specimens for mixes with 100 percent of the aggregate passing the 1-inch sieve shall be 3 inches by 3 inches in cross section.
 - b. For mixes with aggregates larger than 1 inch and with 100 percent passing a 2-inch sieve, drying shrinkage specimens shall be 4 inches by 4 inches in cross section.
- 3. Report shrinkage test results at intervals as specified for the type of test, either laboratory or field, according to ASTM C 157 and the type of storage, water or air respectively.
 - a. The field test shrinkage shall not exceed the laboratory mix shrinkage requirement by more than 50 percent on the initial placement of each class of concrete required to be shrinkage tested.
 - b. If the required field shrinkage limitation is not met, all aspects of concrete mixing, placing, sampling and curing will be subject to review. If this review points toward a solution, participate in a solution of making changes as required to establish compliance. These changes may include, but are necessarily limited to the following:
 - 1) Changing the curing methods.
 - 2) Changing the source of aggregates.
 - 3) Changing the amount of crushed or fractured coarse aggregate.
 - 4) Changing cement quantity and/or admixtures.
 - 5) Use of a shrinkage reducing admixture.
 - 6) Reducing water/cement ratio.
 - 7) Making changes to sampling practices
 - 8) Possible consideration by the ENGINEER of modifying the field shrinkage acceptance criteria.

- 9) Other actions designed to minimize shrinkage and to provide better quality assurance.
 4. Once the initial field shrinkage testing requirement is met, subsequent shrinkage testing, done, if needed, during the remainder of the construction period shall use the field test result from each initial field shrinkage test as a base line for acceptance.
 - a. If the base line acceptance criteria are not met, then the provisions of Article 1.2 F.3.b above shall be enacted to re-establish compliance.
 5. These tests shall be considered a part of the normal compression tests for the project. Allowable shrinkage limitations shall be as specified in Part 2, herein.
- G. Certification of Concrete Mix:
1. The need for a trial batch may be waived if the following requirements are met. The compressive strength of each specified mix shall be verified by data from a series of a minimum of 30 consecutive tests that have been made within the previous 12 months. A test is defined as the average strength of all specimens of the same age fabricated from a sample taken from a single batch of concrete. All tests shall have been made on concrete which is identical in mix design to the submitted proposed mix design, including sources of aggregate and manufacturers of cementitious materials and admixtures. The tests must average above the specified strength with no individual test falling more than 500 psi below specified strength and no three consecutive tests averaging below specified strength. In addition, the standard deviation for the series of tests shall not exceed 640 psi as defined by ACI 214.

1.3 SUBMITTALS

- A. Samples: Submit samples of materials as specified and as otherwise may be requested by ENGINEER, including names, sources and descriptions.
- B. Shop Drawings: Submit for approval the following:
 1. Comply with the requirements of Section 01332, Shop Drawing Procedures.
 2. Manufacturer's specifications with application and installation instructions for proprietary materials and items, including admixtures and bonding agents.
 3. List of concrete materials and concrete mix designs proposed for use. Include the results of all tests performed to qualify the materials and to establish the mix designs.
 4. The following information, if ready-mixed concrete is used.
 - a. Physical capacity of mixing plant.
 - b. Trucking facilities available.
 - c. Estimated average amount that can be produced and delivered to the site during a normal eight hour day, excluding the output to other customers.
 - d. Each Shop Drawing Submittal shall include a hard copy of the relevant Specification Section and shall be clearly marked to indicate whether the requirements for equipment and/or services in the Specification Section are

met by writing "accept" or "deviate" next to each Paragraph. If clarifications are needed to any of the Paragraphs in the Specification Sections due to deviations, they shall be addressed next to the Paragraph as such and explained further with any additional information necessary. If any exceptions and/or deviations are proposed to any of the Specifications, they shall be clearly noted as such in the Submittal, and an explanation of any deviation and/or exception shall be provided. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected.

5. Refer to and comply with the requirements of Section 01332, Shop Drawing Procedures.
 6. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures.
 7. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.
- C. Laboratory Test Reports: Submit copies of laboratory test reports for concrete cylinders, materials and mix design tests. ENGINEER'S review will be for general information only. Production of concrete to comply with specified requirements is the responsibility of CONTRACTOR.
- D. Submit notarized certification of conformance to referenced standards when requested by ENGINEER.
- E. Certification of coarse aggregate impurities as relates to alkali-silica reactivity per ASTM C33, as supplemented by ASTM C1778.
- F. Certification that the aggregates to be supplied for the Project are from the same location as the aggregates represented in the following concrete mix design submittal documents:

1. Compressive strength test reports.
 2. Certification document for the class of coarse aggregate meeting the requirements of ASTM C33 and project requirements for type and location of concrete construction.
 3. Shrinkage test reports.
- G. Certification from synthetic macrofiber manufacturer indicating that proposed fiber dosage meets or exceeds the minimum specified f_{e3} values in accordance with ASTM C1609.
- H. Delivery Tickets: Furnish to ENGINEER copies of all delivery tickets for each load of concrete delivered to the site. Provide items of information as specified in ASTM C 94, Section 16.1.
1. Provide batch tickets for each batch of job-site mixed concrete, as specified.

1.4 PRODUCT DELIVERY, STORAGE AND HANDLING

- A. All materials used for concrete must be kept clean and free from all foreign matter during transportation and handling and kept separate until measured and placed in the mixer. Bins or platforms having hard clean surfaces shall be provided for storage. Suitable means shall be taken during hauling, piling and handling to ensure that segregation of the coarse and fine aggregate particles does not occur and the grading is not affected.

1.5 CONCRETE COORDINATION MEETING

- A. A Concrete Coordination Meeting shall be held to review the detailed requirements of CONTRACTOR'S proposed concrete design mixes, to determine the procedures for producing proper concrete construction, and to clarify the roles of the parties involved shall be held no later than 14 days after the Notice to Proceed.
- B. All parties involved in the concrete Work shall attend the conference, including but not limited to the following:
1. CONTRACTOR'S representative.
 2. Testing laboratory representative.
 3. Concrete subcontractor.
 4. Reinforcing steel subcontractor and detailer.
 5. Concrete supplier.
 6. Admixture manufacturer's representative.
 7. ENGINEER.
- C. The Concrete Coordination Meeting shall be held at a mutually agreed upon time and place. The ENGINEER shall be notified no less than five days prior to the date of the Concrete Coordination Meeting.

PART 2 - PRODUCTS

2.1 GENERAL

- A. All admixtures, curing compounds, etc. used in concrete or the curing and repair of concrete, which can contact potable water, shall be certified as conforming to the requirements of ANSI/NSF 61 for contact with potable water when in the finished concrete.
- B. Maximum total chloride ion content for concrete mix including all ingredients measured as a weight percent of cement in accordance with ASTM C1218:
 - 1. Prestressed concrete: 0.06.
 - 2. All other concrete: 0.10.

2.2 CEMENTITIOUS MATERIALS

- A. Cement:
 - 1. Portland cement, ASTM C 150, Type II or V.
 - 2. Use Portland cement made by a well-known acceptable manufacturer and produced by not more than one plant. Alternate cement sources may be used provided that a mix design has been accepted and a trial batch verifying performance has been made.
 - 3. Do not use cement which has deteriorated because of improper storage or handling.
- B. Fly Ash Mineral Admixture:
 - 1. Mineral admixtures, when used, shall meet the requirements of ASTM C 618 Class F, except as follows:
 - a. The loss on ignition shall be a maximum of 4 percent.
 - b. The maximum percent of sulfur trioxide (SO₃) shall be 4.0.
 - 2. Fly ash shall be considered to be a cementitious material.
 - 3. Laboratory trial batches shall be tested to determine compliance with strength requirements, times of setting, slump, slump loss, and shrinkage characteristics.
 - 4. A substitution by weight, of the portland cement by fly ash, so that the total tricalcium aluminate content of the resulting cement plus fly ash is not greater than eight percent, will be considered. However, the fly ash shall not exceed 25 percent by weight of the cement plus fly ash.

2.3 AGGREGATES

- A. General:
 - 1. Aggregates shall conform to the requirements of ASTM C 33 and as herein specified.
 - 2. Do not use aggregates containing soluble salts or other substances such as iron sulfides, pyrite, marcasite, ochre, or other materials that can cause stains on exposed concrete surfaces.

3. Do not use aggregates that may be deleteriously reactive when combined with alkalis in cement.
 - a. Evaluate proposed aggregates for potential deleterious expansion due to alkali silica reactivity per ASTM C33, ASTM C227, ASTM C1260, ASTM 1293, or ASTM C1567.
- B. Fine Aggregate: Clean, sharp, natural sand free from loam, clay, lumps or other deleterious substances.
 1. Dune sand, bank run sand and manufactured sand are not acceptable.
- C. Coarse Aggregate: Clean, uncoated, processed aggregate containing no clay, mud, loam, or foreign matter, as follows:
 1. Crushed stone, processed from natural rock or stone.
 2. Washed gravel, either natural or crushed. Use of slag and pit or bank run gravel is not permitted.
 3. Lightweight Aggregate: ASTM C 330.

2.4 WATER

- A. Water used in the production and curing of concrete shall be clean and free from injurious amounts of oils, acids, alkalis, organic materials or other substances that may be deleterious to concrete or steel.

2.5 CONCRETE ADMIXTURES

- A. Provide admixtures produced by established reputable manufacturers, and use in compliance with the manufacturer's printed instructions. All admixtures shall be compatible and by a single manufacturer capable of providing qualified field service representation. Admixtures shall not contain thiocyanates nor more than 0.05 percent chloride ion, and shall be non-toxic in the concrete mix after 30 days. Do not use admixtures that have not been incorporated and tested in the accepted mixes, unless otherwise authorized in writing by ENGINEER.
- B. Air-Entraining Admixtures: Provide air entraining admixtures complying with ASTM C 260, and that are vinsol resin or vinsol resin based.
 1. Product and Manufacturer: Provide one of the following:
 - a. SIKA AER or SIKA AEA-15, as manufactured by Sika Corporation.
 - b. MB-VR, as manufactured by BASF.
 - c. Daravair 1000, as manufactured by Grace Construction Products.
 - d. Or equal.
- C. Water-Reducing Admixture: ASTM C 494, Type A.
 1. Proportion all Type "1" "2" and "3" concrete with non-air entraining, normal setting, water-reducing, aqueous solution of a modification of the salt of polyhydroxylated organic acids. The admixture shall not contain any lignin, nitrates or chlorides added during manufacture.

2. Product and Manufacturer: Provide one of the following:
 - a. Eucon WR-75, as manufactured by Euclid Chemical Company.
 - b. Pozzoloth series, as manufactured by Master Builders, Inc.
 - c. WRDA-15, as manufactured by W.R. Grace & Company.
 - d. Plastocrete 161 or Plastiment NS, as manufactured by Sika Corporation.
 - e. Or equal.
- D. High Range Water-Reducing Admixture (HRWR): ASTM C 494, Type F/G.
 1. High range water-reducer shall be used in classifications of concrete, where specified, and shall be permitted, at CONTRACTOR'S option, in all other classifications of concrete. It shall be added to concrete in compliance with the manufacturer's printed instructions. The specific admixture formulation shall be as recommended by the manufacturer for the project conditions. Provide one of the following:
 - a. Sikament series, as manufactured by Sika Corporation.
 - b. Rheobuild series, as manufactured by Master Builders, Inc.
 - c. Daracem-100, as manufactured by W.R. Grace & Company.
 - d. Eucon 37 or Eucon 537, as manufactured by the Euclid Chemical Company.
 - e. Or equal.
- E. Set-Control Admixtures: ASTM C 494, as follows:
 1. Type B, Retarding.
 2. Type C, Accelerating.
 3. Type D, Water-reducing and Retarding.
 4. Type E, Water-reducing and Accelerating.
 5. Type F, Water-reducing, high range admixtures.
 6. Type G, Water-reducing, high range, and retarding admixtures.
- F. Calcium Chloride: Calcium chloride shall not be used.
- G. Shrinkage Reducing Admixture:
 1. A shrinkage reducing admixture shall be permitted to be used in the mix design where necessary to meet specified shrinkage limitations provided that specified strength requirements are met and there is no reduction in sulfate resistance and no increase in permeability.
 2. Conform to ASTM C494, Type S.
 3. Shrinkage reducing admixtures shall be one of the following:
 - a. Eclipse, as manufactured by Grace Construction Products.
 - b. Tetraguard AS20, as manufactured by Master Builders, Inc.
 - c. Or equal.
- H. Synthetic Macrofibers:
 1. Conform to ASTM C1116, Type III
 - a. Minimum length: 1.5 IN

- b. Aspect ratio: 50 to 90.
 2. Acceptable manufacturers:
 - a. MasterFiber MAC Series by BASF Corporation.
 - b. Strux 90/40 by GCP Applied Technologies, Inc.
 - c. Tuf-Strand SF by Euclid Chemical Company.
 - d. Sika Fibermesh-650 by Sika Corporation.
- I. If super plasticizers are used in mix designs, the mix shall be slumped at jobsite prior to addition of plasticizer.

2.6 PROPORTIONING AND DESIGN OF MIXES

- A. Prepare concrete design mixes subject to the following limitations:

Classification	Coarse Aggregate	Min. Cementitious (lbs/cu yd)	Max W/C	Slump	Air	Min. Comp. Strength (psi)
Type "1"	#67	564	0.42	4"	4 ½	4500
Type "2"	#8	517	0.50	6"	1 ½	3000
Type "3"	#67	611	0.42	4"	4 ½	5000
Type "4"	Any ASTM C33	No requirements				150

1. The slumps listed are prior to the addition of high range water reducer (super plasticizer).
 2. Mix designs shall be made for all but Type "4", which does not require a trial batch, so that the compressive strength achieved for the laboratory trial batches will be no less than 125 percent of the specified design strength. This is to assure meeting the design strength for all concrete batched during the project.
 3. The quantity of water to be used in the determination of the water-cementitious materials ratio shall include free water on aggregates in excess of SSD and the water portion of admixtures.
 4. Allowable tolerance for air content at the point of delivery is +/- 1.5 percent.
- B. Use an independent testing facility acceptable to ENGINEER for preparing and reporting proposed mix designs.
 1. The testing facility shall not be the same as used for field quality control testing.
- C. Submit written reports of laboratory trial batch test results for proposed mixes of concrete to ENGINEER at least 15 days prior to start of Work. Do not begin concrete production until mixes have been approved by ENGINEER.
- D. Adjustment to Concrete Mixes: Mix design adjustments may be requested by CONTRACTOR when characteristics of materials, job conditions, weather, test

- results, or other circumstances warrant; at no additional cost to the OWNER and as accepted by ENGINEER. Laboratory test data for revised mix designs and strength results must be submitted to and accepted by ENGINEER before using the revised mixes.
- E. Admixtures:
1. Use air-entraining admixture in all concrete, unless otherwise shown or specified. Add air-entraining admixture at the manufacturer's prescribed rate to result in concrete at the point of placement having air content within the prescribed limits.
 2. Use amounts of admixtures as recommended by the manufacturer for climatic conditions prevailing at the time of placing. Adjust quantities and types of admixtures as required to maintain quality control.
- F. Slump Limits with High Range Water Reducer:
1. Slump shall not exceed 4-inches prior to adding high range water-reducer and shall not exceed 8 inches, measured at point of placement, after adding high range water reducer.
- G. Synthetic Macrofibers:
1. Dosage:
 - a. All concrete members:
 - 1) Fiber dosage shall provide a minimum fiber required residual strength (f_{e3}) value, as follows, in accordance with ASTM C1609:
 - a) 10 IN thick element: $f_{e3} = 170$ PSI
 - b) Over 10 IN thick element: $f_{e3} = 150$ PSI
 - 2) Under no circumstance shall dosage be less than 3 LBS per cubic yard
 - b. Final dosage rates to be determined by Contractor and concrete supplier as required to meet the specified minimum average residual strength.
 2. Provide synthetic macrofibers in the following concrete members:
 - a. Mat foundations of water bearing structures
 - b. Walls of water bearing structures
 - c. Elevated concrete slabs and beams of water bearing structures
 - d. Other locations as specified on the Drawings
 3. Uniformly disperse in concrete mixtures.
- H. Shrinkage Limitation:
1. The maximum concrete shrinkage for specimens cast in the laboratory from the trial batch, as measured at 28-day drying age shall be 0.032 percent. Only use a mix design for construction that has first met the trial batch shrinkage requirements. Shrinkage limitations apply only to Type "1" concrete.
- I. Color: Provide colored concrete where shown on the Drawings and specified. Incorporate pigments into the concrete mix according to manufacturer's written instructions. Match sample color approved by ENGINEER.

2.7 BONDING AGENT

- A. Provide epoxy and epoxy-cement bonding agents as specified in Section 03251, Concrete Joints.

2.8 CONCRETE CURING MATERIALS

- A. Absorptive Cover: Burlap cloth made from jute or kenaf, weighing approximately 10 ounces per square yard and complying with AASHTO M 182, Class 3.
- B. Curing Mats: Curing mats shall be heavy carpets or cotton mats, quilted at 4-inches on center. Curing mats shall weigh a minimum of 12 ounces per square yard when dry.
- C. Moisture Retaining Cover: One of the following, complying with ASTM C 171.
 - 1. Waterproof paper.
 - 2. Polyethylene film.
 - 3. White burlap-polyethylene sheet.
- D. Curing Compound: ASTM C 309 Type 1-D (water retention requirements):
 - 1. Product and Manufacturer: Provide one of the following:
 - a. Super Aqua Cure VOX, as manufactured by The Euclid Chemical Company.
 - b. 1100 Clear, as manufactured by W.R. Meadows, Incorporated.
 - c. Atlas Res-Cure, as manufactured by Atlas Tech Products.
 - d. Or equal.
 - 2. Provide fugitive dye.
 - 3. Curing compound must be applied by roller or power sprayer.

2.9 FINISHING AIDS

- A. Evaporation Retardant:
 - 1. Product and Manufacturer: Provide one of the following:
 - a. Confilm, as manufactured by BASF.
 - b. Eucobar, as manufactured by Euclid Chemical Company.
 - c. SikaFilm by Sika Corporation.
 - d. Or equal.

2.10 CONCRETE REPAIR MATERIALS

- A. Concrete repair mortar shall be a prepackaged polymer-modified cementitious repair mortar with the following minimum properties:
 - 1. Compressive strength at one day: 2000 psi (ASTM C 109).
 - 2. Compressive strength at 28 days: 7000 psi (ASTM C 109).
 - 3. Bond strength at 28 days: 1800 psi (ASTM C 882 modified).

- B. Concrete repair mortar shall be:
1. Five Star Structural Concrete, manufactured by Five Star Products, Inc. The formulation recommended by the manufacturer for the specific application conditions shall be used.
 2. SikaTop 122 Plus, SikaTop 123 Plus, SikaTop 111 Plus, or Sikacem 133, manufactured by the Sika Corporation. The formulation, among those listed, recommended by the manufacturer for the specific application conditions shall be used.
 3. Emaco S88-CA or S66-CR, manufactured by Master Builders Inc. The formulation, among those listed, recommended by the manufacturer for the specific application conditions shall be used.
 4. Verticoat, Verticoat Supreme, or Euco SR-VO, manufactured by the Euclid Chemical Company. The formulation, among those listed, recommended by the manufacturer for the specific application conditions shall be used.
 5. Or equal.
- C. Cement Mortar: Cement mortar shall consist of a mix of one part cement to 1 1/2 parts sand with sufficient water to form a trowelable consistency. Minimum compressive strength at 28 days shall be 4000 psi. Where required to match the color of adjacent concrete surfaces, white portland cement shall be blended with standard portland cement so that, when dry, the patching mortar shall match the color of the surrounding concrete.

PART 3 - EXECUTION

3.1 INSPECTION

- A. Examine the substrate and the conditions under which Work is to be performed and notify ENGINEER, in writing, of unsatisfactory conditions. Do not proceed with the Work until unsatisfactory conditions have been corrected in a manner acceptable to ENGINEER.

3.2 CONCRETE MIXING

- A. General:
1. Concrete may be produced at batch plants or it may be produced by the ready-mixed process. Batch plants shall comply with the recommendations of ACI 304, and shall have sufficient capacity to produce concrete of the qualities specified, in quantities required to meet the construction schedule. All plant facilities are subject to testing laboratory inspection and acceptance of ENGINEER.
- B. Ready-Mix Concrete:

1. Comply with the requirements of ASTM C 94, and as herein specified. Proposed changes in mixing procedures, other than herein specified, must be accepted by ENGINEER before implementation.
 - a. Plant equipment and facilities: Conform to National Ready-Mix Concrete Association "Plant and Delivery Equipment Specification".
 - b. Mix concrete in revolving type truck mixers that are in good condition and which produce thoroughly mixed concrete of the specified consistency and strength.
 - c. Do not exceed the proper capacity of the mixer.
 - d. Mix concrete for a minimum of two minutes after arrival at the job site, or as recommended by the mixer manufacturer.
 - e. Do not allow the drum to mix while in transit.
 - f. Mix at proper speed until concrete is discharged.
 - g. Maintain adequate facilities at the job site for continuous delivery of concrete at the required rates.
 - h. Provide access to the mixing plant for ENGINEER at all times.
 2. When silica fume is used in the dry compacted form, the following mix requirements shall be followed to ensure full dispersion.
 - a. For all types of mixing equipment, mix times shall be increased by 40 percent over the minimum mix time required to achieve mix uniformity as defined by ASTM C 94.
 - b. For truck-mixed and central mixed concrete, maximum allowable batch size shall be 80 percent of the maximum in accordance with ASTM C 94.
- C. Maintain equipment in proper operating condition, with drums cleaned before charging each batch. Schedule rates of delivery in order to prevent delay of placing the concrete after mixing, or holding dry-mixed materials too long in the mixer before the addition of water and admixtures.

3.3 TRANSPORTING CONCRETE

- A. Transport and place concrete not more than 90 minutes after water has been added to the dry ingredients.
- B. Take care to avoid spilling and separation of the mixture during transportation.
- C. Do not place concrete in which the ingredients have been separated.
- D. Do not retemper partially set concrete.
- E. Use suitable and approved equipment for transporting concrete from mixer to forms.

3.4 PREPARTION FOR CONCRETING

- A. All reinforcement, installation of waterstop and positioning of embedded items shall be inspected and approved by the ENGINEER a minimum of four hours prior to concrete placement.
- B. Subgrade surfaces shall be thoroughly wetted by sprinkling, prior to the placing of any concrete, and these surfaces shall be kept moist by frequent sprinkling up to the time of placing concrete thereon. The surface shall be free from standing water, mud, and debris at the time of placing concrete.
- C. All reinforcing steel and embedded items shall be completely cleaned of mortar, loose rust, form release compounds, dirt, or any other substance which would interfere with proper bonding with concrete. Protective coatings on embedded aluminum items shall continuously cover the surface to be in contact with concrete. Any defects in the coating shall be repaired.
- D. No concrete shall be placed in any structure until all water entering the space to be filled with concrete has been properly cut off or has been diverted by pipes, or other means, and carried out of the forms, clear of the work. No concrete shall be deposited underwater nor shall CONTRACTOR allow still water to rise on any concrete until the concrete has attained its initial set. Water shall not be permitted to flow over the surface of any concrete in such manner and at such velocity as will injure the surface finish of the concrete. Pumping or other necessary dewatering operations for removing ground water, if required, will be subject to the review of the ENGINEER.
- E. Joint surfaces shall be prepared as required by Section 03251, Concrete Joints.

3.5 CONCRETE PLACEMENT

- A. General: Place concrete continuously so that no concrete will be placed on concrete that has hardened sufficiently to cause the formation of seams or planes of weakness within the section. If a section cannot be placed continuously, provide construction joints as specified in Section 03251, Concrete Joints. Deposit concrete as nearly as practical in its final location to avoid segregation due to rehandling or flowing. Do not subject concrete to any procedure which will cause segregation.
 - 1. Screed concrete that is to receive other construction to the proper level to avoid excessive skimming or grouting.
 - 2. Do not use concrete which becomes non-plastic and unworkable, or does not meet the required quality control limits, or which has been contaminated by foreign materials. Do not use retempered concrete. Remove rejected concrete from the job site and dispose of it in an acceptable location.
 - 3. Do not place concrete until all forms, bracing, reinforcement, and embedded items are in final and secure position.

4. Unless otherwise approved, place concrete only when ENGINEER is present.
 5. Allow a minimum of three days before placing concrete against a slab or wall already in place.
- B. Bonding for Next Concrete Pour:
1. Prepare for bonding of fresh concrete to new concrete that has set but is not fully cured, as follows:
 - a. Thoroughly wet the surface, but allow no free standing water.
 - b. For horizontal surfaces place a 2-inch layer of mortar, one part sand and one part cement with water added to a flowable consistency, or a 6-inch layer of Construction Joint Grout, as specified in Section 03600, Grout, over the hardened concrete surface.
 - c. Place fresh concrete before the mortar/grout has attained its initial set.
 - d. If a high range water reducer is used to increase the concrete slump to at least 6-inches, the mortar/grout layer may be omitted.
 2. Bonding of fresh concrete to fully-cured hardened existing concrete shall be accomplished by using a bonding agent as specified in Section 03251, Concrete Joints.
- C. Concrete Conveying:
1. Handle concrete from the point of delivery and transfer to the concrete conveying equipment and to the locations of final deposit as rapidly as practical by methods that will prevent segregation and loss of concrete mix materials.
 2. Provide mechanical equipment for conveying concrete to ensure a continuous flow of concrete at the delivery end. Provide runways for wheeled concrete conveying equipment from the concrete delivery point to the locations of final deposit. Keep interior surfaces of conveying equipment, including chutes, free of hardened concrete, debris, water, ice and other deleterious materials.
 3. Do not use chutes for distributing concrete, unless approved in writing by ENGINEER.
 - a. Provide sketches showing methods by which chutes will be employed when requesting such approval.
 - b. Design chutes, if permitted, with proper slopes and supports to permit efficient handling of the concrete.
 4. Pumping concrete is permitted, however do not use aluminum pipe for conveying.
- D. Placing Concrete into Forms:
1. Deposit concrete in forms in horizontal layers not deeper than 18-inches and in a manner to avoid inclined construction joints. Where placement consists of several layers, place concrete at such a rate that concrete that is being integrated with fresh concrete is still plastic.
 2. Do not permit concrete to free fall within the form from a distance exceeding four feet. Where high range water reducer is used to extend slump to at least 6-inches, the maximum free fall of concrete may be increased to six feet. If a 12-

inch thick layer of construction joint grout, as specified in Section 03251, Concrete Joints, is placed on the horizontal joint, concrete with slump extended by a high range water reducer may free fall up to eight feet in walls that are 24-inches and thicker. Use "elephant trunks" to prevent free fall and excessive splashing on forms and reinforcement. Free falls in excess of four feet shall be discontinued if there is any evidence of segregation.

3. Remove temporary spreaders in forms when concrete placing has reached the elevation of such spreaders.
4. Consolidate concrete placed in forms by mechanical vibrating equipment supplemented by hand-spading, rodding or tamping. Use equipment and procedures for consolidation of concrete in accordance with the applicable recommended practices of ACI 309. Vibration of forms and reinforcing will not be permitted, unless otherwise accepted by ENGINEER.
5. Where height of concrete placement in walls exceeds 14 feet, temporary windows shall be installed in the formwork to facilitate vibration. The windows shall be properly closed when the height of concrete approaches the windows. Location, size, and spacing of the windows shall be determined by CONTRACTOR to suit equipment used.
6. Do not use vibrators to transport concrete inside of forms. Insert and withdraw vibrators vertically at uniformly spaced locations not farther than the visible effectiveness of the machine. Place vibrators to rapidly penetrate the layer of concrete and at least 6-inches into the preceding layer. Do not insert vibrators into lower layers of concrete that have begun to set. At each insertion, limit the duration of vibration to the time necessary to consolidate the concrete and complete embedment of reinforcement and other embedded items without causing segregation of the mix.
7. Do not place concrete in beam and slab forms until the concrete previously placed in columns and walls is no longer plastic.
8. Force concrete under pipes, sleeves, openings and inserts from one side until visible from the other side to prevent voids.

E. Placing Concrete Slabs:

1. Deposit and consolidate concrete slabs in a continuous operation, within the limits of construction joints, until the placing of a panel or section is completed.
2. Consolidate concrete during placing operations using mechanical vibrating equipment, so that concrete is thoroughly worked around reinforcement and other embedded items and into corners.
3. Consolidate concrete placed in beams and girders of supported slabs, and against bulkheads of slabs on ground, as specified for formed concrete structures.
4. Bring slab surfaces to the correct level. Smooth the surface, leaving it free of humps or hollows. Do not sprinkle water on the plastic surface. Do not disturb the slab surfaces prior to beginning finishing operations.
5. Where slabs are placed in conditions of high temperature or wind that could lead to formation of plastic shrinkage cracks, an evaporation retardant shall be

applied in accordance with the manufacturer's recommendations, when required by the ENGINEER.

F. Quality of Concrete Work:

1. Make all concrete solid, compact and smooth, and free of laitance, cracks and cold joints.
2. All concrete for liquid retaining structures, and all concrete in contact with earth, water, or exposed directly to the elements shall be watertight.
3. Cut out and properly replace to the extent directed by ENGINEER, or repair to the satisfaction of ENGINEER, surfaces which contain cracks or voids, are unduly rough, or are in any way defective. Thin patches or plastering shall not be acceptable.
4. All leaks through concrete that exhibit any flowing water, and cracks, holes or other defective concrete in areas of potential leakage, shall be repaired and made watertight by CONTRACTOR.
5. Repair, removal, and replacement of defective concrete as directed by ENGINEER shall be at no additional cost to the OWNER.

G. Cold Weather Placing:

1. Protect all concrete Work from physical damage or reduced strength that could be caused by frost, freezing actions, or low temperatures, in compliance with the requirements of ACI 306 and as herein specified.
2. When the air temperature has fallen to or may be expected to fall below 40°F, provide adequate means to maintain the temperature, in the area where concrete is being placed, at between 50°F and 70°F for at least seven days after placing. Provide temporary housings or coverings including tarpaulins or plastic film. Maintain the heat and protection, if necessary, to ensure that the ambient temperature does not fall more than 30°F in the 24 hours following the seven-day period. Avoid rapid dry-out of concrete due to overheating, and avoid thermal shock due to sudden cooling or heating.
3. When air temperature has fallen to or is expected to fall below 40°F, uniformly heat all water and aggregates before mixing as required to obtain a concrete mixture temperature of not less than 55°F and not more than 85°F at point of placement.
4. Do not use salt and other materials containing antifreeze agents or chemical accelerators, or set-control admixtures, unless approved by ENGINEER, in mix designs.

H. Hot Weather Placing:

1. When hot weather conditions exist that would seriously impair the quality and strength of concrete, place concrete in compliance with ACI 305 and as herein specified.
2. When ambient air temperature is at or above 90°F, cool ingredients before mixing to maintain concrete temperature at time of placement below 80°F when the air temperature is rising and below 85°F when the air temperature is falling.

Mixing water may be chilled, or chopped ice may be used to control the concrete temperature provided the water equivalent of the ice is calculated in the total amount of mixing water. In addition, the reduction in time from addition of mix water to placement or the use of a set retarding admixture may be required.

3. Cover reinforcing steel with water-soaked burlap if it becomes too hot, so that the steel temperature will not exceed the ambient air temperature immediately before embedment in concrete.
4. Wet forms thoroughly before placing concrete.
5. Do not place concrete at a temperature so as to cause difficulty from loss of slump, flash set, or cold joints.
6. Do not use set-control admixtures, unless approved by ENGINEER in mix designs.
7. Obtain ENGINEER'S approval of other methods and materials proposed for use.

3.6 FINISH OF FORMED SURFACES

A. Standard Form Finish:

1. Standard form finish shall be basically smooth and even but shall be permitted to have texture imparted by the form material used. Defects shall be repaired as specified herein.
2. Use standard form finish for the following:
 - a. Exterior vertical surfaces from the foundation up to one foot below grade.
 - b. Vertical surfaces not exposed to view.
 - c. Other areas shown.

B. Smooth Form Finish:

1. Produce smooth form finish by selecting form materials that will impart a smooth, hard, uniform texture. Arrange panels in an orderly and symmetrical manner with a minimum of seams. Repair and patch defective areas as specified herein.
2. Use smooth form finish for the following:
 - a. Exterior surfaces that are exposed to view.
 - b. Surfaces that are to be covered with a coating material. The material may be applied directly to the concrete or may be a covering bonded to the concrete such as waterproofing, dampproofing, painting or other similar system.
 - c. Interior vertical surfaces of liquid containers.
 - d. Interior and exterior exposed beams and undersides of slabs.
 - e. Surfaces to receive an abrasive blasted finish.
 - f. Surfaces to receive a smooth rubbed or grout cleaned finish.
 - g. Other areas shown.

C. Smooth Rubbed Finish:

1. Provide smooth, Class A, rubbed finish to concrete surfaces, which have received smooth form finish and where all defects have been repaired, as follows:
 - a. Rubbing of concrete surfaces not later than the day after form removal.
 - b. Moistening of concrete surfaces and rubbing with carborundum brick or other abrasive until a uniform color and texture is produced. Do not apply cement grout other than that created by the rubbing process.
2. Except where surfaces have been previously covered as specified above, use smooth, Class A, rubbed finish for the following:
 - a. Interior exposed walls and other vertical surfaces.
 - b. Exterior exposed walls and other vertical surfaces down to one foot below grade.
 - c. Interior and exterior horizontal surfaces, except exterior exposed slabs and steps.
 - d. Interior exposed vertical surfaces of liquid containers down to one foot below liquid level.
 - e. Other areas shown on the Drawings.

D. Grout Cleaned Finish:

1. Provide grout cleaned finish to concrete surfaces, which have received smooth form finish and where all defects have been repaired, as follows:
 - a. Combine one part portland cement to 1-1/2 parts fine sand by volume, and mix with water to the consistency of thick paint. Blend standard portland cement and white portland cement, amounts determined by trial patches, so that the final color of dry grout will closely match adjacent concrete surfaces.
 - b. Thoroughly wet the concrete surface and apply grout uniformly by brushing or spraying immediately to the wetted surfaces. Scrub surface with cork float or stone to coat surface and fill surface holes. Remove excess grout by scraping, followed by rubbing with clean burlap to remove any visible grout film. Keep grout damp during the setting period by means of fog spray at least 36 hours after final rubbing. Complete any area in the same day it is started, with the limits of any area being natural breaks in the finished surface.
2. Except where surfaces have been previously covered as specified above, use grout cleaned finish for the following:
 - a. Interior exposed walls and other vertical surfaces.
 - b. Exterior exposed walls and other vertical surfaces down to one foot below grade.
 - c. Interior and exterior horizontal surfaces, except exterior exposed slabs and steps.
 - d. Interior exposed vertical surfaces of liquid containers down to one foot below liquid level.
 - e. Other areas shown on the Drawings.

E. Abrasive Blasted Finish:

1. Provide abrasive blasted finish where shown.
2. Where abrasive blasted finish is indicated, it shall be applied to a smooth formed finish after the end of the curing period, with all defects repaired, to match the approved finish provided on the mock-up panel.
3. Heavy Abrasive Blasted Finish: Abrasive blast to uniformly expose coarse aggregate.
4. Light Abrasive Blasted Finish: Abrasive blast to uniformly expose fine aggregate.

F. Related Unformed Surfaces:

1. At tops of walls, horizontal offsets, and similar unformed surfaces occurring adjacent to formed surfaces, strike off smooth and finish with a texture matching the adjacent formed surfaces. Continue the final surface treatment of formed surfaces uniformly across the adjacent unformed surfaces, unless otherwise shown.

3.7 SLAB FINISHES

A. Float Finish:

1. After placing concrete slabs, do not work the surface further until ready for floating. Begin floating when the surface water has disappeared or when the concrete has stiffened sufficiently. Check and level the surface plane to a tolerance not exceeding 1/4-inch in ten feet when tested with a ten foot straightedge placed on the surface at not less than two different angles. Cut down high spots and fill all low spots. Uniformly slope surfaces to drains. Immediately after leveling, refloat the surface to a uniform, smooth, granular texture.
2. Use float finish for the following:
 - a. Interior exposed horizontal surfaces of liquid containers, except those to receive grout topping.
 - b. Exterior below grade horizontal surfaces.
 - c. Surfaces to receive additional finishes, except as shown or specified.

B. Trowel Finish:

1. After floating, begin the first trowel finish operation using a power-driven trowel. Begin final troweling when the surface produces a ringing sound as the trowel is moved over the surface.
2. Consolidate the concrete surface by the final hand troweling operation. Finish shall be free of trowel marks, uniform in texture and appearance, and with a surface plane tolerance not exceeding 1/8-inch in ten feet when tested with a ten foot straight edge. Grind smooth surface defects that would telegraph through applied floor covering system.
3. Use trowel finish for the following:
 - a. Interior exposed slabs, unless otherwise shown or specified.

b. Slabs to receive resilient floor finishes.

C. Non-Slip Broom Finish:

1. Immediately after float finishing, slightly roughen the concrete surface by brooming in the direction perpendicular to the main traffic route. Use fine fiber-bristle broom, unless otherwise directed by the ENGINEER. Coordinate the required final finish with ENGINEER before application.
2. Use Non-Slip Broom Finish for the following:
 - a. Exterior exposed horizontal surfaces subject to light foot traffic.
 - b. Interior and exterior concrete steps and ramps.
 - c. Horizontal surfaces which will receive a grout topping or a concrete equipment base slab.

3.8 CONCRETE CURING AND PROTECTION

A. General:

1. Protect freshly placed concrete from premature drying and excessive cold or hot temperature, and maintain without drying at a relatively constant temperature for the period of time necessary for hydration of the cement and proper hardening of the concrete.
2. Start initial curing after placing and finishing concrete as soon as free moisture has disappeared from the concrete surface. Keep continuously moist for not less than 72 hours.
3. Begin final curing procedures immediately following initial curing and before the concrete has dried. Continue final curing for at least seven days and in accordance with ACI 301 procedures for a total curing period, initial plus final, of at least ten days. For concrete sections over 30-inches thick, continue final curing for an additional seven days, minimum. Avoid rapid drying at the end of the final curing period.

B. Curing Methods:

1. Water retaining and below grade structures shall be moist cured by the addition of water to maintain the surface in a continually wet condition. Other concrete shall be cured by moist curing, by moisture retaining cover curing, or by the use of curing compound. Use curing compound at water retaining and below grade structures only in cold weather and only when permitted by ENGINEER.
 - a. For curing, use water that is free of impurities that could etch or discolor exposed, natural concrete surfaces.
2. Provide moisture curing by any of the following methods:
 - a. Keeping the surface of the concrete continuously wet by covering with water.
 - b. Continuous water-fog spray.
 - c. Covering the concrete surface with curing mats, thoroughly saturating the mats with water, and keeping the mats continuously wet with sprinklers or porous hoses. Place curing mats so as to provide coverage of the concrete

surfaces and edges, with a 4-inch lap over adjacent mats. If necessary, the curing cover shall be weighted to maintain contact with the concrete surface.

- d. At the end of the curing period apply one coat of curing compound, unless concrete surface is to receive a topping or coating or application is waived by the ENGINEER.
 3. Provide moisture retaining cover curing as follows:
 - a. Cover the concrete surfaces with the specified moisture retaining cover for curing concrete, placed in the widest practical width with sides and ends lapped at least 3-inches and sealed by waterproof tape or adhesive. Immediately repair any holes or tears during the curing period using cover material and waterproof tape.
 4. Provide liquid curing compound as follows:
 - a. Apply the specified curing compound to all concrete surfaces when permitted by ENGINEER. Slabs to receive terrazzo floors, chemical resistant heavy duty concrete topping or ceramic tile, shall not be cured with liquid curing compound, but shall be moisture cured. The compounds shall be applied immediately after final finishing in a continuous operation by power spray equipment in accordance with the manufacturer's directions. Recoat areas that are subjected to heavy rainfall within three hours after initial application. Maintain the continuity of the coating and repair damage to the coat during the entire curing period. For concrete surfaces that will be in contact with potable water, the manufacturer shall certify that the curing compound meets the requirements of ANSI/NSF 61.
 - b. When curing compound is authorized for application to water retaining or below grade members, it shall be applied at the manufacturer's recommended coverage rate and then applied again at the same rate to provide twice the recommended coverage.
 - c. At the end of the curing period, curing compound shall be removed where required by the ENGINEER.
- C. Curing Formed Surfaces:
1. Cure formed concrete surfaces; including the undersides of girders, beams, supported slabs and other similar surfaces by moist curing with the forms in place unloosened for the full curing period or until forms are removed. Where wood forms are kept in place, water shall be added to keep the forms wet. If forms are removed, continue curing by methods specified above, as applicable.
- D. Curing Unformed Surfaces:
1. Initially cure unformed surfaces, such as slabs, floor topping, and other flat surfaces by using the appropriate method specified above.
 2. Final cure unformed surfaces, unless otherwise specified, by utilizing methods specified above, as applicable.
- E. Temperature of Concrete During Curing:

1. When the atmospheric temperature is 40°F and below, maintain the concrete temperature between 50°F and 70°F continuously throughout the curing period. When necessary, make arrangement before concrete placing for heating, covering, insulation or housing as required to maintain the specified temperature and moisture conditions continuously for the concrete curing period. Provide cold weather protection complying with the requirements of ACI 306.
2. When the atmospheric temperature is 80°F and above, or during other climatic conditions which will cause too rapid drying of the concrete, make arrangements before the start of concrete placing for the installation of wind breaks or shading, and for fog spraying, wet sprinkling, or moisture retaining covering. Protect the concrete continuously for the concrete curing period. Provide hot weather protection complying with the requirements of ACI 305, unless otherwise specified.
3. Maintain concrete temperature as uniformly as possible, and protect from rapid atmospheric temperature changes. Avoid temperature changes in concrete which exceed 5°F in any one hour and 50°F in any 24 hour period.

F. Protection from Mechanical Injury:

1. During the curing period, protect concrete from damaging mechanical disturbances including load stresses, heavy shock, excessive vibration, and from damage caused by rain or flowing water. Protect all finished concrete surfaces from damage by subsequent construction operations.

3.9 FIELD QUALITY CONTROL

A. The OWNER shall employ a testing laboratory to perform field quality control testing. ENGINEER will direct the number of tests and cylinders required. Make standard compression test cylinders and entrained air tests as specified below, under the direct inspection by ENGINEER. Also, provide all labor, material and equipment required including, scale, glass tray, cones, rods, molds, air tester, thermometer, curing in a heated storage box, and all other incidentals required. Above will be subject to approval by ENGINEER. Furnish all necessary storage and curing, and transportation required by the testing.

B. Quality Control Testing During Construction:

1. Perform sampling and testing for field quality control during the placement of concrete, as follows:
 - a. Sampling Fresh Concrete: ASTM C 172.
 - b. Slump: ASTM C 143; one test for each concrete load at point of discharge; and one for each set of compressive strength test specimens.
 - c. Air Content: ASTM C 231; one for every other concrete load at point of discharge, or when required by an indication of change.
 - d. Compressive Strength Tests: ASTM C 39; one set of compression cylinders for each 50 cubic yards or fraction thereof, of each mix design

placed in any one day; one specimen tested at seven days, and three specimens tested at 28 days.

- 1) Adjust mix if test results are unsatisfactory and resubmit for ENGINEER'S approval.
 - 2) Concrete that does not meet the strength requirements is subject to rejection and removal from the Work, or to other such corrective measures as directed by ENGINEER, at the expense of CONTRACTOR.
- e. Compression Test Specimens: ASTM C 31; make one set of four standard cylinders for each compressive strength test, unless otherwise directed.
- 1) Cast, store and cure specimens as specified in ASTM C 31.
- f. Water Cementitious Materials Ratio: Perform one test from each sample from which compression test specimens are taken in accordance with AASHTO TP 23.
- g. Concrete Temperature: Test hourly when air temperature is 40°F and below, and when 80°F and above; and each time a set of compression test specimens is made.
2. The testing laboratory shall submit certified copies of test results directly to ENGINEER and CONTRACTOR within 24 hours after tests are made.

C. Evaluation of Quality Control Tests:

1. Do not use concrete delivered to the final point of placement, which has slump or total air content outside the specified values.
2. When water content testing indicates water-cementitious materials ratio to exceed specified requirements by more than 0.02, remaining batches needed to complete the concrete placement shall have water content decreased in the mix and water reducing admixture dosage increased as needed to bring the subsequently batched concrete within the specified water-cementitious materials ratio. Additional testing shall be done to verify compliance with the specified water-cementitious materials ratio. Concrete production for further concrete placements shall not resume until CONTRACTOR has identified the cause of the excess water in the mix and revised batching procedures and/or adjustments to mix design needed to bring water-cementitious materials ratio into conformance with specified requirements have been accepted by ENGINEER.
3. Compressive strength tests for laboratory-cured cylinders will be considered satisfactory if the averages of all sets of three consecutive compressive strength tests results equal or exceed the 28 day design compressive strength of the type or class of concrete; and, no individual strength test falls below the required compressive strength by more than 500 psi.
 - a. Where questionable field conditions may exist during placing concrete or immediately thereafter, strength tests of specimens cured under field conditions will be required by ENGINEER to check the adequacy of curing and protecting of the concrete placed. Specimens shall be molded at the same time and from the same samples as the laboratory cured specimens.

- 1) Provide improved means and procedures for protecting concrete when the 28 day compressive strength of field-cured cylinders is less than 85 percent of companion laboratory-cured cylinders.
 - 2) When laboratory-cured cylinder strengths are appreciably higher than the minimum required compressive strength, field-cured cylinder strengths need not exceed the minimum required compressive strength by more than 500 psi even though the 85 percent criterion is not met.
 - 3) If individual tests of laboratory-cured specimens produce strengths more than 500 psi below the required minimum compressive strength, or if tests of field-cured cylinders indicate deficiencies in protection and curing, provide additional measures to assure that the load-bearing capacity of the structure is not jeopardized. If the likelihood of low-strength concrete is confirmed and computations indicate the load-bearing capacity may have been significantly reduced, tests of cores drilled from the area in question will be required at CONTRACTOR'S expense.
- b. If the compressive strength tests fail to meet the minimum requirements specified, the concrete represented by such tests will be considered deficient in strength and subject to replacement, reconstruction or to other action approved by ENGINEER.

D. Testing Concrete Structure for Strength:

1. When there is evidence that the strength of the in-place concrete does not meet specification requirements, employ at CONTRACTOR'S expense the services of a concrete testing service to take cores drilled from hardened concrete for compressive strength determination. Tests shall comply with the requirements of ASTM C 42 and the following:
 - a. Take at least three representative cores from each member or suspect area at locations directed by ENGINEER.
 - b. Strength of concrete for each series of cores will be considered satisfactory if their average compressive strength is at least 85 percent and no single core is less than 75 percent of the 28 day required compressive strength.
 - c. Report test results to ENGINEER, in writing, on the same day that tests are made. Include in test reports, the Project identification name and number, date, name of CONTRACTOR, name of concrete testing service, location of test core in the structure, type or class of concrete represented by core sample, nominal maximum size aggregate, design compressive strength, compression breaking strength and type of break (corrected for length-diameter ratio), direction of applied load to core with respect to horizontal plane of the concrete as placed, and the moisture condition of the core at time of testing.
2. Fill core holes solid with non-shrink, high strength grout, and finish to match adjacent concrete surfaces.

3. Conduct static load test and evaluations complying with the requirements of ACI 318 if the results of the core tests are unsatisfactory, or if core tests are impractical to obtain, as directed by ENGINEER.

3.10 MISCELLANEOUS CONCRETE ITEMS

A. Temporary Openings

1. Openings in concrete walls and/or slabs required for passage of Work or installation of equipment and not shown on the Drawings shall be provided, but only with approval of the ENGINEER.
2. All temporary openings made in concrete shall be provided with waterstop in below grade or water retaining members. Continuity of required reinforcement shall be provided in a manner acceptable to the ENGINEER.
3. Temporary openings left in concrete structures shall be filled with concrete after the Work causing the need for the opening is in place, unless otherwise shown or directed. Mix, place and cure concrete as specified herein, to blend with in-place construction. Provide all other miscellaneous concrete filling shown or required to complete the Work.

B. Equipment Bases:

1. Unless specifically shown otherwise, provide concrete bases for all pumps and other equipment. Coordinate and construct bases to the dimensions shown, or as required to meet manufacturers' requirements and Drawing elevations. Where no specific elevations are shown, bases shall be 6-inches thick and extend 3-inches outside the metal equipment base or supports. Bases shall have smooth trowel finish, unless a special finish such as terrazzo, ceramic tile or heavy duty concrete topping is required. In those cases, provide appropriate concrete finish.
2. Include all concrete equipment base work not specifically included under other Sections.
3. In general, place bases up to 1-inch below the metal base. Properly shim equipment to grade and fill 1-inch void with non-shrink grout as specified in Section 03600, Grout.

C. Curbs:

1. Provide monolithic finish to interior curbs by stripping forms while concrete is still green and steel-troweling surfaces to a hard, dense finish with corners, intersections, and terminations slightly rounded.
2. Exterior curbs shall have rubbed finish for vertical surfaces and a broomed finish for top surfaces.

3.11 CONCRETE REPAIRS

A. Repair of Formed Surfaces:

1. The following defects shall be repaired in all types of formed finishes:

- a. Spalls, air bubbles, rock pockets, form depressions, and other defects that are more than 1/4-inch in depth.
 - b. Holes from tie rods and other form tie systems.
 - c. Fins, offsets and other projections that extend more than 1/4-inch beyond the designated member surface.
 - d. Structural cracks, as defined by the ENGINEER.
 - e. Non-structural cracks, as defined by the ENGINEER, which are greater than 0.010-inch wide. In water retaining members, elevated slabs subject to rainfall and washdown, and below grade members, any crack that shows any amount of leakage. Where it is not possible to verify that a crack is not leaking, it shall be repaired.
 2. The following defects shall be repaired in smooth finish surfaces, in addition to those listed above:
 - a. Spalls, air bubbles, rock pockets, form depressions, and other defects which extend to more than 1/2-inch in width in any direction, no matter how deep.
 - b. Spalls, air bubbles, rock pockets, form depressions, and other defects of any size that exceed three in number in a 12-inch square or 12 in number in a three foot square.
 - c. Fins, offsets and other projections shall be completely removed and smoothed.
 - d. Scratches and gouges in the surface.
 - e. Texture and color irregularities. At water retaining surfaces, texture and color irregularities need not be repaired when greater than 12-inches below the minimum normal operating water surface, except where such defects are indicative of reduced durability.
 3. Where a smooth rubbed or grout cleaned finish is specified, minor surface defects repairable by the finishing process need not be repaired prior to the finish application, when approved by the ENGINEER.
- B. Method of Repair of Formed Surfaces:
1. Repair and patch defective areas with cement mortar or concrete repair mortar immediately after removal of forms and as directed by ENGINEER. Repairs made to water bearing and buried surfaces shall be made with repair mortar only. Repairs of form tie holes on water bearing or buried surfaces shall be made with non-shrink grout as specified in Section 03600, Grout.
 2. Cut out honeycomb, rock pockets, voids, and holes left by tie rods and bolts, down to solid concrete but, in no case, to a depth of less than 1-inch for cement mortar and 1/2-inch for repair mortar. Make edges of cuts perpendicular to the concrete surface. Before placing the cement mortar, thoroughly clean and brush-coat the area to be patched with the specified bonding agent. Where concrete repair mortar is used, bonding agent shall be optional and the surface prepared and mortar placed per manufacturers recommendations.
 - a. Repairs at exposed-to-view surfaces shall match the color of surrounding concrete, except color matching is not required for the interior surfaces of liquid containers up to one foot below liquid level. Impart texture to

repaired surfaces to match texture of existing adjacent surfaces. Provide test areas at inconspicuous locations to verify mixture, texture and color match before proceeding with the patching. Compact mortar in place and strike off slightly higher than the surrounding surface.

3. Structural cracks shall be pressure grouted using an injectable epoxy using a pumped pressure system. Apply in accordance with the manufacturer's directions and recommendations.
4. Non-structural cracks shall be pressure grouted using hydrophilic resin. Apply in accordance with the manufacturer's directions and recommendations.
5. Determination of the crack type shall be made by the ENGINEER.
6. Fill holes extending through concrete by means of a plunger- type gun or other suitable device from the least exposed face, using a flush stop held at the exposed face to ensure completely filling. At below grade and water retaining members, fill holes with concrete repair mortar except use a color matched cement mortar for the outer 2-inches at exposed to view surfaces.
7. Where powerwashing and/or scrubbing is not adequate, abrasive blast exposed-to-view surfaces that require removal of stains, grout accumulations, sealing compounds, and other substances marring the surfaces. Use sand finer than No. 30 and air pressure from 15 to 25 psi.

C. Repair of Unformed Surfaces:

1. Test unformed surfaces, such as monolithic slabs, for smoothness and to verify surface plane to the tolerances specified for each surface and finish. Correct low and high areas as herein specified.
2. Test unformed surfaces sloped to drain for trueness of slope, in addition to smoothness, using a template having the required slope. Correct high and low areas as herein specified.
3. Repair finish of unformed surfaces that contain defects that adversely affect the durability of the concrete. Surface defects include crazing, cracks in excess of 0.01-inch wide, spalling, popouts, honeycomb, rock pockets, and other objectionable conditions.
4. Repair structural cracks in all structures and non-structural cracks in water-holding structures. In water-holding structures, where the dry face of the concrete member can be observed, cracks that show any rate of water flow shall be repaired. Where the dry face of the member cannot be observed, all cracks shall be repaired.

D. Methods of Repair of Unformed Surfaces:

1. Correct high areas in unformed surfaces by grinding, after the concrete has cured sufficiently so that repairs can be made without damage to adjacent areas.
2. Correct low areas in unformed surfaces during, or immediately after completion of surface finishing operations by cutting out the low areas and replacing with fresh concrete. Finish repaired areas to blend into adjacent concrete. Where the concrete has already set and repairs are required, sawcut around the perimeter of the area to be repaired to a 1/2-inch depth and remove concrete so that the

minimum thickness of the repair is 1/2-inch. Apply specified concrete repair mortar in accordance with the manufacturer's directions and recommendations.

3. Repair defective areas, except random cracks and single holes not exceeding 1-inch diameter, by cutting out and replacing with fresh concrete. Remove defective areas to sound concrete with clean, square cuts, and expose reinforcing steel with at least 3/4-inch clearance all around. The minimum thickness of the repair shall be 1.5-inches. Dampen all concrete surfaces in contact with patching concrete and brush with the specified bonding agent. Place patching concrete while the bonding agent is still tacky. Mix patching concrete of the same materials and proportions to provide concrete of the same classification as the original adjacent concrete. Place, compact and finish as required to blend with adjacent finished concrete. Cure in the same manner as adjacent concrete.
4. Repair isolated random non-structural cracks (in members which are not below grade or water retaining), and single holes not over 1-inch diameter, by the dry-pack method. Groove the top of cracks, and cut out holes to sound concrete and clean of dust, dirt and loose particles. Dampen all cleaned concrete surfaces and brush with the specified bonding agent. Place dry-pack before the cement grout takes its initial set. Mix dry-pack, consisting of one part portland cement to 2-1/2 parts fine aggregate passing a No. 16 mesh sieve, using only enough water as required for handling and placing. Compact dry-pack mixture in place and finish to match adjacent concrete. Keep patched areas continuously moist for not less than 72 hours.
5. Structural cracks shall be pressure grouted using an injectable epoxy. Apply in accordance with the manufacturer's directions and recommendations.
6. Non-structural cracks in below grade and water retaining structures shall be pressure grouted using hydrophilic resin. Apply in accordance with the manufacturer's directions and recommendations.
7. Determination of the crack type shall be made by the ENGINEER.
8. Assure that surface is acceptable for flooring material to be installed in accordance with manufacturer's recommendations.

E. Other Methods of Repair:

1. Repair methods not specified above may be used if approved by ENGINEER.

++ END OF SECTION ++

SECTION 03600

GROUT

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Scope:
1. Provide all labor, materials, equipment, and incidentals as shown, specified and required to furnish and install grout.
 2. Section includes:
 - a. Requirements for the grouts, except masonry grouts, referenced on the Contract Drawings and in other Specification Sections.
 - b. Types of grouts and their applications. Unless indicated otherwise, provide grouts as listed in this Section whether called for on the Drawings or not.

1.2 QUALITY ASSURANCE

- A. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown or specified.
1. ACI 211.1, Practice for Selecting Proportions for Normal, Heavy-Weight and Mass Concrete.
 2. ACI 301, Specification for Structural Concrete (Includes ASTM Standards referred to herein).
 3. ASTM C 33, Specification for Concrete Aggregates.
 4. ASTM C 109, Test Method for Compressive Strength of Hydraulic Cement Mortars (using 2-in. or 50 mm. Cube Specimens).
 5. ASTM C 150, Specification for Portland Cement.
 6. ASTM C 230, Specification for Flow Table for use in Tests of Hydraulic Cement.
 7. ASTM C 531, Test Method for Linear Shrinkage and Coefficient of Thermal Expansion of Chemical-Resistant Mortars, Grouts, and Monolithic Surfacing.
 8. ASTM C 579, Test Method for Compressive Strength of Chemical-Resistant Mortars, Grouts, Monolithic Surfacing and Polymer Concretes.
 9. ASTM C 827, Test Method for Early Volume Change of Cementitious Mixtures.
 10. ASTM C 882, Test Method for Bond Strength of Epoxy-Resin Systems Used with Concrete.
 11. ASTM C 937, Specification for Grout Fluidifier for Preplaced-Aggregate Concrete.
 12. ASTM C 939, Test Method for Flow of Grout for Preplaced-Aggregate Concrete (Flow Cone Method).

13. ASTM C 942, Standard Test Method for Compressive Strength of Grouts for Preplaced-Aggregate Concrete in the Laboratory.
14. ASTM C 1107, Specification for Packaged Dry, Hydraulic-Cement Grout (Non-shrink).
15. ASTM C 1181, Test Method for Compressive Creep of Chemical-Resistant Polymer Machinery Grouts.
16. ASTM D 696, Test Method for Coefficient of Linear Thermal Expansion of Plastics.

B. Field Tests:

1. Compression test specimens will be taken during construction from the first placement of each type of grout, and at intervals thereafter as selected by the ENGINEER to ensure continued compliance with these specifications. The specimens will be made by the ENGINEER or its representative.
2. Compression tests and fabrication of specimens for non-shrink grout will be performed as specified in ASTM C 109 at intervals during construction as selected by the ENGINEER. A set of three specimens will be made for testing at seven days, 28 days, and each additional time period as appropriate.
3. Compression tests and fabrication of specimens for epoxy grout will be performed as specified in ASTM C 579, Method B, at intervals during construction as selected by the ENGINEER. A set of three specimens will be made for testing at seven days, and each earlier time period as appropriate.
4. The cost of all laboratory tests on grout will be borne by the OWNER, but CONTRACTOR provide assistance to the ENGINEER in obtaining specimens for testing. However, the cost of any additional tests and investigation on work performed which does not conform to the requirements of the specifications belongs to CONTRACTOR. Supply all materials necessary for fabricating the test specimens.

1.3 SUBMITTALS

A. Shop Drawings: Submit for approval the following:

1. Refer to and comply with the requirements of Section 01332, Shop Drawing Procedures.
2. For Grout Fill and Construction Joint Grout, copies of grout design mix and laboratory test reports for grout strength tests.
3. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures.
4. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR,

each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

- B. Reports and Certificates, submit for approval the following:
1. For proprietary materials, submit copies of manufacturer's certification of compliance with the specified properties.
 2. Submit certified testing lab reports for ASTM C 1107, Grade B and Grade C (as revised herein) requirements for grouts tested at a fluid consistency for temperatures of 45, 73.4, 90°F with a pot life of 30 minutes at fluid consistency.
 3. Submit certification that materials meet specification requirements for nonproprietary materials.
 4. Submit certifications that all grouts used on the project are free of chlorides or other chemicals causing corrosion.
 5. Manufacturer's specifications and installation instructions for all proprietary materials.
- C. Informational Submittals:
1. Grout placement plan.
 2. Description of proposed curing methods.

1.4 PRODUCT DELIVERY, STORAGE AND HANDLING

- A. Delivery of Materials: Grout materials from manufacturers shall be delivered in unopened containers and shall bear intact manufacturer's labels.
- B. Storage and Handling of Materials:
1. Grout materials shall be stored in a dry shelter and shall be protected from moisture.
 2. Store and precondition all grout and grout materials per grout manufacturer requirements. Provide air conditioned storage facility as may be required.
 3. Store grout materials in undamaged condition with seals and labels intact as packaged by the manufacturer.

PART 2 - PRODUCTS

2.1 CEMENT BASED NON-SHRINK GROUTS

A. General:

1. Non-shrink grout shall be a prepackaged, inorganic, non-gas-liberating, cement-based grout requiring only the addition of water. Manufacturer's instructions shall be printed on each bag or other container in which the materials are packaged. The specific formulation for each type of non-shrink grout specified herein shall be that recommended by the manufacturer for the particular application.
2. Provide grout that meets the performance requirements of ASTM C 1107.
3. Use hydraulic cement complying with ASTM C 150, Type I for exposures not in contact with sewage or soils, and Type II (sulfate resistant) for exposures in contact with sewage or soils.
4. Do not add other materials to the grout with the exception of aggregate only as recommended by the manufacturer.
5. The non-shrink property must not be based on a chemically generated gas or gypsum expansion.
6. The grout when tested shall not bleed at maximum allowed water.
7. Volume change:
 - a. The length change from placement to time of final set shall not have a shrinkage greater than the amount of expansion measured at 3 or 14 days. The expansion at 3 or 14 days shall not exceed the 28-day expansion.
 - b. Range: 0.0 to 0.3 percent when tested in accordance with ASTM C1090.
8. Provide Non-Shrink Non-Metallic Grout wherever "non-shrink grout" is referenced on the Drawings, unless noted otherwise.

B. Non-Shrink Metallic Precision Grout:

1. Application: Use for precision grouting where non-shrink reliability for optimum bearing, high strength, high impact resistance, non-raveling edges, and fluid consistency are required. Not intended for exposures where submerged in water for extended periods of time. Example applications are as follows.
 - a. Heavy duty machinery and equipment.
 - b. Bearing plates.
 - c. Structural base plate or precast concrete bearing for greater than 2 stories.
2. Aggregate: internally reinforced with metallic aggregate.
3. Compressive Strength: Tested per ASTM C 109 or ASTM C 942.
 - a. Flowable Consistency at 28 days: 9000 psi.
 - b. Fluid Consistency at 28 days: 7000 psi.
4. Consistency: Shall meet the requirements of ASTM C 1107 Grade C and B (as modified below) when tested using the amount of water required to achieve the following properties:
 - a. Fluid consistency (20 to 30 seconds) in accordance with ASTM C 939.

- b. At temperatures of 45, 73.4, and 95°F.
 - 5. Product and Manufacturer: Provide one of the following:
 - a. Embeco 885, as manufactured by BASF Construction Chemicals, LLC.
 - b. NS Metallic Grout, as manufactured by the Euclid Chemical Company.
 - c. Or equal.
- C. Non-Shrink Non-Metallic Precision Grout:
 - 1. Application: Use for precision grouting where positive expansion for optimum bearing, high strength, and fluid consistency are required. Example applications are as follows.
 - a. Machinery and equipment.
 - b. Bearing plates.
 - c. Structural steel or precast concrete bearing.
 - d. Anchor bolts and dowels.
 - e. Storage tanks and vessels.
 - 2. Aggregate: Provide mineral, non-metallic aggregate.
 - 3. Compressive Strength: Tested per ASTM C 109 or ASTM C 942.
 - a. Flowable Consistency at 28 days: 9000 psi.
 - b. Fluid Consistency at 28 days: 7000 psi.
 - 4. Consistency: Shall meet the requirements of ASTM C 1107 Grade C and B (as modified below) when tested using the amount of water required to achieve the following properties:
 - a. Fluid consistency (20 to 30 seconds) in accordance with ASTM C 939.
 - b. At temperatures of 45, 73.4, and 95°F.
 - 5. Acceptable Product and Manufacturer:
 - a. Masterflow 928, as manufactured by BASF Construction Chemicals, LLC.
 - b. NS Grout, as manufactured by the Euclid Chemical Company.
 - c. Five Star High Strength Grout, as manufactured by Five Star Products, Inc.
 - d. Or equal.
- D. Non-Shrink Non-Metallic Grout:
 - 1. Application: Use for general construction grouting where non-shrink, high strength, and flowable to damp pack consistency are required. Example applications are as follows.
 - a. Bedding grout for precast panels.
 - b. Base plates.
 - c. Low-rise Structural steel or precast concrete bearing.
 - d. Anchor bolts and dowels.
 - e. Damp pack applications.
 - f. Filling blocked out spaces for embedded items such as railing posts, gate guide frames, etc.
 - 2. Aggregate: Provide mineral, non-metallic aggregate.
 - 3. Compressive Strength: Tested per ASTM C 109 or ASTM C 942.
 - a. Flowable Consistency at 28 days: 7000 psi.

- b. Damp Pack at 28 days: 8000 psi.
- 4. Consistency: Shall meet the requirements of ASTM C 1107 Grade C and B (as modified below) when tested using the amount of water required to achieve the following properties:
 - a. Flowable consistency (125 to 145 percent flow on flow table, ASTM C 230, five drops in 30 seconds.)
 - b. Flowable for at least 30 minutes.
- 5. Product and Manufacturer: Provide one of the following:
 - a. Construction Grout, as manufactured by BASF Construction Chemicals, LLC.
 - b. Five Star Grout, as manufactured by Five Star Products, Inc.
 - c. Dry Pack Grout, as manufactured by the Euclid Chemical Company.
 - d. Or equal.

2.2 EPOXY GROUTS

- A. Non-Shrink Epoxy Grout:
 - 1. Application: Use for high performance grouting under conditions of heavy impact, dynamic, and vibratory loading. Example applications are as follows.
 - a. High performance machinery grouting,
 - b. Reciprocating engines,
 - c. Crane rail grouting,
 - d. Precision seating and alignment generators, pumps, and electric motors,
 - e. Grouted anchors, bars and dowels.
 - 2. Epoxy grout shall be a pourable, non-shrink, 100 percent solids system. The epoxy grout system shall have three components: resin, hardener, and specially blended aggregate, all premeasured and prepackaged. The resin component shall not contain any non-reactive diluents. Resins containing butyl glycidyl ether (BGE) or other highly volatile and hazardous reactive diluents are not acceptable. Variation of component ratios is not permitted, unless specifically recommended by the manufacturer. Manufacturer's instructions shall be printed on each container in which the materials are packaged. The following properties shall be attained with the minimum quantity of aggregate allowed by the manufacturer.
 - 3. The vertical volume change at all times before hardening shall be between 0.0 percent shrinkage and 4.0 percent expansion when measured according to ASTM C 827 (modified for epoxy grouts by using an indicator ball with a specific gravity between 0.9 and 1.1). Alternately, epoxy grouts which maintain an effective bearing area of not less than 95 percent are acceptable.
 - 4. The length change after hardening shall be negligible (less than 0.0006 in/in) and the coefficient of thermal expansion shall be less than 0.00003 in/in/F when tested in accordance to the requirements of ASTM C 531.

5. The compressive creep at one year shall be negligible (less than .001 in/in) when tested under a 400 psi constant load at 140°F in accordance to the requirements of ASTM C 1181.
6. The seven day compressive strength shall be a minimum of 14,000 psi when tested in accordance to the requirements of ASTM C 579.
7. The grout shall be capable of maintaining at least a flowable consistency for a minimum of 30 minutes at 70°F.
8. The shear bond strength to portland cement concrete shall be greater than the shear strength of the concrete when tested in accordance to the requirements of ASTM C 882.
9. The effective bearing area shall be a minimum of 95 percent.
10. Product and Manufacturer: Provide one of the following:
 - a. E3-HP, High Performance Epoxy Grout System, as manufactured by The Euclid Chemical Company.
 - b. Sikadur 42 Grout Pak, as manufactured by Sika Corporation.
 - c. Five Star Epoxy Grout, as manufactured by Five Star Products, Incorporated.
 - d. Masterflow 648 CP, as manufactured by BASF Construction Chemicals, LLC.
 - e. Or equal.

2.3 CONCRETE GROUTS

A. Grout Fill, Topping Grout:

1. Grout for topping of slabs and concrete fill for built-up surfaces of tank, channel, and basin bottoms shall be composed of cement, fine aggregate, coarse aggregate, water, and admixtures proportioned and mixed as specified herein. All materials and procedures specified for normal concrete in Section 03300, Cast-In-Place Concrete, shall apply except as noted otherwise herein.
2. Topping grout and concrete fill shall contain a minimum of 564 pounds of cement per cubic yard with a maximum water cement ratio of 0.45. Where concrete fill is thicker than 4-inches, Type "1" concrete, as specified in Section 03300, Cast-In-Place Concrete, may be used when accepted by the ENGINEER.
3. Coarse aggregate shall be graded as follows:

U.S. STANDARD	PERCENT BY
<u>SIEVE SIZE</u>	<u>WEIGHT PASSING</u>
1/2-inch	100
3/8-inch	90-100
No. 4	20-55
No. 8	5-30
No. 16	0-10
No. 30	0

4. Final mix design shall be as determined by trial mix design under supervision of the approved testing laboratory.
 5. Strength: Minimum compressive strength of Grout Fill at the end of 28 days shall be 4000 psi.
- B. Construction Joint Grout:
1. Construction Joint Grout approximates Type “1” concrete, as specified in Section 03300, Cast-In-Place Concrete, with aggregate coarser than 1/2-inch removed. The mix is to be designed as flowable with a high mortar content. It is intended to be placed over construction joints and mixed with Type “1” concrete as specified in Section 03300, Cast-In-Place Concrete. The mix requirements are as follows:
 - a. Compressive Strength: 4,500 psi minimum at 28-days.
 - b. Maximum Water-Cement Ratio: 0.45 by weight.
 - c. Coarse Aggregate: ASTM C33, No. 8 size.
 - d. Fine Aggregate: ASTM C33, approximately 60 percent by weight of total aggregate.
 - e. Air Content: 8±1 percent.
 - f. Minimum Cement Content: 752 pounds per cubic yard.
- C. Requirements for Grout Fill and Construction Joint Grout:
1. Proportion mixes by either laboratory trial batch or field experience methods, using materials to be employed on the Project for grout required. Comply with ACI 211.1 and report to ENGINEER the following data:
 - a. Complete identification of aggregate source of supply.
 - b. Tests of aggregates for compliance with specified requirements.
 - c. Scale weight of each aggregate.
 - d. Absorbed water in each aggregate.
 - e. Brand, type and composition of cement.
 - f. Brand, type and amount of each admixture.
 - g. Amounts of water used in trial mixes.
 - h. Proportions of each material per cubic yard.
 - i. Gross weight and yield per cubic yard of trial mixtures.
 - j. Measured slump.
 - k. Measured air content.
 - l. Compressive strength developed at seven days and 28 days, from not less than three test specimens cast for each seven day and 28-day test, and for each design mix.
 2. Submit written reports to ENGINEER of proposed mix of grout at least 30 days prior to start of Work. Do not begin grout production until mixes have been approved by ENGINEER.
 3. Laboratory Trial Batches: When laboratory trial batches are used to select grout proportions, prepare test specimens and conduct strength tests as specified in ACI 301, Section 4 - Proportioning. However, mixes need not be designed for

greater than 125 percent of the specified strength, regardless of the standard deviation of the production facility.

4. Field Experience Method: When field experience methods are used to select grout proportions, establish proportions as specified in ACI 301, Section 4.
5. Admixtures: Use air-entraining admixture in all grout. Use amounts of admixtures as recommended by the manufacturer for climatic conditions prevailing at the time of placing. Adjust quantities and types of admixtures as required to maintain quality control. Do not use admixtures which have not been incorporated and tested in the accepted design mix, unless otherwise authorized in writing by ENGINEER.

2.4 CURING MATERIALS

- A. Curing materials shall be as specified in Section 03300, Cast-in-Place Concrete, and as recommended by the manufacturer of prepackaged grouts.

2.5 CONSISTENCY

- A. The consistency of grouts shall be that necessary to completely fill the space to be grouted for the particular application. Dry pack consistency is such that the grout is plastic and moldable but will not flow. Where "dry pack" is called for in the Contract Documents, it shall mean a grout of that consistency; the type of grout to be used shall be as specified herein for the particular application.
- B. The slump for topping grout and grout fill shall be adjusted to match placement and finishing conditions, but shall not exceed 4-inches.
- C. The slump for Construction Joint Grout shall be 7 ± 1 -inches.

2.6 MATERIALS

- A. Aggregate:
 1. Provide aggregate that conforms to the material requirements of Section 03300 Cast-In-Place Concrete, and the grout manufacturer's requirements.
 2. Store and precondition aggregate per grout manufacturer's requirements.
- B. Water:
 1. Potable quality water that is free from deleterious amounts of acids, alkalis, and organic substances.

PART 3 - EXECUTION

3.1 INSPECTION

- A. Examine the substrate and conditions under which grout is to be placed and notify ENGINEER, in writing, of unsatisfactory conditions. Do not proceed with the Work until unsatisfactory conditions have been corrected in a manner acceptable to ENGINEER.

3.2 INSTALLATION

- A. General:
1. Place grout as shown and in accordance with manufacturer's instructions. If manufacturer's instructions conflict with the Specifications do not proceed until ENGINEER provides clarification.
 2. Manufacturers of proprietary products shall make available upon 72 hours notification the services of a qualified, full time employee to aid in assuring proper use of the product under job conditions.
 3. Placing grout shall conform to temperature and weather limitations in Section 03300, Cast-In-Place Concrete.
 4. Grout shall be cured following manufacturer's instructions for prepackaged grout and the requirements in Section 03300, Cast-In-Place Concrete, for grout fill and topping grout.
- B. Columns, Beams and Equipment Bases:
1. Epoxy grout: After shimming equipment to proper grade, securely tighten anchor bolts. Properly form around the base plates, allowing sufficient room around the edges for placing the grout. Adequate depth between the bottom of the base plate and the top of concrete base must be provided to assure that the void is completely filled with the epoxy grout.
 2. Non-shrink, non-metallic grout: After shimming columns, beams and equipment to proper grade, securely tighten anchor bolts. Properly form around the base plates allowing sufficient room around the edges for placing the grout. Adequate depth between the bottom of the base plate and the top of concrete base must be provided to assure that the void is completely filled with the non-shrink, non-metallic grout.
- C. Handrails and Railings:
1. After posts have been properly inserted into the holes or sleeves, fill the annular space between posts and sleeve with the non-shrink, non-metallic grout. Bevel grout at juncture with post so that moisture flows away from post.

D. Construction Joints:

1. Place a 6-inch minimum thick layer of Construction Joint Grout over the contact surface of the old concrete at the interface of horizontal construction joints as specified in Section 03251, Concrete Joints, and Section 03300, Cast-In-Place Concrete.

E. Topping Grout:

1. All mechanical, electrical, and finish work shall be completed prior to placement of topping grout. The base slab shall be given a roughened textured surface by sandblasting or hydroblasting exposing the aggregates to ensure bonding to the base slab.
2. The minimum thickness of grout topping shall be 1-inch.
3. The base slab shall be thoroughly cleaned and wetted prior to placing topping and fill. No topping concrete shall be placed until the slab is complete free from standing pools or ponds of water. A thin coat of neat Type II cement slurry shall be broomed into the surface of the slab and topping or fill concrete shall be placed while the slurry is still wet. The topping and fill shall be compacted by rolling or tamping, brought to established grade, and floated. Grouted fill for tank and basin bottoms where scraping mechanisms are to be installed shall be screeded by blades attached to the revolving mechanism of the equipment in accordance with the procedures outlined by the equipment manufacturer after the grout is brought to the established grade.
4. Topping grout placed on sloping slabs shall proceed uniformly from the bottom of the slab to the top, for the full width of the placement.
5. The surface shall be tested with a straight edge to detect high and low spots which shall be immediately eliminated. When the topping has hardened sufficiently, it shall be steel troweled to a smooth surface free from pinholes and other imperfections. An approved type of mechanical trowel may be used as an assist in this operation, but the last pass over the surface shall be by hand-troweling. During finishing, no water, dry cement or mixture of dry cement and sand shall be applied to the surface.
6. Cure and protect the grout topping as specified in Section 03300, Cast-In-Place Concrete.

F. Grout Fill:

1. All mechanical, electrical, and finish work shall be completed prior to placement of grout fill. Grout fill shall be mixed, placed, and finished as required in Section 03300, Cast-In-Place Concrete.
2. The minimum thickness of grout fill shall be 1-inch. Where the finished surface of grout fill is to form an intersecting angle of less than 45 degrees with the concrete surface it is to be placed against, a key shall be formed in the concrete surface at the intersection point. The key shall be a minimum of 3-1/2-inches wide by 1-1/2-inches deep.

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3. The surface shall be tested with a straight edge to verify that the surface slopes uniformly to drain and to detect high and low spots which shall be immediately eliminated. When the grout fill has hardened sufficiently, it shall be steel troweled to a smooth surface free from pinholes and other imperfections. During finishing, no water, dry cement or mixture of dry cement and sand shall be applied to the surface.

++ END OF SECTION ++

SECTION 03601

CELLULAR LOW DENSITY GROUT

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Provide all labor, materials, and equipment and perform all operations required to furnish and install, complete, low pressure grouting (less than 6 psi) in the annular space between the steel casing and the gravity sewer pipe. The annular space shall be completely grouted to support the sewer pipe and provide long-term stability. Use a neat cement grout, sanded cement grout, or cellular cement grout for the annular space grouting.
- B. Provide all labor, materials, and equipment and perform all operations required to furnish and install, complete, low pressure grouting (less than 6 psi) in the abandoned sewer lines as shown per the Contract Drawings. Use a neat cement grout, sanded cement grout, or cellular cement grout for filling the abandoned sewer pipe sections.
- C. Performance Requirement:
 - 1. Determine grouting equipment, materials, and methods, subject to the limitations specified herein and elsewhere in the Contract Documents. It is the responsibility of the CONTRACTOR to ensure that the steel casing, the gravity sewer pipes, and all appurtenances are not damaged by the grouting operation.

1.2 QUALITY ASSURANCE

- A. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown or specified.
 - 1. ACI 211.1, Practice for Selecting Proportions for Normal, Heavy-Weight and Mass Concrete.
 - 2. ACI 301, Specification for Structural Concrete (Includes ASTM Standards referred to herein).
 - 3. C31, Practice for Making and Curing Concrete Test Specimens in the Field
 - 4. ASTM C 33, Specification for Concrete Aggregates.
 - 5. ASTM C39, Test Method for Compressive Strength of Cylindrical Concrete Specimens
 - 6. ASTM C87, Test Method for Effect of Organic Impurities in Fine Aggregate on Strength of Mortar.
 - 7. ASTM C 109, Test Method for Compressive Strength of Hydraulic Cement Mortars (using 2-in. or 50 mm. Cube Specimens).

8. ASTM C 150, Specification for Portland Cement.
9. ASTM C 230, Specification for Flow Table for use in Tests of Hydraulic Cement.
10. ASTM C 531, Test Method for Linear Shrinkage and Coefficient of Thermal Expansion of Chemical- Resistant Mortars, Grouts, and Monolithic Surfacing.
11. ASTM C 579, Test Method for Compressive Strength of Chemical-Resistant Mortars, Grouts, Monolithic Surfacing and Polymer Concretes.
12. ASTM C 827, Test Method for Early Volume Change of Cementitious Mixtures.
13. ASTM C 882, Test Method for Bond Strength of Epoxy-Resin Systems Used with Concrete.
14. ASTM C 937, Specification for Grout Fluidifier for Preplaced-Aggregate Concrete.
15. ASTM C 939, Test Method for Flow of Grout for Preplaced-Aggregate Concrete (Flow Cone Method).
16. ASTM C 942, Standard Test Method for Compressive Strength of Grouts for Preplaced-Aggregate Concrete in the Laboratory.
17. ASTM C 1107, Specification for Packaged Dry, Hydraulic-Cement Grout (Non-shrink).
18. ASTM C 1181, Test Method for Compressive Creep of Chemical-Resistant Polymer Machinery Grouts.
19. ASTM D 696, Test Method for Coefficient of Linear Thermal Expansion of Plastics.

1.3 QUALITY CONTROL

- A. The CONTRACTOR or specialty grouting subcontractor shall demonstrate experience and submit references for at least three projects using similar concrete backfill, grouting materials, and procedures.
- B. The low-density cellular grout shall be tested in accordance with ASTM C495 and C796. Determine the wet and dry densities of cellular grout mixes in accordance with ASTM C796. Test neat cement in accordance with ASTM C109.
- C. Prepare trial mixes of the proposed grout backfill 30 days prior to the commencement of grouting operations. Prepare two sets of compression test cylinders, four cylinders per set, from each trial mix. Test one set of cylinders at the age of 24 hours, one at 7 calendar days, and the other set of cylinders at an age of 28 calendar days.

1.4 FIELD TESTS

- A. CONTRACTOR shall take compression test specimens during construction from the first placement of each type of grout, and at intervals thereafter as selected by

the ENGINEER to ensure continued compliance with these specifications. The specimens will be made by the CONTRACTOR or its representative.

- B. Measure cellular grout weights at the point of placement in the annular space to determine its cast density. Weights used at the beginning of the day shall be used to adjust the mix accordingly to achieve the proper cast density. Approximately each hour thereafter, weights shall be measured to check and ensure correct density.
- C. One set of four grout test cubes or cylinders (2-inch by 2-inch or 2-inch diameter) shall be made once per day or for each 100 cubic yards of material placed until 7 calendar day breaks are returned. CONTRACTOR shall employ testing laboratory services which shall use necessary grout containers and sampling techniques in order to obtain the required compressive breaks. Test one cube at 7 calendar days and one cube/cylinder at 28 calendar days. With acceptable 7 calendar day breaks, the collection of grout test cubes/cylinders may be decreased with the written approval of the ENGINEER. Grout cube testing will be provided by the CONTRACTOR.
- D. Provide a system of valves or other means at both the point of discharge and at the point of placement to allow sampling and density checks.
- E. Compression tests and fabrication of specimens for non-shrink grout will be performed as specified in ASTM C 109 at intervals during construction as selected by the ENGINEER. A set of three specimens will be made for testing at seven days, 28 days, and each additional time period as appropriate.
- F. Compression tests and fabrication of specimens for epoxy grout will be performed as specified in ASTM C 579, Method B, at intervals during construction as selected by the ENGINEER. A set of three specimens will be made for testing at seven days, and each earlier time period as appropriate.
- G. The cost of all laboratory tests on grout will be borne by the CONTRACTOR. CONTRACTOR to obtain specimens for testing. The cost of any additional tests and investigation on work performed which does not conform to the requirements of the specifications belongs to CONTRACTOR. Supply all materials necessary for fabricating the test specimens.

1.5 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
 - 1. Refer to and comply with the requirements of Section 01332, Shop Drawing Procedures.
 - 2. For Grout Fill and Construction Joint Grout, copies of grout design mix and laboratory test reports for grout strength tests.

3. Submit drawings showing the method of preventing liner flotation, buckling, excessive deflection and damage, and how the grout supply will be terminated at the location where grout is being injected.
 4. Submit proposed grout mix design and the procedures, methods, and equipment for placing grout between steel casing and gravity sewer pipe.
 5. Submit trial mix details and testing results for grout trial mixes.
 6. Submit field logs documenting grouting operations, containing, as a minimum, date, tests or sampling done on the grout, injection points, volume of grout injected, maximum grouting pressure at injection points, time of day procedures occurred, crew hours, equipment, crew size, special conditions or information, and grout mixes used. For each grout batch, submit a record, or if mixed continuously, submit a continuous record, of grout mix preparation including the amounts of cement, aggregate, water, admixtures, and other constituents. The field log or grout mix record shall include material delivery times and dates. Submit field log and grout mix record to the ENGINEER within one calendar day of each grouting shift.
 7. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures.
 8. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.
- B. Reports and Certificates, submit for approval the following:
1. For proprietary materials, submit copies of manufacturer's certification of compliance with the specified properties.
 2. Submit certified testing lab reports for ASTM C 1107, Grade B and Grade C (as revised herein) requirements for grouts tested at a fluid consistency for temperatures of 45, 73.4, 90°F with a pot life of 30 minutes at fluid consistency.

3. Submit certification that materials meet specification requirements for nonproprietary materials.
 4. Submit certifications that all grouts used on the project are free of chlorides or other chemicals causing corrosion.
 5. Manufacturer's specifications and installation instructions for all proprietary materials.
- C. Informational Submittals:
1. Grout placement plan.
 2. Description of proposed curing methods.

PART 2 - MATERIALS

2.1 LOW PRESSURE GROUT

- A. Cement shall conform to ASTM C150, Type II, with maximum tricalcium aluminate not to exceed 8%. The maximum percent alkalis shall not exceed 0.6%. Pozzolans and other cementitious material may be used. If cellular grout is used as a foaming agent, the manufacturer or grouting subcontractor shall design the mix, prepare trial batch, test sample, and provide detailed testing data.
- B. Admixtures may be used to improve pumpability, control time of set, and to prevent segregation and bleeding, subject to the acceptance of the ENGINEER and if cellular grout is used, the foaming agent shall be recommended by the manufacturer. If cellular grout is used, use admixtures in accordance with the foaming agent manufacturer's recommendations. Do not use admixtures that promote steel corrosion.
- C. If cellular grout is used, a company that specializes in the manufacture of such a product shall supply the foaming agent. The foaming agent manufacturer shall have been continuously manufacturing the foaming agent for the past five years. The foaming agent shall meet the requirements of ASTM C869 when tested in accordance with ASTM C796.
- D. Water shall be free of organic material and other impurities that might reduce the strength, durability or other quality of the grout. Water shall have a pH of 7.0 to 9.0, a maximum chloride concentration of 300 mg/l (per ASTM D512), and a maximum sulfate concentration of 500 mg/l (per ASTM D516).
- E. Mix Design: Develop and submit for approval one or more mixes to completely fill the annular space based on the size of the annular void and to provide less than 0.5% shrinkage by volume. The mix shall contain a minimum of six sacks cement per cubic yard of grout. The CONTRACTOR shall prepare trial mixes for testing and review.

2.2 PROPERTIES

- A. Cast Density: The grout shall have a minimum cast density of 45 lbs per cubic foot and a maximum of 60 lbs per cubic foot.
- B. Compressive Strength: The grout fill shall have a minimum 24 hour compressive strength of 100 psi and a minimum 28 calendar day compressive strength of 300 psi.
- C. The apparent viscosity shall not exceed 20 seconds in accordance with ASTM C939, unless otherwise approved in writing by the ENGINEER.
- D. Grout pH shall be 10.5 or greater.

2.3 EQUIPMENT

- A. The pressure grouting machine shall be of a low pressure type that has been used successfully for similar work over a period of at least three years. Bidders shall be required to present evidence that his organization or designated subcontractor has successfully performed similar work by the use of a machine of the type proposed to be used hereunder.
- B. General Configuration:
 - 1. Means of accurately measuring and controlling grout ingredients at the mixer shall be provided.
 - 2. The arrangement of grouting equipment shall provide a continuous supply of grout throughout the system and permit accurate pressure control. Grout pumps, mixers, and agitators shall be located as close as practical to the hole being grouted using the minimum length of grout line. Equipment for mixing and injecting grout shall be adequate to satisfactorily mix and agitate the grout and force it into the grout holes in a continuous flow at the desired pressure. Pumps shall be capable of continuously developing a sustained pressure of 3 psi at the grout connection; however, do not exceed the maximum specified pressure.
 - 3. Hoses for pressure grouting shall have an inside diameter not less than 1-1/2-inches and be capable of withstanding the maximum water and grout pressures to be used. Equipment lines shall be kept clean by constant circulation of grout and periodic flushing with water.
 - 4. Provide mechanical agitator tanks equipped with suitable screens in addition to the grout mixer.
- C. Controls and Instrumentation:
 - 1. Pressure Gages:
 - a. Equip the grouting plant with reliable pressure gages capable of measuring the maximum grout pressure anticipated at the manifold

- hookup at the collar of each hole being grouted and at the pump. Make available four clean spare pressure gauges on site at all times.
- b. Provide and protect an accurately calibrated check gage and check operating gages daily or as directed by the ENGINEER.
- 1) Grout Flow: At the grout injection point, provide suitable valves so the pressure and grout flow at the grout starting location may be regulated and monitored by increasing or decreasing the flow in the grout return line.
 - 2) Volume of Grout: Provide means for accurately determining the amount of grout injected into the starting location. Provide grouting equipment with a meter, calibrated in cubic feet to the nearest 0.10 of a cubic foot.
 - 3) Provide a mixer with a meter for measuring the amount of mixing water used, calibrated to read in cubic feet to the nearest 0.10 of a cubic foot.
 - 4) Stop Valves: Provide required stop (check or ball) valves at the collar of the injection location and approved in writing by the ENGINEER for use in maintaining pressure as required until the grout has set.

PART 3 - EXECUTION

3.1 SAFETY REQUIREMENTS

- A. Methods of construction shall be such as to ensure the safety of the Work, Project participants, the public, third parties, and adjacent property, whether public or private. All Work shall conform to the requirements of all Federal, State, and local laws and regulations.
- B. Perform grouting operations by methods and with equipment, which will positively control dust, fumes, vapors, gases, fibers, fogs, mists, and other atmospheric impurities.
- C. Provide lighting and ventilation to ensure proper performance and inspection of work. Provide ventilation equipment and ductwork as close to the working area as practicable. Provide electrical, water, and other facilities required to complete the grouting of the steel liner.
- D. Control noise and dust in accordance with applicable Federal, State, and local laws, safety codes, regulations, and ordinances.
- E. Administer an accident prevention program, and prepare a code of safe practices and an emergency plan. Provide the ENGINEER with a copy of each prior to

starting grouting. Hold safety meetings weekly and provide safety instruction for new employees.

3.2 QUALITY CONTROL

- A. Perform all Work, including required testing and inspection, in the presence of the ENGINEER. Notify the ENGINEER at least 24 hours in advance of the start-up of a grouting operation.
- B. Perform all Work in accordance with all current applicable regulations and codes of Federal, State, and local agencies. In the event of conflict, comply with the strictest or most restrictive applicable requirements.

3.3 GROUT COMPOSITION

- A. Provide grout by combining controlled quantities of sand, cement, water, fluidifier, bentonite, and other admixtures in the proportions specified herein or as approved in writing by the ENGINEER. The resulting grout shall show no shrinkage and have low water absorptive characteristics. Anticipate that the addition of sand will require additional fluidifier to be added to the grout mix.
- B. Recirculate all grout mixes in the grout lines when any initial or new mix is batched or after adding water, fluidifier, or sand to the mix. Recirculate the mix for a minimum of two minutes prior to pumping the grout in the grout hole.

3.4 GROUTING

- A. Following steel casing and gravity sewer installation, completely fill the annular space between the casing and the gravity sewer pipe with grout in accordance with the following:
 - 1. The limits of each grout placement operation shall be acceptable to the ENGINEER.
 - 2. At the start of the grouting operations, extend the discharge pipeline from the pumping equipment to the point of lowest elevation of the section of steel casing being grouted. The methods used in placing the grout shall be such as to ensure the complete filling of the annular space between the steel casing and the gravity sewer pipe. Grout shall be of proper consistency and shall be adequately consolidated to fill all voids. Grouting shall generally progress sequentially in a constant up gradient direction from the lowest point to the highest point.
 - 3. Place grout in a sufficient number of lifts to prevent pipe flotation, unless stiffeners or pipe supports and closely-spaced blocking of sufficient structural rigidity are provided to prevent pipe flotation, movement, buckling, excessive deflection, collapse, and damage to the pipe. Lift heights required to prevent flotation and damage to the pipe shall be determined by the CONTRACTOR.

- B. Commence pressure grouting of steel casing after completion of gravity sewer installation or abandonment of existing gravity sewer pipes.
- C. Fill all voids between the steel casing and the gravity sewer pipe. Do not place new lifts of grout on top of the previous lifts until the previous lifts have achieved sufficient strength to withstand the force exerted by the new lift.
- D. Provide an arrangement of grouting equipment for continuous circulation of grout in the system and accurate pressure control. Keep equipment and hoses clean by constant circulation of grout and by periodic flushing with water.
- E. Provide equipment and conduct grouting procedures such that grouting pressures shall not exceed the maximum specified at the grout injection location. Use stop valves at the collar of each injection location to maintain pressure as required until the grout has set.
- F. Maintain grouting equipment in satisfactory operating condition to ensure continuous and efficient grouting operations.

3.5 MIXING AND INJECTION OF GROUT

- A. Provide materials free of lumps when put into the mixer and agitate the grout mix constantly. Grout shall flow unimpeded and shall completely fill all voids. After 90 minutes of mixing, remove grout not injected from the mixing and injection equipment and dispose of off-site.
- B. In general, grouting lifts shall be considered completed when the pressure at the grout nozzle has reached the maximum specified pressure. After the grouting lift at any grout pipe is finished, maintain the pressure by means of a stopcock or other suitable device until the grout has set. Fill any voids left after removing the packer or grout pipe with mortar.

3.6 CLEANUP AND RESTORATION

- A. Take necessary precautions to protect and preserve the steel casing and the gravity sewer pipes from damage. Replace or repair any damage to the gravity sewer pipes or other appurtenances caused by or occurring during the grouting operations as determined by the ENGINEER.
- B. During grouting work, provide for adequate disposal of waste and wastewater. Minimize grout and mortar spills and cleanup immediately after the grouting shift is complete. Grouting contractor to clean section of pipe after injection prior to commencing the next section of grout injection. Pipe shall be cleaned of all debris caused by grout contractor.

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+ + END OF SECTION + +

SECTION 03930

CONCRETE REPAIR AND REHABILITATION

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. Provide all labor, materials, equipment and incidentals as shown, specified and required to repair or rehabilitate all existing concrete members and surfaces identified in the Contract Documents.
2. Repair all damage to new concrete construction as specified herein, except that where such repairs are specified in Section 03300, Cast-in-Place Concrete.

B. Coordination:

1. Review installation procedures in the following Section and coordinate the installation of items that must be included with the repair and rehabilitation of concrete.

1.2 QUALITY ASSURANCE

A. Reference Standards: Comply with the applicable provisions and recommendations of the following, except as otherwise shown or specified:

1. ASTM C 109, Test Method for Compressive Strength of Hydraulic Cement Mortars.
2. ASTM C 157, Test Method for Length Change of Hardened Cement Mortar and Concrete.
3. ASTM C 882, Test Method for Bond Strength of Epoxy-Resin Systems Used with Concrete.
4. ASTM D 412, Test Methods for Vulcanized and Thermoplastic Rubbers and Thermoplastic Elastomers - Tension.
5. ASTM D 624, Test Method for Tear Strength of Conventional Vulcanized Rubber and Thermoplastic Elastomers.
6. ASTM D 903, Test Method for Peel or Stripping Strength of Adhesive Bonds.
7. ASTM G 109, Test Method for Determining the Effects of Chemical Admixtures on the Corrosion of Embedded Steel Reinforcement in Concrete Exposed to Chloride Environments.

B. Construction Tolerances: Construction tolerances shall be as specified in Section 03300, Cast-in-Place Concrete, except as specified herein and elsewhere in the Contract Documents.

1.3 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
1. Submit manufacturer's product information and recommended placement procedures for all repair materials.
 2. Submit Shop Drawings, when requested by ENGINEER, to show all methods for supporting existing structures, pipes, etc., during demolition and repair activities. Comply with the requirements of Section 01332, Shop Drawing Procedures.
 3. Copies of certificates that show that the repair materials are approved by the National Sanitation Foundation (NSF) for use on surfaces, which are in contact with potable water.

1.4 PRODUCT DELIVERY, STORAGE AND HANDLING

- A. Delivery of Materials:
1. Deliver all materials to the job site in original, new and unopened packages and containers bearing manufacturer's name and label, and the following information.
 - a. Name or title of material.
 - b. Manufacturer's stock number and date of manufacture.
 - c. Manufacturer's name.
- B. Storage of Materials:
1. Storage only acceptable project materials on project site.
 2. Store in a suitable location approved by ENGINEER. Keep area clean and accessible.
 3. Restrict storage to repair materials and related equipment.
 4. Comply with health and fire regulations including the Occupational Safety and Health Act of 1970.
- C. Handling of Materials:
1. Handle materials carefully to prevent inclusion of foreign materials.
 2. Do not open containers or mix components until necessary preparatory Work has been completed and application Work will start immediately.

PART 2 - PRODUCTS

2.1 REPAIR MORTAR

- A. Repair mortar shall be a prepackaged cement-based product specifically formulated for the repair of concrete surface defects. The repair mortar shall be a two-component polymer-modified, portland cement, fast setting, trowel-grade mortar. The repair mortar shall be enhanced with a penetrating corrosion inhibitor and shall have the following properties:

Physical Property	Value	ASTM Standard
<u>Compressive Strength (minimum)</u>		C 109
at 1 day	2000 psi	
at 28 days	6000 psi	
<u>Bond Strength (minimum)</u>		C 882*
at 28 days	1800 psi	

* Modified for use with repair mortars.

- B. Where the least dimension of the placement in width or thickness, exceeds 4-inches, the repair mortar shall be extended by addition of aggregate as recommended by the manufacturer.
- C. Product and Manufacturer: Provide one of the following:
1. SikaTop 122 Plus, SikaTop 123 Plus, or SikaTop 126 Plus, as manufactured by Sika Corporation.
 2. Or equal.

2.2 JOINT REPAIR SYSTEM

- A. Joint Repair System: The joint repair system shall consist of two components, an epoxy resin adhesive and hypalon sheeting.
1. Epoxy Resin Adhesive: Provide a two-component epoxy resin as follows:
 - a. Component A shall be a modified epoxy resin of the epichlorohydrin bisphenol A type containing suitable viscosity control agents and pigments. It shall not contain butyl glycidyl ether.
 - b. Component B shall be primarily a reaction product of a selected amine blend with an epoxy resin of the epichlorohydrin bisphenol A type containing suitable viscosity control agents, pigments and accelerators.
 2. Hypalon Sheeting: Provide Hypalon sheeting as follows:
 - a. Hypalon sheeting shall consist of Hypalon rubber. It shall be perforated along the bonding edge to provide a mechanical key. It shall have the ability to be vulcanized with hydrocarbon solvent to permit its adhesion to an epoxy resin adhesive.
 - b. The sheeting shall be provided in 12-inch width with a thickness of 40 mils.
 - c. The sheeting shall be able to be lapped or seamed by heat or by anaromatic hydrosolvent strip.
 - d. The sheeting shall be supplied with a removable center expansion strip.
 3. Product and Manufacturer: Provide one of the following:

- a. Sikadur Combiflex, as manufactured by Sika Corporation.
- b. Or equal.

2.3 EXPOSED REBAR REPAIR

- A. The exposed reinforcing repair system shall consist of two components, a first application of a corrosion inhibitor and then a final application of a protective slurry mortar.
- B. Corrosion Inhibitor:
 1. The corrosion inhibitor shall penetrate the hardened concrete surface and form a protective layer on the reinforcement. It shall have the following properties:
 - a. The product shall not change the substrate's color, appearance, or texture.
 - b. Penetration (SNMS Analysis): 1/10 to 4/5 inches/day.
 - c. Coating thickness (XPS and SIMS Analysis): 100 to 1000 angstroms.
 - d. Corrosion Current Reduction (ASTM G 109): 65 percent at one year.
 - e. Chloride Displacement (XPS and SIMS Analysis): Passes.
 - f. Effectiveness in Carbonated Conditions (Electrochemical): passes.
 - g. The product must not form a vapor barrier.
 - h. The product must be environmentally sound.
 - i. Post-application verification (Chromatography Plate Test): Passes.
 - j. Longevity (Ten Year Accelerated Weather Testing): Passes.
 2. Product and Manufacturer: Provide one of the following:
 - a. Sika FerroGard 903, as manufactured by Sika Corporation.
 - b. Or equal.
- C. Protective Slurry Mortar:
 1. Protective slurry mortar shall be a two-component, polymer-modified, cementitious waterproofing and protective slurry mortar. Provide two coats at a rate of 50 sq. ft./gal./coat.
 2. Product and Manufacturer: Provide one of the following:
 - a. Sikatop Seal 107, as manufactured by Sika Corporation.
 - b. Or equal.

PART 3 - EXECUTION

3.1 INSPECTION

- A. Examine areas and conditions under which repair Work is to be installed, and notify ENGINEER, in writing, of conditions detrimental to proper and timely completion of Work. Do not proceed with Work until unsatisfactory conditions have been corrected in a manner acceptable to ENGINEER.

3.2 GENERAL

A. Surface Preparation:

1. The entire area to be repaired shall have all laitance, foreign material, and unsound concrete removed by chipping, abrasive blasting or hydroblasting. The surface shall be further roughened as specified herein. Where non-shrink grout or repair mortar is used, any additional surface preparation steps recommended by the manufacturer shall be performed.
2. Where repair concrete, shotcrete, or cement grout is used, and a bonding agent is not required, or where the repair mortar or non-shrink grout manufacturer recommends a wet or saturated surface, water shall be delivered to the surface continuously for a minimum of four hours. Where large surface areas are to be repaired, fog spray nozzles mounted on stands shall be provided in sufficient numbers such that the entire surface to be repaired is in contact with the fog spray cloud. The concrete shall be prevented from drying until after the repair operation is completed. Unrepaired surfaces shall be rewetted by water spray on at least a daily basis. Should more than four days elapse without rewetting the unrepaired surfaces, the original saturating procedure shall be repeated. All standing water in areas to be repaired shall be removed prior to placement of repair material. Means to remove excess water from the structure shall be provided.
3. Where the repair material manufacturer recommends the use of an epoxy-bonding agent, the recommendations of both the repair material and bonding agent manufacturers shall be followed.

B. Care shall be taken to fully consolidate the repair material, completely filling all portions of the area to be filled.

C. The repair surface shall be brought into alignment with the adjacent existing surfaces to provide a uniform, even surface. The repair surface shall match adjacent existing surfaces in texture and shall receive any coatings or surface treatments which had been provided for the existing surface.

D. Curing:

1. Curing of repair mortar and non-shrink grout shall be according to the manufacturer's recommendations except that the minimum cure period shall be three days.
2. Curing of other materials shall be according to Section 03300, Cast-in-Place Concrete.

3.3 TREATMENT OF SURFACE DEFECTS

A. Surface defects are depressions in a concrete surface which do not extend all the way through the member. The depressions can result from the removal of an embedded item, the removal of an intersecting concrete member, physical

damage, unrepaired rock pockets created during original placement, or spalls from corroded reinforcing steel or other embeds.

B. Preparation:

1. All loose, damaged concrete shall be removed by chipping to sound material.
2. Where existing reinforcing bars are exposed, concrete shall be removed to a minimum of 1-inch all around the bars. If the existing bars are cut through, cracked, or the cross sectional area is reduced by more than 25 percent, the ENGINEER shall be notified immediately.
3. The perimeter of the damaged area shall be score cut to a minimum depth of 0.5-inch and a maximum depth to not cut any existing reinforcing steel. Existing concrete shall be chipped up to the score line so that the minimum thickness of repair mortar is 0.5-inch.

C. Repair Material:

1. Repair of surface defects in members, which are normally in contact with water or soil, or in the interior surfaces of enclosed chambers that contain water shall be made only with repair mortar.
2. Repair of other surface defects may be by the application of repair mortar, repair concrete, shotcrete, or cement grout, as appropriate.

3.4 PATCHING OF LINED HOLES

- A. This Section applies to those openings which have embedded material over all or a portion of the inside edge. Unless indicated to remain in place on the Drawings or by the ENGINEER, such embedded materials shall be removed and the remaining hole repaired as specified above. The requirements for repairing holes in concrete specified above shall apply as modified herein.
- B. Where embedded material is allowed to remain, it shall be trimmed back a minimum of 2-inches from the concrete surface. The embedded material shall be roughened or abraded to promote good bonding to the repair material. Any substance that interferes with good bonding shall be completely removed.
- C. Any embedded item that is not securely and permanently anchored into the concrete shall be completely removed.
- D. Embedded items which are larger than 12-inches in their least dimension shall be completely removed, unless they are composed of a metal to which reinforcing steel can be welded. Where reinforcement is required, it shall be welded to the embedded metal.
- E. The following additional requirements apply to concrete members which are in contact with water or soil.
 1. Lined openings, which are less than 4-inches in their least dimension, shall be filled with epoxy grout.

2. Lined openings which are greater than 4-inches, but less than 12-inches in their least dimension, shall be coated with an epoxy bonding agent prior to being filled with Class I non-shrink grout.
3. Lined openings which are greater than 12-inches in their least dimension shall be coated with an epoxy bonding agent and shall have a hydrophilic rubber waterstop or bead of hydrophilic sealant installed to the interior of the opening at the wall centerline, as required by Section 03251, Concrete Joints, prior to being filled with any approved repair material.

3.5 REPAIR OF DETERIORATED CONCRETE

- A. This Section pertains to concrete which has been damaged due to corrosion of reinforcing steel, physical damage due to abrasion, and damage due to chemical attack. The only material acceptable for surface repair is repair mortar as specified herein. Where the repaired surface is to be subsequently covered with a PVC liner material, the finishing details shall be coordinated with the requirements of installing the liner material.
- B. Surface Preparation:
 1. All loose, broken, softened, and acid contaminated concrete shall be removed by abrasive blasting and chipping down to sound, uncontaminated concrete.
 2. When the removal of deteriorated concrete is completed, CONTRACTOR to notify the ENGINEER, in writing. Two days shall be scheduled for the ENGINEER to inspect the surface, perform testing for acid contamination, determine if additional concrete must be removed, and to develop any special repair details that may be required. Should it be determined that additional concrete must be removed to reach sound, uncontaminated material, another two week period shall be scheduled for further evaluation after the end of the additional removal.
 3. Additional surface preparation shall follow the recommendations of the repair mortar manufacturer.
 4. Isolated areas of exposed reinforcing bars shall be treated as required for repair of surface defects. If extensive areas of reinforcement are uncovered after removal of deteriorated concrete, repair methods shall be as determined by the ENGINEER.
- C. Repair Mortar Placement:
 1. The procedures recommended by the manufacturer for the mixing and placement of the repair mortar shall be followed.
 2. After the initial mixing of the repair mortar, additional water shall not be added to change the consistency should the mix begin to stiffen.
 3. Repair mortar shall be placed to a minimum thickness as recommended by the manufacturer, but not less than 0.50-inch. Where removal of deteriorated concrete results in a repair thickness of less than 0.5-inch to return to original concrete surface location in isolated areas totaling less than ten percent of the

total repair area, additional concrete shall be removed to obtain the 0.5-inch thickness. Where the area with repair thickness of less than 0.5-inch exceeds ten percent of the total repair area, notify the ENGINEER. In any case, repair mortar shall be added so that the minimum cover over existing reinforcing steel is 2-inches. Do not place repair mortar so as to create locally raised areas. Where there is a transition with wall surfaces which are not in need of repair, the repair mortar shall not be feathered at the transition. A score line shall be sawcut to not less than the minimum repair mortar depth and concrete chipped out to it to form the transition. Care shall be taken to not cut or otherwise damage any reinforcing steel.

4. The repair mortar shall be placed to an even, uniform plane to restore the member to its original surface. Tolerance for being out of plane shall be such that the gap between a 12-inch straight edge and the repair mortar surface does not exceed 0.125-inch and the gap between a 48-inch straight edge and the repair mortar surface does not exceed 0.25-inch. This shall apply to straight edges placed in any orientation at any location.

D. Finishing:

1. The repair mortar shall receive a smooth, steel trowel finish.
2. When completed, there shall be no sharp edges. All exterior corners, such as at penetrations, shall be made with a 1-inch radius. All interior corners shall be square except corners to receive PVC lining shall be made with a 2-inch repair mortar fillet.

E. Curing:

1. Curing shall be performed as recommended by the repair mortar manufacturer, except that the cure period shall be at least 24 hours and shall be by means of a continuous fog spray. If the manufacturer recommends the use of a curing compound, no material shall be used that would interfere with the bond of the protective coating system or adhesive used for placing PVC lining, where required.

3.6 TREATMENT OF EXPANSION JOINT REPAIR

- A. Surfaces to be repaired shall have all laitance, foreign material, and unsound concrete removed by chipping, abrasive blasting or hydroblasting.
- B. Follow all other surface preparation and application specifications as recommended by manufacturer.

3.7 EXPOSED REBAR REPAIR

- A. The entire area to be repaired shall have all corrosion, foreign materials, and unsound concrete by means of abrasive blasting or hydroblasting.

- B. Surface shall be visually dry before application of the corrosion inhibitor. The corrosion inhibitor shall be placed liberally to achieve 100 sq ft/gal coverage in two or more coats by allowing it to soak into the substrate. The re-coat time between coats shall be a minimum of one hour. Apply by use of rollers, brushes, or hand-pressure spray equipment.
- C. After the last coat of the corrosion inhibitor is applied, a minimum curing time of 24 hours is required.
- D. High Pressure Wash all surfaces to remove filmy residue that is left on the surface by the corrosion inhibitor. Residue acts like bond breaker and must be removed before mortar coating.
- E. For mortar coating, refer to Paragraph 3.5.C, Repair Mortar Placement, Paragraph 3.5.D, Finishing, and Paragraph 3.5.E, Curing.

3.8 FIELD QUALITY CONTROL

- A. CONTRACTOR to provide will employ a testing laboratory to perform field quality control testing. ENGINEER will direct the CONTRACTOR on the number of standard compression tests and specimens required as specified below, under the direct inspection by ENGINEER. Furnish all necessary assistance required by ENGINEER. Provide all labor, material and equipment required including rods, molds, thermometer, curing in a heated storage box, and all other incidentals required. Above will be subject to approval by ENGINEER. Furnish all necessary storage, curing, and transportation required by the testing.
- B. Field tests of cement-based grouts and repair mortar:
 - 1. Compression test specimens will be taken during construction from the first placement of each type of mortar or grout, and at intervals thereafter as selected by the ENGINEER to ensure continued compliance with these specifications. The specimens will be made by the ENGINEER or its representative.
 - 2. Compression tests and fabrication of specimens for repair mortar and non-shrink grout will be performed as specified in ASTM C 109. A set of three specimens will be made for each test. Tests shall be made at 7 days, 28 days, and additional time periods as appropriate.
 - 3. All material, already placed, which fails to meet the requirements of these specifications, is subject to removal and replacement at the cost of CONTRACTOR.
 - 4. The cost of all laboratory tests on mortar and grout will be borne by the CONTRACTOR to provide and CONTRACTOR shall obtain specimens for testing. The cost of any additional tests and investigation on Work performed which does not conform to the requirements of the specifications

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will be the CONTRACTOR's responsibility. Supply all materials necessary for fabricating the test specimens.

- C. Repair Concrete: Repair concrete shall be tested as required in Section 03300, Cast-in-Place Concrete.

++ END OF SECTION ++

SECTION 05581

FLOOR ACCESS HATCH COVERS

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Scope:
1. Provide all labor, materials, equipment and incidentals as shown on the Drawings, specified and required to furnish hinged floor access hatch covers.
 2. The types of floor access hatch covers include the following:
 - a. Aluminum covers with standard angle frame.

1.2 QUALITY ASSURANCE

- A. Manufacturer: All floor covers for the project shall be the product of a single manufacturer. Covers from more than one manufacturer will not be permitted.
- B. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown or specified.
1. ASTM B 209, Specification for Aluminum and Aluminum Alloy Sheet and Plate.
 2. ASTM B 210, Specification for Aluminum and Aluminum Alloy Drawn Seamless Tubes.
 3. ASTM B 221, Specification for Aluminum and Aluminum Alloy Extruded Bars, Rods, Wire, Shapes and Tubes.
 4. NAAM, Metal Finishes Manual and Metal Box Grating Manual.
 5. Aluminum Association Standards.

1.3 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
1. Dimensional plans of all floor covers, quantity schedule, details of fabrication and erection, and anchorage.
 2. Refer to and comply with the requirements of Section 01332, Shop Drawing Procedures.
 3. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures.
 4. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓)

shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

1.4 GUARANTEE

- A. Furnish a written guarantee obtained from the manufacturer. Guarantee shall state the following:
 - 1. Floor covers are to operate properly and be free of defects in material and workmanship for a period of five years from date of purchase.
 - 2. Should any part fail to function, or break in normal use during this period, manufacturer shall furnish a new part at no additional cost to OWNER.

PART 2 - PRODUCTS

2.1 MATERIALS AND FABRICATION

- A. General:
 - 1. Provide Manufacturer's standard fabricated units, modified, if necessary, to comply with the requirements of the Contract Documents. Where standard units are not available for the sizes and types required, custom fabricate units to match manufacturer's similar units.
 - 2. Fabricate each unit in the shop, complete with anchors, gaskets, hardware and accessory items, as required.
- B. Aluminum Covers with Standard Frame:
 - 1. Provide aluminum covers, with standard angle frame, designed to withstand AASHTO H-20 loadings.
 - 2. Access hatches shall be sized per the Contract Drawings for entry and slide gate removal.
 - 3. Provide frame and cover to prevent storm water infiltration
 - 4. Provide Type 316 stainless steel hinges and hardware.

- 5. Product and Manufacturer: Provide one of the following:
 - a. The Bilco Company.
 - b. Or equal.
- C. Door leaves shall be checkered or diamond plate, or other approved non-slip surface.
- D. Frames shall have anchor flanges or strap anchors.
- E. Door shall have hold-open devices.
- F. Door leaves ten square feet and larger shall have torsion bars, springs or other approved means, for counterbalanced operation.
- G. Covers shall have flush exterior locking device with removable handle, and interior turn handle.
- H. All aluminum covers shall be mill finished. Protect finish with a factory applied coating of lacquer standard with the manufacturer.

PART 3 - EXECUTION

3.1 INSTALLATION

- A. Install doors in accordance with approved Shop Drawings and Manufacturer's instructions.
- B. Set doors plumb, level and true to line or grade, without warp or rack, for anchoring under other Sections of these Specifications.
- C. Protection of Aluminum from Dissimilar Materials: Coat all surfaces of aluminum in contact with dissimilar materials such as concrete, masonry, steel and other metals.

+ + END OF SECTION + +

SECTION 06611

FIBERGLASS REINFORCED PLASTIC GRATINGS
AND SUPPORT FRAMING MEMBERS

A. GENERAL

1.2 DESCRIPTION

A. Scope:

1. Provide all labor, materials, equipment and incidentals as shown on the Drawings, specified and required to furnish and install fiberglass grating and support framing members. The Work also includes:
 - a. Providing grating to accommodate the Work under this and other Sections and attaching to the grating and all items such as fasteners and all items required including embedded angles for which provision is not specifically included under other Sections.
 - b. Providing Fiberglass Reinforced Plastic (FRP) angles where shown on the Drawings or required for embedment in concrete or support of gratings.
 - c. Providing support framing members as required.
2. Comply with the requirements of Section 03252, Anchorage in Concrete and Adhesive Anchors.

B. Coordination:

1. Review installation procedures under this and other Sections and coordinate installation of items that must be installed with or before the fiberglass reinforced plastic gratings, treads and support framing members Work.

1.3 QUALITY ASSURANCE

A. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown on the Drawings or specified.

1. ASTM D 635, Test Method for Rate of Burning and/or Extent and Time of Burning of Plastics in a Horizontal Position.
2. ASTM D 638, Test Method for Tensile Properties of Plastics.
3. ASTM D 790, Test Methods for Flexural Properties of Unreinforced and Reinforced Plastics and Electrical Insulating Materials.
4. ASTM D 2344/D 2344M, Test Method for Short-Beam Strength of Polymer Matrix Composite Materials and their Laminates.
5. ASTM D 495, Test Method for High Voltage, Low-Current, Dry Arc Resistance of Solid Electrical Insulation.
6. ASTM D 696, Test Method for Coefficient of Linear Thermal Expansion of Plastics between -30 Degrees C and 30 Degrees C with a Vitreous Silica Dilatometer.

7. ASTM E 84, Test Method for Surface Burning Characteristics of Building Materials.
 8. National Bureau of Standards, PS 15-69, Custom Contact-Molded Reinforced-Polyester Chemical Resistant Process Equipment.
 9. OSHA Federal Register, Volume 39, No. 125, Section 1910.23.
 10. Standards of the Reinforced Plastic/Composites Institute.
 11. 2024 International Building Code
- B. Source Quality Control:
1. Obtain all gratings, treads and support framing members components and accessories from the same manufacturer.
- C. Qualifications:
1. FRP gratings, structural shapes, and all related accessories shall be manufactured and fabricated in North America and the manufacturers should have at least 5 years of experience in manufacturing the FRP products.
- D. Field Measurements:
1. Take field measurements prior to preparation of Shop Drawings and fabrication where required, to ensure proper fitting of the Work.
- E. Provide manufacturers standard three-year warranty on all FRP products against defects in materials and workmanship.

1.4 SUBMITTALS

- A. Samples: Submit for approval the following:
1. Representative samples of grating, treads and support framing members, fasteners and other finished products. ENGINEER'S review will be for type and finish only. Compliance with all other requirements is the exclusive responsibility of CONTRACTOR.
- B. Shop Drawings:
1. Product technical data including:
 - a. Acknowledgement that products submitted meet requirements of standards referenced.
 - b. Manufacturer's installation instructions.
 - c. Manufacturer's recommendations on reinforcing field cut openings.
 2. Fabrication and/or layout drawings.
 - a. Plan showing profile, location, section and details of each item including anchorage or support system(s).
 - b. Locations and type of expansion joints.
 - c. Materials of construction including shop applied coatings.
 - d. Listing of all accessory items being provided indicating material, finish, etc.

3. Certifications:
 - a. Certification of registered professional engineer's qualifications.
 - b. Certification that all components and systems have been designed and fabricated to meet the loading requirements specified.
 4. Manufacturer's full line of colors available for each component.
- C. Informational Submittals:
1. Complete design calculations of all supporting structure and fastening conditions.
 - a. Design calculations to be for information only meeting specification requirements.
- D. The Work shall not be fabricated until CONTRACTOR submits field measurements of the openings and until the manufacturer's drawings based upon CONTRACTOR'S measurements have been approved by ENGINEER.

1.5 DELIVERY, STORAGE, AND HANDLING

- A. Deliver and handle each item to preclude damage.
- B. Store all items on skids above ground. Keep free of dirt and which will damage items or finish and protect from corrosion and UV exposure.

PART 2 - PRODUCTS

2.1 MANUFACTURERS

- A. Subject to compliance with the Contract Documents, the following manufacturers are acceptable:
 1. Grating and solid plate:
 - a. American Grating
 - b. Enduro Composites.
 - c. Fibergrate Composite Structures, Inc.
 - d. Seasafe, Inc.
 - e. Strongwell Corporation.
 2. Structural shapes:
 - a. American Grating
 - b. Enduro Composites.
 - c. Fibergrate Composite Structures, Inc.
 - d. Strongwell Corporation.

2.2 MATERIALS

- A. Fiberglass Reinforced Plastic (FRP):

1. Flame Retardant Vinyl Ester Resin System.
 2. Resin type shall be confirmed and certified by manufacturer to be appropriate for in-place chemical exposure.
 3. Color: Safety yellow.
- B. Fasteners, Clips, Saddles, and Miscellaneous Components:
1. Fiberglass where possible.
 2. Type 316 stainless steel may be used if fiberglass component is not available.
- C. Adhesive: Recommended by manufacturer.
- D. Skid-resistant Surfacing: Manufacturer-applied abrasive grit coating.
- E. All pultruded grating shall be constructed of strand roving, transverse mat, and a synthetic surface veil, including ultraviolet (UV) light inhibitors.
- F. Gratings shall be reinforced with continuous strand roving, not exceed 35% of grating weight in order to provide optimal corrosion resistance and impact strength.

2.3 PERFORMANCE CRITERIA

- A. The manufacturer shall furnish grating and supports to conform to the following criteria:
1. General Use Grating:
 - a. Design Loads: Uniformly Distributed Load of 300 psf and 2,000 lbs. Concentrated Load on 12-inch by 12-inch area.
 - b. Covered rectangular Mesh Grating:
 - 1) Pattern: 1.5-inch on center maximum.
 - 2) Thickness: 1.5-inches minimum.
 - 3) Fine grit surface
 - c. Maximum Clear Span Deflection (under uniform load) 1/200 of span or 1/4-inch at four-feet, whichever is smaller.
 - d. Fire Resistance: 25, ASTM E 84; self-extinguishing ASTM D 635.
 - e. Chemical resistance.
 - 1) System shall be capable of withstanding splash and spillage of the following chemicals:
 - a) Hydrogen Sulfide Dry/Wet Gas at 115 deg F.
 - f. Sheet size: Grating sheet size shall be sized to be removed from access hatch, CONTRACTOR to coordinate structure access hatch size to determine maximum grating sizes. Grating shall be 5-ft square maximum to allow for removal due to weight, manufacturer shall design support system based on the 5-ft maximum sheet size requirements for the entire grating area as shown per the Drawings.

- B. The manufacturer shall furnish pultruded structural shapes to have the minimum longitudinal mechanical properties listed below:
1. Tensile Strength: ASTM D 638, 30,000 (206) psi, (MPa).
 2. Tensile Modulus: ASTM D 638, 2.5×10^6 (17.2) psi, (GPa).
 3. Flexural Strength: ASTM D 790, 30,000 (206) psi, (MPa).
 4. Flexural Modulus: ASTM D 790, 1.8×10^6 (12.4) psi, (GPa).
 5. Flexural Modulus (Full Section): ASTM D 790, 2.8×10^6 (19.3) psi, (GPa).
 6. Short Beam Shear (Transverse): ASTM D 2344/D 2344M, 4,500 (31) psi, (MPa).
 7. Shear Modulus (Transverse): 4.5×10^5 (3.1) psi, (GPa).
 8. Coefficient of Thermal Expansion: ASTM D 696, 4.4×10^{-6} (8.0×10^{-6}) in/in/°F (cm/cm/°C).
 9. Flame Spread: ASTM E 84, 25 or less.

2.4 FABRICATION

- A. Use materials of the minimum size and thickness as specified above, unless otherwise shown on the Drawings. Work to the dimensions shown on approved Shop Drawings.
- B. Grating and support framing system shall be provided and designed by the grating manufacturer, and shall be coordinated with the piping and electrical conduit locations, and shall comply with the standards of the Reinforced Plastic/Composites Institute, except as specified herein.
1. Grating manufacturer shall provide structural drawings for the grating support system stamped by a licensed structural engineer in the state of Arizona.
 2. Grating support system shall be fiberglass reinforced plastic with stainless steel Type 316 anchor bolts and nuts.
- C. Grating Fabrication:
1. Grating supplied shall meet the dimensional requirements and tolerances as shown on the Drawings or specified. Provide field measurements for the Work fabricated to conform to field conditions.
 2. Each grating section shall be readily removable, except where shown on the Drawings. Gratings shall be fabricated free from warps, twists, or other defects which affect appearance and serviceability.
 3. All shop fabricated grating cuts shall be coated with vinyl ester resin to provide maximum corrosion resistance.
 4. Provide Type 316 stainless steel hold-down clips spaced as recommended by the manufacturer.
- D. Support Framing Member Fabrication:
1. Structural grating shapes supplied shall be 5-ft square maximum and meet the minimum dimensional requirements as shown on the Drawings or specified.

Provide field measurements for the Work fabricated to conform to field conditions. Determine correct size and locations of required holes or coping from field dimensions prior to structural shape fabrication.

2. All shop fabricated cuts or drilling shall be coated with vinyl ester resin to provide maximum corrosion resistance.
3. Provide Type 316 stainless steel bolts.

PART 3 - EXECUTION

3.1 INSTALLATION

- A. Fastening to In-Place Construction: Provide Type 316 stainless steel anchorage devices and fasteners to secure grating to supporting members or prepared openings, as recommended by the manufacturer or as shown on the Drawings.
- B. Cutting, Fitting and Placement:
 1. Perform all cutting, drilling and fitting required for installation. Set the Work accurately in location, alignment and elevation, plumb, level, true and free of rack. Do not use wedges or shimming devices.
 2. Make cutouts or openings in the field as approved by ENGINEER.
 3. All grating which requires cutting during installation shall have the effected surfaces sealed with catalyzed resin sealant of equal or superior corrosion resistance to the grating as recommended by the manufacturer.
 4. Secure grating, to fiberglass structural members, as specified by the manufacturer.
 5. Fit exposed connections accurately together to form tight joints.
 6. Secure edges of grating to each other with end panel clips.
 7. Secure clips to grating with Type 316 stainless steel bolts such that the grating shall act as a unit, or as shown on the Drawings. Place bolts not more than 3-inches from each plate section end and not more than 24-inches on center, or as shown on the Drawings.
- C. Installation of Grating:
 1. Install gratings in accordance with manufacturer's assembly drawings. Lock grating panels securely in place with hold-down fasteners as specified herein. Follow manufacturer's instructions when cutting or drilling fiberglass products or using resin products. Provide adequate ventilation.

++ END OF SECTION ++

SECTION 06614

FIBERGLASS REINFORCED PLASTIC STOP LOGS

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Scope:
 - 1. Provide all labor, materials, equipment and incidentals as shown on the Drawings, specified and required to furnish FRP stop logs, FRP guide frame, and lifting device.
- B. Related Sections:
 - 1. It is the CONTRACTOR's responsibility for scheduling and coordinating the work or subcontractors, suppliers, and other individuals or entities performing or furnishing any of the CONTRACTOR's work.
 - 2. The following Sections are related to the Work described in this Section. This list of Related Sections is provided for convenience only and is not intended to excuse or otherwise diminish the duty of the CONTRACTOR to see that the completed Work complies accurately with the Contract Documents.
 - a. Section 03252, Anchorage in Concrete.

1.2 QUALITY ASSURANCE

- A. Equipment shall be the latest design of a manufacturer engaged in the production of equipment of this type with a minimum of 20 years experience designing and manufacturing stop logs.
- B. Frames, logs, and beams shall be by one manufacturer.
- C. Manufacturer must provide warranty for 25 years against failure due to corrosion.

1.3 REFERENCES

- A. Comply with applicable provisions and recommendations of the following, except as otherwise shown or specified.
 - 1. ASTM A193, Stainless Steel Anchor Bolts
 - 2. ASTM A276, Stainless Steel Bars
 - 3. ASTM D256, Izod Impact Strength
 - 4. ASTM D570, Water Absorption Rate
 - 5. ASTM D638, Tensile Strength
 - 6. ASTM D695, Compressive Properties of Rigid Plastic
 - 7. ASTM D696, Coefficient of Linear Expansion
 - 8. ASTM D790, Flexural Properties
 - 9. ASTM D1056, Polymer Grade

10. ASTM D2583, Indentation Hardness
11. ASTM D2563-0, Visual Defects
12. ASTM D2584, Resin, Glass & Filler Content
13. AWWA C-563, Leakage Rate

1.4 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
1. Shop drawings with complete description of all materials including the material thickness of all structural components of the stop logs, guide frames and stop log lifting assembly.
 2. Manufacturer's installation instructions showing all details of construction, details required for installation, dimensions, and anchor bolt locations.
 3. Design calculations: Submit calculations and design data to substantiate conformance with requirements indicated on the Drawings and specified in the specifications, including stop log load and deflection criteria.
 4. Certificates: Submit manufacturer's certificates reporting the result of leakage tests performed on stop logs.
 5. Refer to and comply with the requirements of Section 01332, Shop Drawing Procedures.
 6. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures.
 7. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

1.5 DESIGN REQUIREMENTS

- A. Design, fabricate, and test stop log or stop gate system and materials in accordance with the manufacturer's procedures and assuming full pipe flow.
- B. Stop log or slide gate systems shall meet AWWA C-563 leakage requirements.

1. Maximum allowable leakage of stop logs with seating head shall not exceed 0.10 GPM/FT of wetted linear seal area under full design head when installed in manufacturer's Guide Frames.
- C. Stop log or slide gate panel shall be fiberglass reinforced polymer (FRP).
- D. Composition of the stop log laminate shall be in accordance with the recommendations shown in the Quality Assurance Report for Reinforced Thermoplastic (RTP) Corrosion Resistant Equipment prepared under the sponsorship the Society of the Plastics Industry, Inc. (SPI) and the Material Technology Institute of the Chemical Process Industries, Inc. (MTI) for "Hand Lay-up Laminates" and shall meet the specifications for Type 1, Grade 10 laminates shown in Appendix M-1 of said report.
 1. Visual inspection for defects shall be made without the aid of magnification and defects shall be classified as shown in Table 1 Level II of ANSI/ASTM D2563-0, approved 1977, (or any subsequent revision).
- E. Deflection
 1. Deflection across the stop log width shall be limited to: L/360 or 1/4" (6mm), whichever is less, at the maximum operating head.
- F. Head Pressure
 1. Stop log system shall be designed for a maximum head pressure as per gate schedule.
- G. Stop log panel size as shown on the contact drawings and/or gate schedule.
- H. Surface Conditions
 1. All stop log panels shall be flat and level.
 2. Warpage throughout the entire stop log panel shall not produce a crown of more than 1/16" (1.6mm) in any direction.

PART 2 - PRODUCTS

2.1 MANUFACTURERS

- A. Manufacturer: One of the following or equal:
 1. Plasti-Fab, Inc.
 2. Approved equal

2.2 MATERIALS

- A. FRP stop logs or slide gate shall meet the following requirements:
 1. FRP resin shall be vinyl ester.
 2. Guide frames to be vinyl ester FRP.
 3. Lifting pins to be T-316L stainless steel.
 4. Anchor bolts to be T-316L stainless steel.

5. Lifting beam to be T-316L stainless steel.
6. Storage rack(s) to be vinyl ester FRP.
7. Seal material shall be EPDM.
8. The stop logs shall be free of any cracks, crazing, blistering, chips, pits, dry spots, fisheyes, burned areas, entrapped air, surface porosity, exposed glass or steel reinforcing, and foreign material.
9. The stop logs shall not have any wrinkles or solid blisters exceeding 1/8 inch.
10. Guide frame shall be embedded to precast manhole base as part of the manhole base casting manufacturing process.

B. Physical Properties

1. Structural characteristics for FRP glass mat laminates shall meet the following minimum physical properties:

Tensile strength	15,000 psi (1034 ksc)
Flexural Modulus	1,000,000 psi (70307 ksc)
Flexural Strength	20,000 psi (1406 ksc)
Compressive Strength	22,000 psi (1547 ksc)
Impact Strength	9.0 ft-lbs/in. (1.24
Water absorption	0.13% (in 24 hours)

2. Seals: Extruded EPDM seals shall have the following physical characteristics:

Specific Gravity	1.25
Hardness	55 – 65 Shore A Durometer
Tensile Strength	1500 psi min. (0.07ksc)
Elongation	300%
Low temperature brittleness	- 40°

2.3 FABRICATION

A. Stop logs:

1. The stop log shall be fabricated by means of vacuum infusion so as to totally encapsulate the internal structural matrix and protect it against corrosion from moisture or chemical deterioration with a minimum thickness of ¼ inch (6mm) FRP on the front and back facings, and ¾ inch (19mm) FRP on the remaining perimeter. Stop logs shall be designed so that the maximum fiber stress (ultimate or yield, whichever applies) shall exceed 2.5 times the working stress. Stop logs shall be suitably reinforced to withstand the maximum seating head with a deflection less than $\ell/360$ of the stop log width, or ¼ inch (6mm), whichever is less. Stop log covers that are fabricated from pressed or laminated sheet material and/or glued/bonded to a substructure shall not be acceptable. No seams or joints that may allow water intrusion will be acceptable. Each Stop Log shall be molded individually to the exact dimensions specified.
2. Stop Log shall be manufactured of reinforced thermoset plastic in the form of FRP.
3. Stop Log shall have UV Stabilizing pigment in the Resin to provide long-term protection from UV.
4. The surface shall be resin rich to a depth of 0.010 inches (2.5mm) to 0.020 inches (5mm) and reinforced with C-glass or polymeric fiber surfacing material.

5. The surface shall be free of exposed reinforcing fibers.
6. The composition of these layers shall be approximately 95% (by weight) resin. The remaining laminate shall be made up of copolymer composite and reinforcing fibers in a form, orientation and position to meet the mechanical requirements.
7. Structural reinforcing shall be utilized to attain the necessary stiffness to meet deflection requirements, and shall be well encapsulated with a laminate not less than 1/4" (6mm) thick on each side to ensure against any permeation by water to the core areas. Internal steel structure to be welded per ASME/ASTM standards, sandblasted and coated with vinyl ester resin immediately prior to vacuum infusion in order to ensure complete bonding with external corrosion barrier.
8. T-316L stainless steel lifting pins shall be attached to the stop log by passing completely through the log. Stainless steel lifting pin shall be fastened to the log with sufficient reinforcing to withstand the lifting force. Lifting pins attached to the surface of the log are not acceptable. The through holes shall not pass through or be in contact with the internal steel reinforcing.
9. Core material must be 100% resistant to decay and attack by fungus and bacteria and be resistant to hydrocarbons.
10. To assure maximum service life, the copolymer composite shall be ultraviolet stabilized and seamless to protect inner structural members from corrosion.
11. Metal, concrete, or wood stop logs subject to corrosion/bacterial breakdown/rot shall not be acceptable alternatives to composite FRP material.
12. Stop Log panels shall be manufactured using advanced technology vacuum infusion resin transfer processes. The closed mold vacuum process must completely evacuate all air from the mold prior to infusing the mold with premium quality resin as specified. The vacuum infusion process must eliminate the potential of air entrapment and/or voids in the matrix of the stop log panel (which cause defects and performance-detracting irregularities) thus producing a finished product that is one-piece, seamless and uniformly impenetrable by fluids eliminating interior corrosion. Stop Logs produced by techniques that employ adhesives or mechanical fasteners to attach individual panels to a pre-fabricated framework resulting in seams along vertical and horizontal axes of the stop log which create stress-potential areas, portals for fluid infiltration, subsequent de-lamination and product failure due to corrosion.

B. Seals

1. The stop logs shall be equipped with elastomeric bottom seals to seal between the logs. Seals shall be made of molded EP, per ASTM D- 2000 having a hardness of 55 – 65 Shore A Durometer, conforming to ASTM D-2000, with a maximum compression set of 25%, and low temperature brittleness to meet suffix F-17 (- 40oF/C).

C. Guide Frames

1. Guide frames shall be styled for embedded guide frames as shown on the contract drawings and/or stop log schedule.
2. Guide frames shall be fabricated from vinyl ester FRP and shall have a slot suitable for mating with the stop log panels.

2.4 PORTABLE CRANE

- A. Product and manufacturer: Provide one of the following:
 - 1. Thern
 - a. Davit Crane Commander 2000; Model 5PT20-E2 must have extended mast (5-inches) and hydraulic cylinder to be able to adjust hook reach under load
 - b. Galvanized finish
 - c. Flush Mount Base; Model 5BF20
 - d. Base Extension; Model 5BE20-15
 - e. 304 Stainless Steel Wire Rope; Model WS25-45NS
 - f. 304 Stainless Steel Base Anchor Kit; Model AN50A-5S
 - 2. Approved equivalent
- B. Qualified manufacturer representative shall provide one, four (4) hour training session for city operations staff.
- C. Manufacturer shall provide a certification of proper installation and provide copies to the OWNER, ENGINEER, CONTRACTOR, and manufacturing facility.

PART 3 - EXECUTION

3.1 INSTALLATION

- A. Thoroughly clean and remove all shipping materials prior to setting.
- B. Install stop log systems per Manufacturer's recommendations.
- C. Manufacturer shall provide a certification of proper installation and provide copies to the OWNER, ENGINEER, CONTRACTOR, and manufacturing facility.
- D. Qualified manufacturer representative shall provide one, four (4) hour training session for city operations staff.
- E. Testing:
 - 1. After stop logs are installed, conduct field leakage tests at specified operating conditions.
 - 2. The manufacturer shall certify in writing that the field leakage test is within the allowable leakage limits.

CITY OF CHANDLER: Public Works and Utilities Department
PROJECT NAME: Loop 101 and 202 Freeway 66-inch Redundant Sewer Project
PROJECT NUMBER: WW2402.401

<u>Stop Log Schedule</u>						
Reference Drawing	Width of Opening (Inches)	Height of Each Log (Inches)	Number of Logs	Design Pressure, Seating, Feet	Design Pressure, Unseating, Feet	Remarks
M-2.0/ M-2.2	74	24 12	3 1	7	7	Each pipe opening to have three (3) 24" tall logs and one (1) 12" tall log
M-3.0/ M-3.6	74	24 12	3 1	7	7	Each pipe opening to have three (3) 24" tall logs and one (1) 12" tall log

++ END OF SECTION ++

SECTION 09910

SPECIAL FINISHES

PART 1 - GENERAL

1.1 SUMMARY

- A. CONTRACTOR shall provide all labor, materials, equipment, services, and incidentals required to furnish and install corrosion resistant coatings for the sanitary sewer manhole rehabilitation as specified herein.

1.2 SECTION INCLUDES

- A. Corrosion resistant coating.

1.3 SUBMITTALS

- A. Submit the following for approval:
1. Letters of Certification of Compliance on materials, equipment, etc.
 2. Identify product name and number, manufacturer and local distributor name, address, and telephone number.
 3. Samples.
 4. Operating and/or service manuals, and all other data pertinent to operating or servicing the complete apparatus. Include detailed procedures for light repairs such as dents and scratches.
 5. Manufacturer's installation methods:
 - a. Safety data sheets (SDS)
 - b. Maximum storage life and storage requirements
 - c. Mixing and proportioning requirements
 - d. Environmental requirements for application and worker safety, including ventilation, humidity, and temperature ranges
 - e. Application film thickness PM coat of primer and finish coat
 - f. Curing time required
 6. Identify manufacturer's recommended, published spreading rate and dry film thickness of each coating material proposed for use.
 7. Mixing instructions, shelf life, method of application, recommended number of coats, and drying time.
 8. General bulletins and catalog cuts describing complete coating system, including all principles and fundamentals.
 9. Materials of construction of all components.
 10. Material gradation, design mix, job mix formula, and/or material analysis.
 11. Copies of the CONTRACTOR'S proposed protection procedures in each area of Work.
 12. Sample of finished product showing final color: Coating shall be light in color.

13. A written certification of the applicator's qualifications to the ENGINEER for approval. Applicator shall be an Arizona licensed contractor. Each of the applicator's employees applying coatings and underlayment's shall be certified by the manufacturer as having sufficient training and knowledge to properly apply their products
14. Quality Assurance and Testing Reports.

1.4 PRODUCT DELIVERY, STORAGE, AND HANDLING

- A. All surface coating materials shall be delivered to the job site in original sealed containers and shall be used directly from these sealed containers.
- B. All containers shall have the following labels naming title of material, manufacturer's name and stock number, contents by volume, color name and number, and application instructions.
- C. All surface coating materials shall be stored in an area according to manufacturer's recommendations.
- D. The storage area shall be kept neat and clean. Floors and other surfaces shall be adequately protected. Any damage caused by the CONTRACTOR to surfaces in the storage area shall be thoroughly cleaned and repaired.
- E. All oily rags, waste, etc., shall be placed in a covered material container and removed by the CONTRACTOR at the end of each shift.

1.5 QUALITY ASSURANCE

- A. Corrosion resistant coating manufacturer and applicator shall have at least five years experience in production and coating applications, respectively.
- B. Certify long-term compatibility of all coatings with all substrates.
- C. Provide the services of a qualified manufacturer's representative at the Project site for a minimum of two trips. Within seven calendar days after inspection by the manufacturer, the CONTRACTOR shall provide a written report from the manufacturer certifying the coatings have been applied properly and in accordance with the manufacturer's recommendations and requirements. Deficiencies in the coating system, if any, noted by the manufacturer during final inspection shall be defined in the manufacturer's report including corrective measures to be implemented by the CONTRACTOR at the CONTRACTOR'S expense. Following corrective measures by the CONTRACTOR, the manufacturer shall re-inspect the Work. Within seven days after re-inspection, provide a written report from the manufacturer certifying the coatings have been applied properly and in accordance with the manufacturer's recommendations and requirements.

- G. Maintain a Coating Application Log containing the information. The Paint Application Log shall be maintained on a daily basis for all areas where the Work is being performed. The Paint Application Log shall be turned over to the ENGINEER the following day that the work was performed. The log shall include the following:
1. Date.
 2. Time.
 3. Weather condition (at work location).
 4. Air temperature (at work location).
 5. Surface temperature (at work location).
 6. Dew point (at work location).
 7. Humidity (at work location).
 8. Material temperature (Before (Separately) and Mixed (Combined))
 9. Location/area square footage.
 10. Description of Work performed.
 11. Materials used, colors and batch numbers, quantity of materials used (not including waste).
 12. Application/surface preparation equipment and personnel.
 13. WFT/surface profile measurements.
 14. Comments, quality control procedures.
 15. Signature/title.

1.6 WARRANTY

- A. The corrosion resistant coating manufacturers shall warrant the products being supplied to the OWNER against defects for a period of five years under normal use. The warranty shall be in printed form.
- B. Warranty Period Inspection:
1. OWNER may conduct inspection at any time prior to five (5) years following completion of new coating work and /or repaired coating work. CONTRACTOR and representative of coating manufacturer shall be notified of any apparent coating failures. Defective work or coating failures shall be repaired in accordance with this specification and to the satisfaction of OWNER.
 2. If warranty inspections are not held, CONTRACTOR is not relieved of responsibilities under Contract Documents.

PART 2 - PRODUCTS

2.1 PRODUCTS FOR COATINGS

- A. Sanitary sewer manhole rehabilitation:
1. Sauereisen Sewergard 210TN; Troweled Industrial coating

2. Sewer Shield 100
3. Or equal.

2.2 MATERIALS

- A. Rehabilitated Sewer Manholes:
 1. Product shall be applied in one layer to a minimum of 250 mils dry film thickness after the concrete filler/sealer has been applied (or as required by the coating manufacturer and approved by the ENGINEER) to the concrete surfaces. In floor coatings, CONTRACTOR shall incorporate silica sand or other approved natural or synthetic grit to provide a slip-resistant surface. The additive shall be mixed in the coating compound prior to application.
 2. Product shall be applied in combination with FRP per specification 02720 - Fiber Reinforced Polymer

PART 3 - EXECUTION

3.1 SAFETY

- A. The CONTRACTOR shall perform all surface preparation and application of the coatings as described below. All surface preparation and coating application shall be in compliance with all applicable Federal, State, and local occupational safety, health, and air pollution control regulations. All safety precautions recommended by the manufacturer in printed instructions or special bulletins, and as required by applicable regulations. The CONTRACTOR shall provide forced ventilation in all areas where inadequate ventilation exists.

3.2 SURFACE PREPARATION

- A. New concrete shall cure as required for an acceptable surface to apply coating system as determined by the coating manufacturer. Testing the new concrete to determine whether it is acceptable for a coating system includes capillary moisture in accordance with ASTM D4263 and further testing in accordance with ASTM F1869 if moisture is detected and ground water intrusion is suspected. Test for moisture content of concrete surfaces before commencement of painting Work. There are many tests for moisture in concrete such as ASTM D 4263, ASTM F 1869 or ASTM F 2170. Provide from the submitted coating manufacturer in writing as to which standard they want used and what results must be obtained for satisfactory compliance. Report results to ENGINEER before starting Work.
- B. The concrete shall be prepared in accordance with SSPC/NACE Preparation of Concrete. The concrete surface shall be abrasive blast cleaned in accordance with ASTM D4259 to a uniform texture of 50-60 grit sandpaper and a surface profile of 4-6 mils. When sandblasting a surface to be coated, the CONTRACTOR shall

- protect all surrounding surfaces from sustaining any damage. All surfaces to be sound, clean, dry, and free of all contaminants prior to application of the coating system. Patch all holes as recommended by the coating manufacturer.
- C. Test for moisture content of concrete surfaces before commencement of painting Work. There are many tests for moisture in concrete such as ASTM D 4263, ASTM F 1869 or ASTM F 2170. Provide from the submitted coating manufacturer in writing as to which standard they want used and what results must be obtained for satisfactory compliance. Report results to ENGINEER before starting Work.
 - D. Verify that the pH levels, of the cleaned concrete surfaces to be coated, are 10 to 12 or within the coating manufacturers' current written acceptable range. The pH of the concrete substrate will be measured using pH indicating papers. The pH testing is to be performed once every 50 square feet (5 square meters). Acceptable pH values can be measured using color indicating pH paper with readable color calibrations and a scale at whole numbers (minimum). Use Hydrion Insta-Check Jumbo 1-12, or equal. The paper shall be touched to the surface once using moderate gloved finger pressure. The surface shall not be wiped or moved laterally to disturb the surface during pH testing. Following the one touch, lift the paper vertically to not "wipe" the surface. Compare the color indicated with the scale provided and record the pH. Spot check any areas that may be questionable with phenolphthalein solution. Where paint system is used to provide chemical containment barrier protection, repair cracks and expansion joints in concrete and provide 2-inch radius cove base fillets at all equipment pads and containment walls as part of the complete chemical containment paint system Work. Use materials and techniques recommended by the specified Manufacturer.
 - E. Remove all projections, tie wire, nails and other surface irregularities, which would protrude above the level of finished intermediate fillers and surfacers by chipping and scarification by mechanical abrasion.
 - F. Using specified filler and surfacer, patch all concrete and precast concrete surfaces as required to completely fill surface air holes and honeycombing. Level all protrusions and grind filler and surfacing compounds smooth and level with adjacent surfaces.
 - G. Where concrete masonry unit block filler is specified, spot patch holes and cracks with a putty knife using specified block filler to provide smooth finish for large surfaces coating materials can be applied by airless spray and backroll uniformly using a roller with a synthetic nap cover. While the coating material; is still wet a rubber squeegee can be used to provide a smooth finish.

3.3 INSTALLATION

- A. The corrosion resistant coating shall be applied by a coating applicator specialist qualified to apply the coating in accordance with the manufacturer's specification.
- B. Do not open containers or mix components until necessary preparatory work has been completed and application work will start immediately.
- C. Handling of materials before, during, and after application in such a manner as to prevent inclusion of foreign materials.
- D. All interior spaces of the rehabilitated sewer manhole shall be coated as described in the drawings.
- E. Prior to Coating Installation, CONTRACTOR shall perform adhesion testing of the concrete per ASTM C 1583.
- F. Use only application methods approved by the manufacturer.

3.4 TESTING

- A. Contractor shall give ENGINEER a minimum of three days advance notice before the start of any surface preparation, underlayment application work, coating application work or testing.
- B. All testing shall be performed in presence of ENGINEER.
- C. Acceptance for holiday testing, adhesion testing and dry film thickness testing shall be witnessed by an independent Testing Agency or Laboratory approved by the OWNER. Costs of this inspection and Testing shall be the responsibility of the CONTRACTOR. At OWNER's option, OWNER may waive requirement of CONTRACTOR provided testing agency or laboratory and have ENGINEER witness this testing.
- D. Additional illumination, scaffolding, and permit required confined space entry equipment and support shall be provided by CONTRACTOR as necessary to facilitate inspection by ENGINEER and/or Testing Agency when requested.
- E. Holiday testing equipment and procedures shall be in strict accordance with the latest edition of NACE "Standard Recommended Practice-Discontinuity (Holiday) Testing of Protective Coatings." and ASTM D4787-13. Areas containing holidays shall be marked repaired or recoated and re-tested in accordance with coating manufacturer's printed instructions.
 - 1. Holiday detectors shall be high voltage plus-type detectors as manufactured by Tinker & Rasor or D.E.Sterns Co. Unit Shall be adjusted to operate at voltage

required to cause sparks jump across air gap equal to twice specified coating thickness.

- F. Dry film thickness measurements shall be provided by report submitted by CONTRACTOR to ENGINEER. The report shall be presented after completion of underlayment, top coating operations and shall state number of manufacturer's product units used and total square footage of surfaced area covered. ENGINEER shall have option of requiring CONTRACTOR to document number of units (coating materials) on hand before and after coating operations to verify actual minimum dry film thickness applied
 - 1. All film thickness not meeting required minimum will be recoated per manufacturer's recommendations to required minimum 250 mil thickness.
- G. CONTRACTOR shall perform adhesion test on 15% of coated surface areas. Adhesion tests shall conform to ASTM D7234, minimum pull off strength of 200 psi concrete. A minimum pull off strength of 150 psi on concrete will be acceptable if substrate is adhered to coating and dolly on more than ½ the area of the dolly. 50mm dollies shall be used for adhesion testing. In the event of a failure, ENGINEER and CONTRACTOR shall determine limits of failure through additional investigation, sounding and pull tests. Failed areas shall be removed and repaired in accordance with these specification and manufacturer's recommendations. Repaired area shall undergo testing as specified herein.

3.5 CLEAN-UP

- A. During progress of the Work, remove from the site all discarded materials, rubbish, cans or buckets, and rags at the end of each Work day.
- B. Upon completion of the Work, clean all coating-spattered surfaces. Remove spattered coating by proper methods of washing and scraping, using care not to scratch or otherwise damage surfaces.
- C. At the completion of Work of other trades, touch-up and restore all damaged or defaced coated surfaces as determined by the ENGINEER.
- D. Ensure that all Federal, State, and Municipal laws and regulations are met during the disposal of coating material or any other hazardous material associated with the coating process.

++ END OF SECTION ++

SECTION 13210

POLYMER CONCRETE STRUCTURES

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. The purpose for this Technical Specification is to provide the requirements to furnish and install the polymer concrete structure(s). The structure is comprised of a base, barrel, and lid segments as a multi-piece designed unit constructed and installed in a manner consistent with acceptable manufacturing and construction techniques and practices.
2. This Technical Specification covers the work necessary to furnish and install polymer concrete structures. The CONTRACTOR shall be licensed in the State of Arizona. The CONTRACTOR shall deliver polymer concrete structure(s) including all materials, labor, equipment, and services necessary for product installation, all quality controls and samples for performance of required material tests, final inspection and warranty work, all as specified in these Contract Documents. Anticipated work to be provided also includes all traffic control, bypass pumping and/or diversion of sewage flows (as necessary), and removal and replacement of affected existing grade to existing condition or to OWNER's standards, whichever is greater.
3. The prices submitted by the CONTRACTOR shall include all costs of permits, labor, equipment, and materials for the various bid items necessary for furnishing, transporting and installing complete in place, the polymer concrete structure(s), in accordance with these Technical Specifications. All items of work not specifically mentioned herein which are required to make the product perform as intended shall be included in the respective lump sum and/or unit price bid items depicted in the Guaranteed Maximum Price (GMP).

1.2 SAFETY REQUIREMENTS

- A. The CONTRACTOR shall conform to all work safety and health requirements of pertinent regulatory agencies. The CONTRACTOR shall perform existing material testing as required by regulatory requirements and erect such signs and other devices as are necessary and be responsible for safety at the work site.

1.3 NOTIFICATION

- A. It shall be the CONTRACTOR's responsibility to notify all utility companies involved whenever a utility is to be cut, tapped, moved, or in any way disturbed from its original placement. Sufficient notice shall be given to the utility company so that its users can be

informed of any disruption of service. Such notice shall be given no less than 48 hours in advance.

- B. It is the CONTRACTOR's responsibility to inform the city of construction activities which may lead to interruption in sewer service. The CONTRACTOR shall provide 72-hour notice prior to construction in order for the CONTRACTOR to alert all applicable residents.

1.4 QUALITY CONTROL

A. REFERENCES:

1. ASTM C33: Specification for Concrete Aggregates
2. ASTM C273: Standard Test Method for Shear Properties of Sandwich Core Materials
3. ASTM C478: Standard Specification for Precast Reinforced Concrete Manholes Sections (Structural intent only)
4. ASTM C478: Standard Specification for Circular Precast Reinforced Concrete Manhole Sections
5. ASTM C497: Test Methods for Concrete Pipe, Manhole Sections, or Tile
6. ASTM C579: Standard Test Methods for Compressive Strength of Chemical-Resistant Mortars, Grouts, Monolithic Surfacing, and Polymer Concretes
7. ASTM C581: Practices for Determining Chemical Resistance of Chemical Thermosetting Resins Used in Glass-Fiber Reinforced Structures Intended for Liquid Service.
8. ASTM C582: Plastic Laminate
9. ASTM C587: Standard Practice for Minimum Structural Design Loading for Underground Utility Structure
10. ASTM D543: Test Method for Resistance of Plastics to Chemical Reagents
11. ASTM D638: Standard Test Method for Tensile Properties of Plastics
12. ASTM D695: Standard Test Methods for Compressive Properties of RigidPlastics
13. ASTM D790: Standard Test Methods for Flexural Properties of Unreinforced and Reinforced Plastics and Electrical Insulating Materials
14. ASTM D1244: Standard Test Method for Concrete Sewer Manholes by the Negative Air Pressure (Vacuum) Test Prior to Backfill
15. ASTM D1622: Standard Test Method for Apparent Density of Rigid Cellular Plastics
16. ASTM D1623: Standard Test Method for Tensile and Tensile Adhesion Properties of Rigid Cellular Plastics
17. ASTM D2412: Test Method for Determination of External Loading Characteristics of Plastic Pipe by Parallel Plate Loading
18. ASTM D2583: Standard Test Method for Indentation Hardness of Rigid Plastics by Means of a Barcol Impressor
19. ASTM D2842: Standard Test Method for Water Absorption of RigidCellular Plastics

- 20. ASTM D3753: Standard Specification for Glass-Fiber Reinforced Polyester Manholes
- 21. ASTM D6783: Standard Specification for Polymer Concrete Pipe
- 22. AASHTO HS-20: Standard Specification for Highway Bridges Codes - Axle Loading
- 23. AWWA C950: Glass-Fiber Reinforced Thermosetting Resin Pressure Pipe

1.5 SUBMITTALS

- A. The CONTRACTOR shall provide submittal information to the ENGINEER in accordance with Specification Section 01330, Submittals.
- B. Polymer Concrete Structure Manufacturer's Qualifications: The manufacturer of the structure shall be able to show experience in the manufacture of polymer concrete structures for more than five (5) years.
- C. Documentation of CONTRACTOR's Experience: CONTRACTOR shall provide references for all jobs within the last two (2) years that were either completed or under construction using the proposed installation method.
- D. Installer's Qualifications: The installer shall be able to show experience in the installation of the proposed structure or manholes for more than five (5) years. References shall be provided for a minimum of five (5) jobs. Information provided shall include a description of the job, the location of the job, the value of the job, the OWNER name, and the OWNER contact person for the job including name, title, address, and phone number.
- E. Submit Manufacturer's published product and installation literature and published quality control testing criteria data for the proposed structure.
- F. Structural Calculations: The manufacturer shall submit structural calculations sealed by a registered ENGINEER in the State of Arizona, demonstrating the required reinforcement and other materials are adequate to resist the anticipated loading.
- G. Submit independent test report showing that the physical properties of the proposed structure meet the requirements of these Technical Specifications and will meet or exceed the physical properties given in the manufacturer's published literature submitted as required by Part 2 – Products.
- H. Submit a report outlining the proposed installation process and procedures as directed by the structure manufacturer to be used. The report shall include information specific to the installation instructions and any unique site condition(s) that must be taken into consideration including access coordination, installation timing and identification and any coordination requirements/requests from the city or others.
- I. Shop Drawings: CONTRACTOR shall provide shop drawings of proposed materials for approval prior to ordering. Shop drawings shall depict:

1. All measurements made by the CONTRACTOR to verify structure critical dimensions including but not limited to depth and diameter along with any pipe penetrations or equipment hanger requirements.
 2. Material(s) proposed for construction.
 3. Product data sheets, catalog sheets, safety data sheets for components to be provided by a third party to be implemented into the structure.
 4. Critical dimensions, jointing and connections, fasteners and anchors with sizes, spacing, and location, connections, attachments, and openings.
 5. Anticipated grade adjustment ring(s) and height.
 6. Frame and cover (to be city provided corrosion composite frame, cover and rings per Chandler Supplements to MAG Detail No. C-400 and C-401 Type "B").
- J. Warranty: CONTRACTOR shall submit warranty documents signed by an Officer of the Company and notarized stating the terms and conditions of the warrantee identified in Paragraph 1.9.
- K. Any item the CONTRACTOR proposes to utilize as an "or equal" to that proposed in the Contract Documents.

1.6 SUBSTITUTIONS

- A. Any item the CONTRACTOR proposes to substitute for a specified item as an "or equal" must be made in writing for review and approval by the city. Substitutions or deviations not approved by the city shall risk rejection and replacement at no additional cost to the city.
1. For any proposed design change or deviation from these Technical Specifications.
 2. For anything in these Technical Specifications found to conflict with applicable codes and ordinances.
 3. For anything not understood by the CONTRACTOR.
 4. One (1) electronic copy of submittal(s) shall be provided to the city at least two (2) weeks before a determination is required.
 5. Submittals may be drawings, sketches, manufacturer's literature, catalog descriptions, or other descriptions in sufficient detail to allow a decision. The submittal shall indicate the amount to be added to or deducted from the contract price should the submittal be accepted.

1.7 QUALITY ASSURANCE/QUALITY CONTROL

- A. Examination: Each structure component part shall be examined by the manufacturer before delivery and by the CONTRACTOR for dimensional requirements, soundness, and workmanship.

- B. Composition Control: The manufacturer shall control the polymer concrete for each part of the structure fabrication. Documentation of the control checks shall be maintained by the manufacturer.
- C. Testing Records: Records of all materials testing shall be kept, and copies of test results shall be submitted to the city in accordance with Paragraph 1.5.

1.8 INSPECTION POINTS

- A. CONTRACTOR shall keep the city or ENGINEER informed of milestone progress on each of the following items:
 - 1. Start of excavation.
 - 2. Installation of base.
 - 3. Installation of stop log or slide gate system if included.
 - 4. Installation of access hatch and/or manhole rims and covers.
 - 5. Completion of backfill placement.

1.9 WARRANTY

- A. The CONTRACTOR shall replace promptly and at no expense to the city, any materials which fail during the warranty period. Warranty work shall include all ancillary items including, but not limited to permits, traffic control, and flow control required to compete the installation. Warranty and durations are described below:
 - 1. Project Warranty: The CONTRACTOR shall warrant all materials and workmanship performed under this contract for a period of **one (1) year** from the date of final acceptance or as stated in the project Contract Documents whichever is longer.
 - 2. Materials Warranty: The structure shall be free of defects that will affect the long term life and operation. The CONTRACTOR shall warrant in writing all materials provided and work performed under this contract for a period of **five (5) years** from the date of formal acceptance.
 - 3. Structure: Material(s) shall be tested for a design life of a minimum of **fifty (50) years** against deterioration from H2S corrosion and typical chemicals found in municipal sanitary sewage.

PART 2 - PRODUCTS

2.1 POLYMER CONCRETE STRUCTURES

- A. Sewer manholes and junction structures (except Junction Structure B) shall be manufactured using polymer concrete. Approved structure manufacturers:
 - 1. Armorock™ - Manufactured by Geneva Polymer Products: 3555 21st St, 9, Boulder City, NV (702) 479-1772
 - 2. Approved Equal

2.2 MATERIAL and DESIGN

- A. This section is to provide minimum design and testing requirements for a barrel section and lid together.
- B. The design of the structure shall be prepared and professionally sealed by an ENGINEER licensed in the State of Arizona. The polymer concrete structure(s) shall meet or exceed the requirements of buried manholes up to 40 feet deep without the use or reliance upon external or internal structural stiffeners. The design shall not allow infiltration of ground water.
- C. It is the CONTRACTOR's responsibility to verify the measurements of the existing sewer connections prior to ordering the structures. The city and the ENGINEER are not responsible for manufacturing errors due to incorrect measurements. Modifying or cutting the structures in the field to adjust for incorrect measurements is not allowed.
- D. Structure shall be designed and tested to withstand AASHTO HS-20 loading with no structural strength derived from the installation of external or internal structural stiffeners.
- E. The manufacturer shall provide lifting points and instruction on how each component of the structure should be lifted to prevent damage.
- F. Material shall be consistent throughout the structure consisting of resin and aggregate material, including reinforcement that is inert to corrosion from sewer gasses per ASTM D6783 - Standard Specification for Polymer Concrete Pipe (material mix design). Epoxy shall be 100% solids, solvent free, low odor emitting, environmentally friendly, and impervious to chemicals. Manufacturer shall determine the need for and type of reinforcement including that for safety and lifting requirements. Reinforcement, if determined necessary, shall be made of non-ferrous material.
- G. The resins used shall be unsaturated, supplier certified commercial grade polyester resins. Mixing lots of resin from different manufacturers or "odd-lotting" of resins shall not be permitted. Quality-assurance records on the resin shall be maintained. Non-pigmented resin is required to allow for light or "sand" color of interior surface in order to facilitate ease of inspection from ground surface. UV inhibitors shall be added directly to resins to prevent degradation.

2.3 GRATE DESIGN SPECIFICATIONS

- A. The grates shall be fiberglass reinforced polymer (FRP).
 - i. FRP shall be vinyl ester per Specification Section 06611, Fiberglass Reinforced Plastic Gratings and Support Framing Members.

2.4 PORTABLE CRANE

- A. The portable crane shall be per Specification Section 06614-2.4, Fiberglass Reinforced Plastic Stop Logs.

2.5 STOP LOGS AND/OR SLIDE GATE DESIGN SPECIFICATIONS

- A. Stop logs and guide frames shall be per Specification Section 06614, Fiberglass Reinforced Plastic Stop Logs.

2.6 ACCESS HATCH SPECIFICATIONS

- A. The access hatch shall be per Specification Section 05581, Floor Access Hatch Covers.

2.7 MARKING AND IDENTIFICATION

- A. The structure shall be marked on both the interior and exterior with the following information:
1. Manufacturer's Company Name identification.
 2. Date of Manufacture.
 3. Manhole wall thickness.
 4. Installation assist marks.

2.8 DELIVERY, STORAGE AND HANDLING

- A. CONTRACTOR shall be responsible for any damage resulting from improper storage and/or handing. Damaged materials shall be replaced at no additional cost to the city.
- B. Materials shall be handled in accordance with MAG 106.
- C. Store all products according to manufacturer's recommendations until installation. Protect from construction traffic and damage.
- D. During the loading, unloading, and storage, care should be taken to ensure that the structure pieces are not dropped or otherwise damaged.
- E. The structure components should be stored on a smooth surface free of sharp objects.
- F. If the structure is stored horizontally, the structure should be placed in such a way as to avoid damage and be braced.

PART 3 - EXECUTION

3.1 PROTECTION OF SITE

- A. Except as otherwise provided herein, the CONTRACTOR shall protect from damage all fences, structures, sidewalks, utilities, trees, shrubbery, lawns, etc., during the progress of this work. All debris and unused materials shall be removed immediately upon completion of the work. The site shall be restored as nearly as possible to its original condition, including the replacement, at the CONTRACTOR's sole expense, of any facility or landscaping which has been destroyed or damaged beyond restoration.

3.2 UTILITIES

- A. Unless otherwise indicated, the CONTRACTOR shall arrange for and provide any required utilities at his sole cost and expense. This includes but is not limited to water for compaction or testing, power for operating any equipment along with personnel sanitation facilities.

3.3 INSTALLATION

- A. The CONTRACTOR shall install the structure in accordance with approved manufacturer's techniques.
- B. The installation of the structure shall have the following tolerances:
1. Inverts of the flow through portion shall have a tolerance +/- 0.01 feet compared to the elevations indicated on the construction drawings.
 2. The invert of the branch portion shall have a tolerance of -0.05 feet compared to the elevation indicated on the construction drawings.
 3. Inverted slopes shall not be permitted even if meeting the tolerances above.
- C. The manufacturer shall provide an authorized representative to be on-site during the installation process of the structure to assist the CONTRACTOR to ensure proper installation.

3.4 ACCEPTANCE

- A. As a condition precedent to Final Acceptance of the work, CONTRACTOR shall certify the proper installation and testing of material provided under this section. Certification shall warrant that materials and installation are free of defects and/or workmanship and suitable for trouble-free operation under the conditions set forth in these Technical Specifications.

++ END OF SECTION ++

SECTION 13220

PVC LINER

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

This specification covers the supply and installation of a flexible PVC sheet liner with continuous locking extensions in cast-in-place structures to effectively protect the exposed concrete surfaces from corrosion. To accomplish this, the liner must be continuous and free of pinholes both across the joints and in the liner itself.

1.2 QUALITY ASSURANCE

A. Manufacturer's Qualifications: Manufacturer shall have a minimum of five years experience in the production of substantially similar materials and shall be able to show evidence of satisfactory service in at least five similar applications.

B. Installer's Qualifications: Lining shall be performed by a single Installer regularly engaged in installation of polyvinyl chloride liner on concrete surfaces by the liner manufacturer. Installer shall be certified (or licensed) as a qualified Liner Installer by the Liner Manufacturer.

1.3 SUBMITTALS

A. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

- B. Certificates: Submit certificates of compliance with referenced standards.

PART 2 - MATERIAL

2.1 MATERIAL

- A. Plastic liner material shall be made from high polymer vinyl chloride resin, pigments and plasticizers, extruded under high temperature and pressure to form a dense, flexible and impervious sheet. PVC resin shall constitute not less than 99 percent, by weight, of the resin used in the formulation. Copolymer resins are not permitted. The liner surface shall not absorb or retain precipitated or crystalline materials. Liner system shall be impermeable to sewage gases and liquids and shall be non-conductive to bacterial or fungus growth. Liner shall be easily cleaned and maintained in a sanitary condition. Liner shall be resistant to oxidizing agents, sulfuric, phosphoric, nitric, chromic, oleic, and stearic acids; sodium and calcium hydroxides; ammonia, sodium, calcium, magnesium, and ferric sulfate, petroleum oils and greases, vegetable and animal oils, fats, greases and soaps. Liner shall have good impact resistance and shall have an elongation sufficient to bridge up to 1/4 inch settling cracks, which may occur after installation, without damage to the liner.
- B. Liner shall have rows of "T" shaped ribs which protrude from the liner to mechanically bond the liner to the concrete being lined. Once cast into the structure, lining shall be permanently and physically attached to the concrete by the "T" shaped ribs and shall not rely on an adhesive bond. "T" shaped ribs shall have a minimum height of 0.375-inches not including the liner sheet. Rows of "T" shaped ribs shall have an approximate spacing of 2 1/2 inches measured center to center. "T" shaped ribs shall be of the same material as the liner and shall be integrally extruded with the sheet. Unsecured terminal edges of the liner sheet shall not extend beyond the base of the final row of "T" shaped ribs by more than 3/8 inch. Liner sheets shall have a nominal width of 48-inches and shall be supplied in lengths which minimize the number of joints.
- C. Where shown on the Drawings, or where approved by ENGINEER, provide plain sheet liner. Plain sheet liner sheets shall have a nominal width of 44-inches and shall be supplied in lengths which minimize the number of joints.
- D. Inside corners, outside corners, "heels" for interior corners, and "L" shaped terminal strips fabricated of plastic material similar to the plastic liner material shall be provided at all corners, and changes in direction of walls and slabs, and at terminations to provide a neat, gas tight installation. All joint, corner edge and other required strips shall be fabricated of the same material as the liner. Joint strips and exposed welding strips shall have minimum widths of 4-inches and 1 inch, respectively. Joint and exposed welding strips shall both

have beveled edges. All terminal bottom edges of the liner in walls of structures shall be sealed with a termination strip or other means approved by ENGINEER.

- E. Minimum thickness of liner sheet and strips shall be as follows:
1. Sheet with integral "T" shaped ribs: 0.065-inches.
 2. Sheet, plain: 0.094-inches.
 3. Joint strip: 0.075-inches.
 4. Weld strip: 0.094-inches.
- F. Adhesive products and application procedures used in the liner installation shall be approved by the ENGINEER prior to use. Adhesive products shall be non flam-mable.
- G. Cleaners used in the liner installation shall be approved by the ENGINEER prior to use. Cleaners shall be water soluble and non-detrimental to the liner.
- H. Product and Manufacturer: Provide the following:
1. T Lock and PVC Sheet Liner as manufactured by ArmorLok Liner as distributed by ArmorLok PVC Protective Linings, San Bernardino California.
 2. Or Approved Equal

2.2 PHYSICAL PROPERTIES

- A. All plastic liner plate sheets, welding strips and other accessory items, shall have the following physical properties when tested at 77°F±5° (25°C±3°). Chemical resistance listed below.

PROPERTY	TEST METHOD	MINIMUM REQUIREMENT
Tensile Strength	ASTM D412-16	2200 PSI
Elongation	ASTM D412	200%
Hardness	ASTM D2240 Type D	50-60 1 sec. – 35-50 10 sec.
Tearing Strength	ASTM D1004 -13	=/>80 N/mm
Plasticizer Permanence	ASTM D1203-16	<1.0 %
Water Absorption	ASTM D570-98	<0.1 %

Pull Out Strength of Key	SSPWC 210.2.6.4.1	=/> 100lbs per linear inch
Brittle Point	D746	5 degrees F
CHEMICAL PROPERTY	CONCENTRATION	MINIMUM REQUIREMENT
Sulfuric Acid (H ₂ SO ₄)	20%	+/- 0.1%
Sodium Hydroxide (NaOH)	5%	+/- 0.2%
Ammonium Hydroxide (NH ₄ OH)	5%	+/- 1.0%
Nitric Acid (HNO ₃)	1%	+/- 0.1%
Ferric Chloride (FeCl ₃)	1%	+/- 0.2%
Sodium Hypochlorite (NaOCl)	1%	+/- 0.1%
Soap	0.10%	NA
Detergent	0.10%	NA
Bacteriological (BOD)	=/> 700 ppm	NA

- B. Tensile specimens shall be prepared and tested in accordance with ASTM D412 using Die B. Weight change specimens shall be 1-inch (25-mm) by 3-inch (75-mm) samples. Specimens for testing of initial physical properties may be taken from liner sheet and welding strip at any time prior to final acceptance of the work.
- C. Continuous locking extensions embedded in concrete shall withstand a test pull of at least 100 pounds per linear inch (1800 kg/m), applied perpendicularly to the concrete surface

for a period of one minute, without rupture of the locking extensions or withdrawal from embedment. This test shall be made at a temperature of 70°- 80°F (21° - 27 °C).

- D. All plastic liner plate sheets, including locking extensions, all joint, corner and welding strips shall be free of cracks, cleavages or other defects adversely affecting the protective characteristics of the material. The ENGINEER may authorize the repair of such defects by approved methods.
- E. The lining shall be repairable at any time during the life of the structure.

2.3 DETAILS AND DIMENSIONS OF BASIC SIZE SHEETS

- A. Liner sheets shall be a minimum of 0.065 inch (1.65 mm) in thickness. Locking extensions (Tor D shaped) of the same material as that of the liner shall be integrally extruded with the sheet. Locking extensions shall be approximately 2 1/2 inches (67 mm) apart and shall be at least 0.375-inch (9.5 mm) high.
- B. Sheets shall have a nominal width of 4 feet and a length of not more than 8 feet, except that longer lengths may be supplied on special order. Lengths specified shall include a tolerance at a ratio of $\pm 1/2$ inch (12 mm) for each 100 inches (2500 mm).
- C. Special sized, factory pre-welded sheets are available on special order.

2.4 PIPE SIZE SHEETS AND ACCESSORIES

- A. Pipe linings shall be supplied as pipe-size sheets, fabricated by shop-welding the basic-size sheets together. Shop welds shall be made by lapping sheets a minimum of 1/2 inch and applying heat and pressure to the lap to produce a continuous welded joint. Tensile strength measured across shop-welded joints in accordance with ASTM D412 shall be at least 2000 psi (14 MPa).
- B. If required, strap channels shall be 1-inch (25-mm) wide maximum and formed by removing the locking extensions so that a maximum of 3/16 inch (5 mm) remains.
- C. Welding strips shall be approximately 1-inch (25 mm) wide with a minimum width of 7/8 inch (22 mm). Thickness of weld strip shall be a nominal 1/8 inch (3 mm).

PART 3 - INSTALLTION

3.1 INSTALLATION OF LINING

- A. General:
 - 1. Install liner in accordance with the recommendations of the manufacturer and the approved Shop Drawings. Coverage of the liner shall not be less than the minimum shown on the Drawings and described in this Section.
 - 2. Do not install when the ambient temperature is outside the acceptable installation range specified by the manufacturer, but under no condition shall the liner be installed when the ambient temperature is below 60°F.

3. All concrete liner sheets and strips shall be free of cracks, cleavages or other defects adversely affecting the protective characteristics of the material. Remove all defective liner material from the job site.
- B. Positioning Liner:
1. Liner installed in walls shall have the "T" shaped ribs running vertically. In areas requiring termination strips, the bottom elevation of the liner shall be considered to be the lower edge of the termination strip.
- C. Securing Liner:
1. Liner sheets shall be closely fitted and properly secured to the liner side of the forms. Nailing through the liner shall be minimized and nails shall be placed no greater than 1/8-inch from each liner sheet edge. Nail holes shall be patched with a 1-inch minimum weld strip. Adjacent sheets shall be butted with not more than a 1/8 inch opening between the sheets using a Type C 1, C-2 and P-1 joint as specified hereinafter. Where the liner must be bonded to metal, the metal shall be sandblasted to white metal (SSPC SP5) before application of the material.
- D. Concrete Placement:
1. Carefully vibrate concrete poured against lining so as to protect the lining and produce a dense, homogeneous concrete, securely anchoring the locking extensions into the concrete.
- E. Removing Forms:
1. Protect the lining from damage when removing forms. Do not use sharp instruments to pry forms from lined surfaces. When forms are removed, pull any remaining nails without tearing the lining. Clearly mark nail holes, form tie holes, all areas of serious abrasion or damage for repair.
 2. Remove banding strips used in securing liner to forms within the limits of the unlined concrete. Fill voids left in the concrete with cement mortar or other approved material.
- F. Field joints in the liner shall be one or a combination of the following types:
1. Type J 1:
 - a. Use this type of joint at concrete expansion joints, and concrete construction joints where the space between adjacent sheets is greater than 1/8 inch, but does not exceed 2-inches.
 - b. Make joint with a separate 4 inch joint strip and two welding strips. Center the 4 inch strip over the joint, and secure to the liner by a weld along each edge to adjacent liner sheets with a 1 inch weld strip. Space between adjacent sheets shall not exceed 1-inch in width. Lap the 4 inch joint strip over each sheet a minimum of 1-inch.
 2. Type J 2:

- a. Use this type of joint at concrete construction joints, where the space between adjacent sheets is greater than 1/8-inch, but does not exceed 1/2-inch.
- b. Make joint by overlapping sheets by at least 1-inch and securing the overlap to the adjacent liner by means of a welding strip. Upstream sheet shall overlap the downstream sheet. Overlapping sheets shall not extend more than 4-inches beyond the "T" shaped ribs. Secure the lap in place prior to welding using the same means utilized for Type C 1 joint. Apply a welding strip to the back of the joint.

G. Welding:

1. Perform all welding by welders certified by Ameron in strict conformance with the Specifications and instructions of the liner manufacturer. Clean all liner surfaces before welding commences. Provide adequate ventilation in confined spaces during welding operations. Incomplete fusion, charred, or blistered welds will be rejected. Hot air welding guns shall provide clean effluent air at constant pressure to the surfaces to be joined within a temperature range between 500°F and 600°F. Welding shall fuse liner material together to provide a continuous joint equal in corrosion resistance and impermeability to the liner sheets. Supply all equipment required to power the welding guns.

H. Application of Plain Liner Sheets to Metal and Concrete Surfaces:

1. Apply plain liner sheets to metal and concrete surfaces by means of adhesive using the following steps:
 - a. Sandblast all surfaces. After sandblasting, repair all imperfections in concrete surfaces.
 - b. In addition to sandblasting, all concrete surfaces shall be acid etched. Thoroughly wash surface etched with acid with clean water supplied by CONTRACTOR and completely dried before applying primer.
 - c. When concrete surfaces are dry, apply at least one brush coat of an approved primer. When primer is dry, apply at least one coat of approved adhesive to concrete surfaces and the back surface of the liner.
 - d. Position the liner when the surface of the adhesive is ready, with one edge firmly pressed down. Roll the liner into place, care being taken to avoid the formation of air pockets. All joints shall be tight fitting butt joints. Rub the surface of the liner vigorously to secure the liner material firmly in place. Position corner fittings and welding strips over all joints as required and weld in place. Concrete poured against lining shall be vibrated, spaded or compacted in a careful manner so as to protect the lining and produce a dense, homogenous concrete, securely anchoring the locking extensions into the concrete.

3.2 APPLICATION TO CAST-IN-PLACE CONCRETE STRUCTURES

- A. Liner sheets shall be closely fitted and properly secured to the inner forms. Sheets shall be cut to fit curved and warped surfaces using a minimum number of separate pieces.
- B. Unless otherwise shown on the plans, the lining shall be returned at least 2 inches (50 mm) at the surfaces of contact between the concrete structure and items not of concrete (including manhole frames, gate guides, clay pipe or brick manholes and clay or cast iron pipes). The same procedure shall be followed at joints where the type of protective lining is changed or the new work is built to join existing unlined concrete. At each return, the returned liner shall be attached to the item in contact with the plastic-lined concrete using manufactures recommendations. If the liner cannot be sealed against the item because of the joint at the return being too wide or rough or because of safety regulations contact ArmorLok for recommendations.
- C. A Termination Strip of a minimum 2 keys must be installed at the lower limits of the liner unless the liner can be returned into the concrete a minimum 2 inches.

3.3 JOINTS IN LINING FOR CAST-IN-PLACE CONCRETE STRUCTURES

- A. Type J-1: The joint shall be made with a separate 4-inch (100 mm) joint strip and two welding strips. The 4-inch (100 mm) joint strip shall be centered over the joint, heat- sealed to the liner with the goal that all overlapping liner is bonded, an angle grinder with a sanding disk shall be used to lightly profile area where 1" welding strip will be placed to remove sheen and contaminants, then welded along each edge to adjacent sheets with a 1-inch (25 mm) wide welding strip all ends of weld strip shall be melted smooth to avoid snags. The 4-inch (100 mm) joint strip shall lap over each sheet a minimum of 1/2 inch (13 mm). It may be used at any transverse or longitudinal joint.
- B. Type J-2: The joint shall be made by lapping sheets not less than 1/2 inch (13 mm). One 1-inch (25 mm) welding strip is required. The upstream sheet shall overlap the one downstream. The lap shall be heat-sealed with the goal that all overlapping liner is bonded, an angle grinder with a sanding disk shall be used to lightly profile area where 1" welding strip will be placed to remove sheen and contaminants prior to welding on the 1-inch (25 mm) welding strip all ends of weld strip shall be melted smooth to avoid snags.

3.4 TESTING AND REPARING DAMAGED SURFACES

- A. At the point of completion of welding where areas can be isolated and or exposure to damage can be mitigated, all surfaces covered with lining, including welds, shall be tested with an approved electrical holiday detector (Tinker & Rasor Model No. AP-W with power pack) with the instrument set between 18,000 and 22,000 volts.
- B. All welds shall be probed with a 1-inch to 2-inch wide putty knife with no sharp edges. The knife shall probe the welded joint, pushing gently, but firmly between the PVC sheets.

The knife blade will not penetrate a properly welded joint. A joint that is not properly welded will separate fairly easily by the probing action of the knife and allow penetration of the knife blade into the joint.

- C. After the welds have been tested and defective welds repaired, the liner shall be completely cleaned to permit visual inspection and spark testing by CONTRACTOR in the presence of ENGINEER. Spark testing shall be performed with an approved electrical holiday detector furnished by CONTRACTOR with the instrument set at between 18,000 and 22,000 volts minimum. Provide adequate ventilation, lighting, ladders, labor, and scaffolding. Liner areas failing to meet the field test shall be properly repaired and retested.
- D. Take all necessary measures to prevent damage to installed lining from equipment and materials used in or taken through the Work. Responsibility for repairing all damage belongs to the CONTRACTOR.
- E. Patch all nail and tie holes and all cut, torn, and seriously abraded areas in the liner. Fuse welding strip patches to the liner over the entire patch area. The use of parallel, overlapping, or adjoining welding strips will not be permitted. Large patches may consist of plain liner over the damaged area, with an adhesive bond and all edges covered with welding strips fused to the patch and the sound liner adjoining the damaged area.
- F. Whenever liner is not properly anchored to concrete, the repair of liner and restoration of anchorage shall be as directed by the manufacturer and approved by ENGINEER.

3.5 MANUFACTURER'S SERVICES

- A. Furnish the services of a qualified factory trained serviceman assist in the installation and to check the installation before it is placed into service, and instruct OWNER'S personnel in care and maintenance.
- B. Serviceman shall make a minimum of two visits to the site. Each visit shall be for a period of one 4 hour. One visit shall be to assist the installation and the second visit to check the completed installation, and to instruct the OWNER'S Operation and Maintenance personnel in the recommended corrective and preventive maintenance procedures and repair techniques.

++ END OF SECTION ++

SECTION 15050

PIPING SYSTEMS

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. This Section specifies systems of process piping and general requirements for piping systems. Detailed Specifications for the components listed on the Piping System Specification Sheets are found in the applicable Sections of Division 15, Mechanical. This Section shall be used in conjunction with those Sections.
2. Provide all labor, materials, equipment and incidentals as shown on the Drawings, specified and required to furnish, install and test all piping, fittings and specials. The Work includes, but is not limited to, the following:
 - a. All types and sizes of piping, except those specified under other Sections.
 - b. Piping beneath, embedded or within structures.
 - c. Supports, restraints and thrust blocks.
 - d. Pipe encasements.
 - e. Work on or affecting existing piping.
 - f. Testing.
 - g. Cleaning.
 - h. Installation of all jointing and gasket materials, specials and all other Work required to complete the piping installation.
 - i. Unless otherwise specifically shown on the Drawings, specified, or included under other Sections, all buried piping Work required begins at the outside face of structures or structure foundations and extending away from structure.

B. Coordination:

1. Review installation procedures under this and other Sections and coordinate with the Work that is related to this Section.

C. Definitions:

1. Pressure terms used in this Section and elsewhere in Division 15, Mechanical, are defined as follows:
 - a. Maximum: The greatest continuous pressure at which piping system operates.
 - b. Test: The hydrostatic or air pressure used to determine system acceptance.

1.2 QUALITY ASSURANCE

- A. Conform to all applicable requirements of Parts 600 and 700 of the Uniform Standard specifications for Public Work Construction by the Maricopa Association of Governments (MAG) as supplemented by the City of Chandler. If there is a conflict between MAG Standard specifications and these Specifications, the Provisions of these Specifications shall govern.
- B. Requirements of Regulatory Agencies:
 - 1. Comply with requirements of UL, FM and other jurisdictional authorities, where applicable.
 - 2. Refer to the General Conditions regarding permit requirements for this Work.
 - 3. Comply with requirements of the City of Chandler Building Construction Code.
- C. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown on the Drawings or specified.
 - 1. ANSI A13.1, Scheme for the Identification of Piping Systems.
 - 2. ANSI B1.20.1, Pipe Threads, General Purpose (Inch).
 - 3. ANSI B16.1, Cast Iron Pipe Flanges and Flanged Fittings Class 25, 125, 250, and 800.
 - 4. ANSI B16.3, Malleable Iron Threaded Fittings Class 150 and 300.
 - 5. ANSI B16.5, Pipe Flanges and Flanged Fittings.
 - 6. ANSI B16.9, Factory-Made Wrought Steel Buttwelding Fittings.
 - 7. ANSI B16.11, Forged Steel Fittings, Socket Welding and Threaded.
 - 8. ANSI B31.1, Power Piping.
 - 9. ANSI B31.3, Chemical Plant and Petroleum Refinery Piping.
 - 10. ASTM A 47/A 47M, Specification for Ferritic Malleable Iron Castings.
 - 11. ASTM A 53/A 53M, Specification for Pipe, Steel, Black and Hot Dipped, Zinc-Coated Welded and Seamless.
 - 12. ASTM A 74, Specification for Cast Iron Soil Pipe and Fittings.
 - 13. ASTM A 105/A 105M, Specification for Carbon Steel Forgings for Piping Components.
 - 14. ASTM A 106/A 106M, Specification for Seamless Carbon Steel Pipe for High-Temperature Service.
 - 15. ASTM A 126, Specification for Gray Iron Castings for Valves, Flanges, and Pipe Fittings.
 - 16. ASTM A 234/A 234M, Specification for Pipe Fittings of Wrought Carbon Steel and Alloy Steel for Moderate and Elevated Temperatures.
 - 17. ASTM A 312/A 312M, Specification for Seamless and Welded Austenitic Stainless Steel Pipe.
 - 18. ASTM A 403/A 403M, Specification for Wrought Austenitic Stainless Steel Piping Fittings.
 - 19. ASTM A 536, Specification for Ductile Iron Castings.

20. ASTM A 570/A 570M, Specification for Steel, Sheet and Strip, Carbon, Hot-Rolled, Structural Quality.
21. ASTM B 88, Specification for Seamless Copper Water Tube.
22. ASTM C 76, Specification for Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe.
23. ASTM C 296, Specification for Asbestos-Cement Pressure Pipe.
24. ASTM C 443-REV A, Specification for Joints for Circular Concrete Sewer and Culvert Pipe, Using Rubber Gaskets.
25. ASTM C 564, Specification for Rubber Gaskets for Cast Iron Soil Pipe and Fittings.
26. ASTM D 1248, Specification for Polyethylene Plastics Molding and Extrusion Materials.
27. ASTM D 1784, Specification for Rigid Poly (Vinyl Chloride) (PVC) Compounds and Chlorinated Poly(Vinyl Chloride) (CPVC) Compounds.
28. ASTM D 2241, Specification for Poly (Vinyl Chloride) (PVC) Plastic Pipe (SDR-PR).
29. ASTM D 2513, Specification for Thermoplastic Gas Pressure Pipe, Tubing, and Fittings.
30. ASTM D 2665, Specification for Poly (Vinyl Chloride) (PVC) Plastic Drain, Waste, and Vent Pipe and Fittings.
31. ASTM D 2996, Specification for Filament-Wound Reinforced Thermosetting Resin Pipe.
32. ASTM D 3034, Specification for Type PSM Poly (Vinyl Chloride) (PVC) Sewer Pipe and Fittings.
33. ASTM D 3261, Specification for Butt Fusion Polyethylene (PE) Plastic Fittings for Polyethylene (PE) Plastic Pipe and Tubing.
34. ASTM D 3262, Specification for "Fiberglass" (Glass-Fiber-Reinforced Thermosetting-Resin) Sewer Pipe.
35. ASTM D 4174, Practice for Cleaning, Flushing, and Purification of Petroleum Fluid Hydraulic Systems.
36. ASTM D 4101, Specification for Propylene Plastic Injection and Extrusion Materials.
37. ASTM F 441, Specification for Chlorinated Poly (Vinyl Chloride) (CPVC) Plastic Pipe, Schedules 40 and 80.
38. AWWA C105, Polyethylene Encasement for Ductile-Iron Piping for Water and Other Liquids.
39. AWWA C110, Ductile-Iron and Gray-Iron Fittings, 3 Inch Through 48 Inch, for Water and Other Liquids.
40. AWWA C111, Rubber-Gasket Joints for Ductile-Iron and Gray-Iron Pressure Pipe and Fittings.
41. AWWA C115, Flanged Ductile-Iron and Gray-Iron Pipe with Threaded Flanges.
42. AWWA C151, Ductile-Iron Pipe, Centrifugally Cast in Metal Molds or Sand-Lined Molds, for Water or Other Liquids.

43. AWWA C205, Cement-Mortar Protective Lining and Coating for Steel Water Pipe--4 In. and Larger--Shop Applied.
44. AWWA C206, Field Welding of Steel Water Pipe.
45. AWWA C207, Steel Pipe Flanges for Waterworks Services--Sizes 4 In. through 144 In.
46. AWWA C208, Dimensions for Fabricated Steel Water Pipe Fittings.
47. AWWA C209, Cold-Applied Tape Coating for special sections, Connections, and Fittings for Steel Water Pipelines.
48. AWWA C210, Liquid Epoxy Coating Systems for the Interior and Exterior of Steel Water Pipe.
49. AWWA C214, Tape Coating Systems for the Exterior of Steel Water Pipelines.
50. AWWA C301, Pre-stressed Concrete Pressure Pipe, Steel Cylinder Type, for Water and Other Liquids.
51. AWWA C303, Reinforced Concrete Pressure Pipe-Steel Cylinder Type, Pre-tensioned, for Water and Other Liquids.
52. AWWA C600, Installation of Ductile-Iron Water Mains and Their Appurtenances.
53. AWWA C651, Disinfecting Water Mains.
54. AWWA C900, Polyvinyl Chloride (PVC) Pressure Pipe, 4 Inches through 12 Inches, for Water.
55. AWWA M11, Steel Pipe-A Guide for Design and Installation.
56. CISPI 301, specification Data for Hubless Cast Iron Sanitary System with No-Hub Pipe and Fittings.
57. FEDSPEC L-C-530 B(1), Coating, Pipe, Thermoplastic Resin or Thermosetting Epoxy.
58. MIL-H-13528B, Hydrochloric Acid, Inhibited, Rust Removing.
59. MIL-STD-810C, Environmental Test Methods.
60. SAE J1227, Assessing Cleanliness of Hydraulic Fluid Power Components and Systems.
61. International Plumbing Code as Supplimented by the City of Chandler
62. Chandler Construction Code.
63. National Sanitation Foundation (NSF-61) and Arizona Administration Code requirements as stated in Specification Section 01420 – References.

D. Fitting and Coupling Compatibility:

1. To assure uniformity and compatibility of piping components, fittings and couplings for grooved end piping systems shall be furnished by the same manufacturers.

1.3 SUBMITTALS

A. Shop Drawings: Submit for approval the following:

1. Comply with the requirements of Section 01332, Shop Drawing Procedures.

2. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures.
3. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

1.4 PRODUCT DELIVERY, STORAGE AND HANDLING

A. General:

1. Deliver materials to the site to ensure uninterrupted progress of the Work.
2. Handle all pipe, fittings, specials and accessories carefully with approved handling devices. Do not drop or roll material off trucks. Do not otherwise drop, roll or skid piping.
3. Store pipes and fittings on heavy wood blocking or platforms so they are not in contact with the ground.
4. Unload pipe, fittings and specials opposite to or as close to the place where they are to be installed as is practical to avoid unnecessary handling. Keep pipe interiors completely free from dirt and foreign matter.
5. Inspect delivered pipe for cracked, gouged, chipped, dented or other damaged material and immediately remove defective pipe from site.

PART 2 - PRODUCTS

2.1 PIPING MATERIALS

- ##### A. Unless otherwise specified, piping materials, including pipe, gaskets, fittings, connection and joint assemblies, linings and coatings, shall be selected from those listed on the Piping System Specification Sheets. Piping materials shall conform

to detailed Specifications for each type of pipe and piping appurtenances specified in the applicable Sections of Division 15, Mechanical.

2.2 PIPING IDENTIFICATION

A. Marking Piping:

1. Clearly mark each piece of pipe or fitting with a designation conforming to that shown on the approved Shop Drawings.
2. Cast or paint material, type and pressure designation on each piece of pipe or fitting 4-inches in diameter and larger.
3. Pipe and fittings smaller than 4-inches in diameter shall be clearly marked by manufacturer as to material, type and rating.
4. Locator wire shall be installed per the City of Chandler Specifications and Standard Details 2025 addition (Supplement to MAG) detail C-402.

PART 3 - EXECUTION

3.1 INSTALLATION - GENERAL

A. Location:

1. Piping shall be provided as specified, except for adjustments to avoid architectural and structural features and shall be coordinated with electrical construction.

B. Manufacturer's Installation Specialist:

1. Provide the services of a competent installation specialist of the pipe manufacturer when pipe laying begins, if CONTRACTOR is not experienced in laying and jointing a particular type of pipe.
2. Retain installation specialist at the site for a minimum of two days or until competency of the pipe laying crew has been satisfactorily demonstrated.

C. Bedding and Backfill:

1. Bedding and backfill for buried piping shall conform to the requirements of Section 15051, Buried Piping Installation.

3.2 WORK AFFECTING EXISTING PIPING

A. Location of Existing Piping:

1. Locations of existing piping shown on the Drawings should be considered approximate.
2. Determine the true locations of existing piping to which connections are to be made, and locations of other facilities which could be disturbed during earthwork operations, or which may be affected by CONTRACTOR'S Work already installed.

3. Conform to applicable requirements of Division 1, General Requirements, pertaining to cutting and patching and connections to existing facilities.
- B. Taking Existing Pipelines Out of Service:
1. Do not take pipelines out of service, unless specifically listed below, or approved by ENGINEER.
 2. Notify ENGINEER at least 72 hours prior to taking pipeline out of service.
- C. Work on Existing Pipelines:
1. Cut or tap pipes as shown on the Drawings or required, with machines specifically designed for this Work.
 2. Install temporary plugs to prevent entry of mud, dirt, water and debris.
 3. Provide all necessary adapters, fittings, pipe and appurtenances required to complete the Work.
 4. Existing pipelines which are cut and abandoned shall be adequately capped or filled with grout.

3.3 TESTING

- A. General:
1. Upon completion of piping, but prior to application of insulation on exposed piping, test the piping systems. Pressures, media and test durations shall be as specified in Article 3.3, below. Equipment which may be damaged by the specified test conditions shall be isolated. Testing shall be performed using calibrated test gages and calibrated volumetric measuring equipment to determine leakage rates. Each test gage shall be selected so that the specified test pressure falls within the upper half of the gage's range. Unless otherwise specified, notify the ENGINEER 24 hours prior to each test.
 2. Unless otherwise specified, testing, as specified herein, shall include existing piping systems which connect with new piping systems. Existing pipe shall be tested to the nearest existing manhole. Any piping which fails the test shall be repaired. Repair of existing piping will be considered and paid for as extra work.
- B. Liquid Systems:
1. Leakage shall be zero at the specified test pressure throughout the specified duration for the following systems: Exposed piping, buried piping, and buried or exposed piping carrying liquid chemicals. Unless otherwise specified, leakage from other buried liquid piping systems shall be less than 0.02 gallon per hour per inch diameter per 100 feet of buried piping.
 2. For HOBAS pipe testing, refer to Section 15108.1 – Centrifugally Cast Fiberglass Reinforced Polymer Mortar Pipe.

3.4 CLEANING AND FLUSHING

A. General:

1. Piping systems shall be cleaned following completion of testing and prior to connection to operating, control, and regulating or instrumentation equipment. At CONTRACTOR'S option, may clean and test sections of the buried or exposed piping systems. Use of this procedure; however, will not waive the requirement for a full pressure test of the completed system. Unless specified otherwise, piping 24-inches in diameter and smaller shall first be cleaned by pulling a tightly fitting cleaning ball or swab through the system. Piping larger than 24-inches in diameter may be cleaned manually or with a cleaning ball or swab.

B. Liquid Systems:

1. After completion of cleaning, liquid systems, unless otherwise specified, shall be flushed with clean water. With temporary screens in place, the liquid shall be circulated through the piping system using connected equipment for a minimum period of 15 minutes and until no debris is collected on the screens.

3.5 PIPING SPECIFICATION SHEET

A. General:

1. Piping schedule is shown in Table D. Table D indicates the working pressure, fluid category, pipe material, interior lining, exterior coating, joint type, and additional information. The following abbreviations in Table A, B, and C are used in the Table D.

Table A - Material Abbreviations

Polyvinyl Chloride	PVC	Copper	COP
Ductile Iron	DIP	Steel	STL
Vitrified Clay	VCP	Reinforced Concrete	RCP
Acrylonitrile Butadiene Styrene	ABS	Galvanized Steel	GST
Chlorinated Polyvinyl Chloride	CPVC	Fiberglass Reinforced Plastic Resin	FRP
Stainless Steel	SST	Prestressed Concrete Cylinder	PCCP
High-Density Polyethylene	HDPE	Rubber Gasketed Reinforced Concrete	RGRCP
Galvanized Steel	GST	Corrugated Metal Pipe	CMP

Table B - Lining/Coating Abbreviations

Cement Mortar	CM	Polyethylene Wrap	PE
Bituminous Coating	BC	Glass Lined	GL
Galvanized	GALV	Paint per Section 09900	P
Petroleum Tape	PT	Polyurethane	PY
Polyvinyl Chloride	PVC	Epoxy	EP

Table C - Joint Abbreviations

Bell and Spigot	BS	Welded	W
Flanged	FLG	Mechanical Joint	MJ
Push On	PO	Grooved End Fitting	GEF
Solvent Welded	SW	Butt Wrapped	BW
Threaded	THD	Butt Fusion	BF
Soldered	SL	Brazed	BZ

CITY OF CHANDLER: Public Works and Utilities Department
 PROJECT NAME: Loop 101 and 202 Freeway 66-inch Redundant Sewer Project
 PROJECT NUMBER: WW2402.401

Table D – Piping Schedule

BURIED PIPE

SERVICE DESCRIPTION	PIPE SYMBOL	PIPE MATERIAL	SPECIFICATION	WORKING PRESSURE	CLASS/TYPE	INTERIOR LINING	EXTERIOR COATING	JOINT TYPE	TEST PRESSURE
SANITARY SEWER 66-INCHES (Open Cut)	SS	HOBAS	15108.1	Gravity	SN46	NA	NA	FWC Coupling	4 psi *
SANITARY SEWER 66-INCHES (Casing Carrier)	SS	HOBAS	15108.1	Gravity	SN71	NA	NA	Flush Bell Spigot	4 psi *
SANITARY SEWER 48-INCHES	SS	PVC	15106	Gravity	SDR26	NA	NA	Bell and Spigot	4 psi*

*See Specification 15051 Buried Piping Installation for specific Test Pressure requirements.

+ + END OF SECTION + +

SECTION 15051

BURIED PIPING INSTALLATION

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. Provide all labor, materials, equipment and incidentals as shown on the Drawings, specified and required to perform all excavating, backfilling, filling, grading and disposing of earth materials and to furnish, install and test all buried piping, fittings, and specials. The Work includes, but is not limited to, the following:
 - a. Excavation and backfill.
 - b. All temporary means required to prevent discharge of sediment to water courses from dewatering systems or erosion.
 - c. All types and sizes of buried piping, except those specified under other Sections.
 - d. Piping beneath structures.
 - e. Pipe encasements.
 - f. Work on or affecting existing piping.
 - g. Testing.
 - h. Cleaning.
 - i. Installation of all jointing and gasketing materials, specials, flexible couplings, mechanical couplings, harnessed and flanged adapters, sleeves, tie rods, and all other Work required to complete the buried piping installation.
 - j. Unless otherwise specifically shown on the Drawings, specified, or included under other Sections, all buried piping Work required, beginning at the outside face of structures or structure foundations and extending away from structure.
2. No classification of excavated materials will be made. Excavation includes all materials regardless of type, character, composition, moisture, or condition thereof.

B. Coordination:

1. Review installation procedures under this and other Sections and coordinate the installation of items that must be installed with, or before, the buried piping installation Work.
2. Section 15051, Buried Piping Installation, specifies the installation of all buried piping materials specified in Sections of Division 15, Mechanical. Coordinate with all applicable Sections.

1.2 QUALITY ASSURANCE

- A. Conform to all requirements of Section 601 of the Uniform Standard Specifications for Public Work Construction by the Maricopa Association of Governments (MAG), as supplemented by the City of Chandler. If there is a conflict between MAG Standard Specifications and these Specifications, the Provisions of these Specifications shall govern.
- B. Testing Services:
 - 1. General: Testing of materials, testing for moisture content during placement and compaction of fill materials, and of compaction requirements for compliance with technical requirements of the Specifications shall be performed by a testing laboratory as designated in Section 01451, Testing Laboratory Services Furnished by OWNER and 01452, Testing Laboratory Services Furnished by CONTRACTOR.
 - 2. ENGINEER'S Testing Agency Scope:
 - a. Test CONTRACTOR'S proposed materials in the laboratory and/or field for compliance with these Specifications.
 - b. Perform field moisture content and density tests to assure that the specified compaction of backfill materials has been obtained.
 - c. Report all test results to the OWNER and CONTRACTOR.
 - 3. Authority and Duties of CONTRACTOR'S Testing Agency: Technicians representing the testing laboratory shall inspect the materials in the field and perform tests and shall report their findings to the ENGINEER and CONTRACTOR. When the materials furnished or Work performed fails to fulfill Specification requirements, the technician will direct the attention of the ENGINEER and CONTRACTOR to such failure.
 - a. The technician shall not act as foreman or perform other duties for CONTRACTOR. Work will be checked as it progresses, but failure to detect any defective Work or materials shall not in any way prevent later rejection when such defect is discovered, nor shall it obligate the ENGINEER for final acceptance. Technicians are not authorized to revoke, alter, relax, enlarge, or release any requirements of the Contract Documents, nor to approve or accept any portion of the Work.
 - 4. Responsibilities and Duties of CONTRACTOR:
 - a. The use of testing services shall in no way relieve CONTRACTOR of the responsibility to furnish materials and construction in full compliance with the Contract Documents.
 - b. To facilitate testing services:
 - 1) Secure and deliver to the testing agency, without cost, preliminary representative samples of the materials he proposes to use and which are required to be tested.
 - 2) Furnish such casual labor as is necessary to obtain and handle samples at the Work site or at other sources of material.

- 3) Advise the ENGINEER'S testing agency sufficiently in advance of operations to allow for completion of quality tests and for the assignment of personnel.
- c. CONTRACTOR'S Testing Service shall inspect and approve subgrades and fill layers before further construction Work is performed thereon.
- d. Responsibility belongs to CONTRACTOR to accomplish the specified compaction for backfill and to control the operations by confirmation tests to verify and confirm compliance, and is complying at all times, with the requirements of these Specifications concerning compaction, control, and testing.
- e. The frequency of CONTRACTOR'S confirmation tests shall be not less than as follows; each test location for trenches shall include tests for each layer, type, or class of backfill from bedding to finish grade.
 - 1) Trenches for buried pipe:
 - a) In Open Fields: Four locations every 200 linear feet, each lift.
 - b) Along Dirt or Gravel Roads or Off Traveled Right-of-way: Four locations every 200 linear feet, each lift.
 - c) Crossing Paved Roads: Two locations along each crossing.
 - d) Under Pavement Cuts or Within Two Feet of Pavement Edges: Two locations every 200 linear feet, each lift.
 - e) Compaction test shall be taken at least every 1 foot of total lift and 50 feet lineally.
- f. Copies of the test reports shall be submitted promptly to the ENGINEER. CONTRACTOR'S tests shall be performed by a soils testing laboratory acceptable to the ENGINEER.
- g. Demonstrate the adequacy of compaction equipment and procedures before exceeding any of the following amounts of earthwork quantities:
 - 1) 100 linear feet of trench backfill.
- h. Until the specified degree of compaction on the previously specified amounts of earthwork is achieved, no additional earthwork of the same kind shall be performed.
- i. Periodic compliance tests will be made by the ENGINEER to verify that compaction is conforming to the requirements previously specified, at no cost to CONTRACTOR. Remove the overburden above the level at which the ENGINEER wishes to test and shall backfill and recompact the excavation after the test is complete.
- j. If compaction fails to conform to the specified requirements, remove and replace the backfill at proper density or shall bring the density up to specified level by other means acceptable to the ENGINEER. Subsequent tests required to confirm and verify that the reconstructed backfill has been brought up to specified density shall be paid by CONTRACTOR. CONTRACTOR'S confirmation tests shall be performed in a manner acceptable to the ENGINEER. Frequency of confirmation tests for

remedial Work shall be double that amount specified for initial confirmation tests.

C. Requirements of Regulatory Agencies:

1. Comply with requirements of NFPA Standard No. 24 for “Outside Protection” where applicable to water pipe systems used for fire protection.
2. Comply with requirements of UL, FM and other jurisdictional authorities, where applicable.
3. Refer to the General and Supplementary Conditions regarding permit requirements for this Project.
4. Comply with requirements of the Chandler Building Code and ADOT.
5. Obtain all necessary permits for Work in roads, rights-of-way, ADOT, etc. Also, obtain permits as required by local, state and federal agencies for discharging water from excavations.
6. Perform excavation Work in compliance with applicable requirements of governing authorities having jurisdiction.

D. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown on the Drawings or specified.

1. Excavation and Backfill:
 - a. ASTM D 422, Standard Test Method for Particle-Size Analysis of Soils.
 - b. ASTM D 427, Test Method for Shrinkage Factors of Soils by the Mercury Method.
 - c. ASTM D 698, Standard Test Method for Laboratory Compaction Characteristics of Soil.
 - d. ASTM D 1556, Standard Test Method for Density and Unit Weight of Soil in Place by the Sand-Cone Method.
 - e. ASTM D 2166, Standard Test Method for Unconfined Compression Strength of Cohesive Soil.
 - f. ASTM D 2922, Standard Test Methods for Density of Soil and Soil-Aggregate in Place by Nuclear Methods (Shallow Depth).
 - g. ASTM D 3017, Standard Test Method for Water Content of Soil and Rock in Place by Nuclear Methods (Shallow Depth).
 - h. ASTM D 4318, Method of Test for Liquid Limit of Soils.
 - i. AISC Specifications for the Design, Fabrication, and Erection of Structural Steel for Buildings.
 - j. OSHA Standard, Title 29, Code of Federal Regulations, Part 1926, Section .650 (Subpart P - Excavations).
 - k. AASHTO T-99, The Moisture-Density Relations of Soils Using a (2.5 kg) 5.5 lb Rammer and a (305 mm) 12 in. Drop {Proctor}.
 - l. ASSHTO-T-191, Density of Soil in Place by the Sand Cone Method.
 - m. Uniform Standard Specifications for Public Work Construction by the Maricopa Association of Governments (MAG), as supplemented by the City of Chandler, Section 601.

2. Piping Materials and Installation:
 - a. ASTM D 2321, Practice for Underground Installation of Flexible Thermoplastic Pipe.
 - b. ASTM D 2774, Practice for Underground Installation of Thermoplastic Pressure Piping.
 - c. AWWA C105, Polyethylene Encasement for Ductile-Iron Piping for Water and Other Liquids.
 - d. AWWA C111, Rubber-Gasket Joints for Ductile-Iron Pressure Pipe and Fittings.
 - e. AWWA C206, Field Welding of Steel Water Pipe.
 - f. AWWA C600, Installation of Ductile-Iron Water Mains and Their Appurtenances.
 - g. AWWA C606, Grooved and Shouldered Joints.
 - h. AWWA C651, Disinfecting Water Mains.
 - i. AWWA M9, Concrete Pressure Pipe.
 - j. AWWA M11, Steel Pipe - A Guide for Design and Installation.
 - k. AWWA M23, PVC - Design and Installation.
 - l. ASCE MOP No. 37, Design and Construction of Sanitary and Storm Sewers.
 - m. Concrete Pipe Handbook, American Concrete Pipe Association.
 - n. NFPA 24, Private Fire Service Mains and Their Appurtenances.
 - o. NFPA 54, National Fuel Gas Code.
 - p. NSF/ANSI 61, Drinking Water System Components Health Effects.
 - q. Arizona Administrative Code R18-4-213, Standards for Additives, Materials, and Equipment.

1.3 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
 1. Excavation and Backfill Submittals:
 - a. Excavation Plan: Prior to start of excavation operations, a written plan shall be submitted to demonstrate compliance with OSHA Standard 29 CFR Part 1926.650. As a minimum, excavation plan shall include:
 - 1) Name of competent person.
 - 2) Excavation method(s) or protective system(s) to be used.
 - 3) Copies of "manufacturer's data" or other tabulated data if protective system(s) are designed on the basis of such data.
 - b. Excavation and backfill requirements detailing sheeting and bracing, or other protective system(s), dewatering systems, cofferdams, and underpinning.
 - c. Proposed Backfill Material and Flowable Backfill:
 - 1) Submit material testing results of materials proposed for backfill.
 - 2) Submit lean concrete flowable backfill mix design for approval, demonstrating compliance with these Specifications.

- d. Shop Drawings shall be prepared by a Registered Professional Engineer, licensed in the State of Arizona, recognized as an expert in the specialty involved. Drawings shall be submitted to ENGINEER for record purposes only. Calculations shall not be submitted. Drawing submittals will not be checked and will not imply approval by ENGINEER of the Work involved. Responsibility belongs to CONTRACTOR for designing, installing, operating and maintaining whatever system is necessary to satisfactorily accomplish all necessary sheeting, bracing, protection, underpinning and dewatering.
 - e. Samples of all materials, including flowable backfill, select backfill, general backfill, granular embedment, crushed stone and sand shall be submitted to the CONTRACTOR'S Testing Agency. Samples of the proposed material shall be submitted at least 14 days in advance of its anticipated use. Samples shall consist of 0.5 cubic feet of each type of material.
- 2. Piping Materials and Installation Submittals:
 - a. Laying schedules for all piping
 - b. Full details of piping, specials, manholes, joints, harnessing and thrust blocks, and connections to existing piping, structures, equipment and appurtenances.
 - c. Certificates of compliance with referenced Standards for proposed pipe material.
 - d. Descriptions of proposed pipe testing methods, procedures and apparatus. Prepare and submit a report for each test conducted.
- 3. Field Test Reports:

Testing laboratory shall submit copies of test reports for Field Density of Backfill directly to ENGINEER, with copy to CONTRACTOR.
- 4. Record Drawings:
 - a. During progress of the Work, keep an up-to-date set of Record Drawings showing field and Shop Drawing modifications.
 - b. Submit Record Drawings prior to the time of Substantial Completion.
 - c. Field As-Builts redlines shall be submitted to the ENGINEER along with each pay application.
- 5. Comply with the requirements of Section 01332, Shop Drawing Procedures.
- 6. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures.
- 7. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the

right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

1.4 JOB CONDITIONS

- A. Existing Structures: The Drawings show certain surface and underground structures adjacent to the Work. This information has been obtained from existing records. It is not guaranteed to be correct or complete and is shown on the Drawings for the convenience of CONTRACTOR. Explore ahead of the required excavation to determine the exact location of all existing structures. Structures shall be supported and protected from damage by CONTRACTOR. If they are broken or damaged, restore them immediately at no additional cost to the OWNER.
- B. Existing Utilities: Locate existing underground utilities in the areas of the Work. CONTRACTOR shall pothole for existing utilities that are shown per the Contract Drawings that are in the vicinity of the trench excavation limits or for utilities that cross the proposed water line alignment. CONTRACTOR shall provide to the ENGINEER pothole results prior to excavation activities that identifies the type of utility, invert elevation, top of utility, northing and easting. CONTRACTOR shall pothole all utilities that may not be shown per the Contract Drawings that are identified by Bluestake. Potholing activities shall be performed prior to excavation to determine final water line alignment or any modifications that may be required. If utilities are to remain in place, provide adequate means of protection during all operations.
 - 1. Should uncharted or incorrectly charted piping or other utilities be encountered during excavation, consult piping or utility owner and ENGINEER immediately for directions as to procedure. Cooperate with OWNER and utility owner in keeping services and facilities in operation. Repair damaged utilities to satisfaction of utility owner.
 - 2. In general, service lines to individual houses and businesses are not shown on the Drawings, however, assume that a service exists for each utility to each house or business.
 - 3. Do not interrupt existing utilities serving facilities occupied and used by OWNER or others, except when permitted in writing by ENGINEER and then only after acceptable temporary utility services have been provided.

4. Demolish and completely remove from site existing underground utilities indicated to be removed. Coordinate with utility companies for shut-off of services if lines are active.
- C. Protection of Persons and Property: Barricade open excavations occurring as part of the Work and post with warning lights. Operate warning lights during hours from dusk to dawn each day and as otherwise required.
 1. Protect structures, utilities, sidewalks, pavements, and other facilities from damage caused by settlement, lateral movement, undermining, washout and other hazards created by earthwork operations.
- D. Dust Control: Conduct all operations meeting the requirements specified in Section 01414, Earthmoving and Dust Control.
- E. Roadways and Walks: Unless otherwise approved by ENGINEER, excavated material and materials of construction shall be so deposited, and the Work shall be so conducted, as to leave open and free for pedestrian traffic all crosswalks, and for vehicular traffic a roadway not less than ten feet in width. All hydrants, valves, fire alarm boxes, letter boxes, and other facilities which may require access during construction shall be kept accessible for use. During the progress of the Work, maintain such crosswalks, sidewalks, and roadways in satisfactory condition and the Work shall at all times be so conducted as to cause a minimum of inconvenience to public travel, and to permit safe and convenient access to private and public property along the line of the Work.

1.5 PRODUCT DELIVERY, STORAGE AND HANDLING

- A. Deliver materials to the site to ensure uninterrupted progress of the Work.
- B. Handle all pipe, fittings, specials and accessories carefully with approved handling devices. Do not drop or roll material off trucks. Do not otherwise drop, roll or skid piping.
- C. Store pipes and fittings on heavy wood blocking or platforms so they are not in contact with the ground.
- D. Unload pipe, fittings and specials opposite to or as close to the place where they are to be installed as is practical to avoid unnecessary handling. Keep pipe interiors completely free from dirt and foreign matter.
- E. Inspect delivered pipe for cracked, gouged, chipped, dented or otherwise damaged material and immediately remove from site.

PART 2 - PRODUCTS

2.1 EMBEDMENT AND BACKFILL MATERIALS

A. Pipe Bedding/Flowable Backfill:

1. Provide a flowable backfill in lieu of aggregate base course (ABC) as shown per City of Chandler Standard Detail C-402, pipe bedding that shall be 6-inches minimum clearance below outer diameter of pipe to 12-inches minimum above outer diameter of pipe crown. Flowable backfill shall conform to a ½ Sack Controlled Low Strength Material (CLSM), as specified in Section 728 of the MAG Uniform Standard Specifications.

B. Backfill Materials:

1. Materials acceptable for use as backfill above pipe bedding shall be stockpiled native sandy clay or granular soils obtained from on-site excavations and which are uniformly mixed, contain no organic matter, nor contain rocks or fragments greater than 4-inches in size, nor have greater than 50-percent passing the 200 sieve. The maximum expansion of on-site materials shall be 1-percent as performed on a sample remolded to approximately 100-percent of the maximum dry density as determined in accordance with ASTM D 698 at two percent below optimum moisture content under a 100-psf surcharge pressure.
2. All materials for use as backfill and fill material shall be tested by the laboratory and approved by the ENGINEER.

2.2

2.2 PIPING MATERIALS

- A. Unless otherwise specified, piping materials, including pipe, gaskets, fittings, connection and joint assemblies, linings and coatings, shall be selected from those listed in Section 15050, Piping Systems. Piping materials shall conform to detailed Specifications for each type of pipe and piping appurtenances specified in the applicable Sections of Division 15, Mechanical.

PART 3 - EXECUTION

3.1 EXCAVATION

- A. Provide ENGINEER with sufficient notice and with means to examine the areas and conditions under which excavation is to be performed. ENGINEER will notify CONTRACTOR if conditions are found that may be detrimental to the proper and timely completion of the Work. Do not proceed with the Work until unsatisfactory conditions have been corrected in an acceptable manner.

- B. Perform all excavation required to complete the Work as shown on the Drawings, specified and required. Excavations shall include earth, sand, clay, gravel, hardpan, boulders not requiring drilling and blasting for removal, decomposed rock, pavements, rubbish and all other materials within the excavation limits.
- C. Excavations for pipelines shall be open excavations. Provide excavation protection system(s) required by ordinances, codes, law and regulations to prevent injury to workmen and to prevent damage to new and existing structures or pipelines. Unless shown on the Drawings or specified otherwise, protection system(s) shall be utilized under the following conditions.
 - 1. Excavation Less Than Five Feet Deep: Excavations in stable rock or in soil conditions where there is no potential for a cave-in may be made with vertical sides. Under all other conditions, excavations shall be sloped and benched, shielded, or shored and braced.
 - 2. Excavations More Than Five Feet Deep: Excavations in stable rock where there is no potential for a cave-in may be made with vertical sides. Under all other conditions, excavations shall be sloped and benched, shielded or shored and braced.
 - 3. Excavation protection system(s) shall be installed and maintained in accordance with drawings submitted under Article 1.3, above.
- D. Where the pipeline is to be placed below the ground water table, well points, cofferdams or other acceptable methods shall be used to permit construction of said pipeline under dry conditions. Dry conditions shall prevail until the pipelines are properly jointed, tested and backfilled. Water level shall be maintained below top of backfill at all times.
- E. Pumping of water from excavations shall be done in such a manner to prevent the carrying away of unsolidified concrete materials, and to prevent damage to the existing subgrade.
- F. Material Storage: Stockpile satisfactory excavated materials in approved areas, until required for backfill or fill. Place, grade and shape stockpiles for proper drainage.
 - 1. Locate and retain soil materials away from edge of excavations.
 - 2. Dispose of excess soil material and waste materials as specified hereinafter.
 - 3. Stockpiled excavated soils for use as subsequent fill shall be classified by laboratory as on-site granular or sandy clay soils. Use and placement of fill shall be performed as specified for each class.
 - 4. Excess soil from excavations shall be disposed of off-site. Disposal shall be in accordance with state and local regulatory requirements.
- G.

- G. Trench width shall be minimized to greatest extent practical but shall conform to MAG Standard Specification 601 and be sufficient to provide room for installing, joining, and inspecting piping.
 - 1. The width of the trench shall not be greater than the maximum indicated per MAG Specification Section 601 at and below the level of the top of the pipe. If the maximum width as specified above is exceeded at the top of the pipe, provide, at no additional cost to the OWNER, the necessary loading bearing capacity by means of bedding, having a higher bedding factor than that specified, higher strength pipe a concrete cradle, cap or encasement, or by other means approved in writing by the ENGINEER.
 - 2. The width of the trench above that level may be made as wide as necessary for shoring or other wall support measures necessary for a safe and proper installation. Trench walls may be sloped in-lieu of shoring, sheeting or other wall support measures. In all cases, responsibility belongs to CONTRACTOR for all costs incurred as a result of increased trench width.
 - 3. Enlargements at pipe joints may be made if required and approved by ENGINEER.
 - 4. Sufficient for shoring and bracing, or shielding and dewatering.
- H. Subgrades for trench bottoms shall be firm, dense, and thoroughly compacted and consolidated; shall be free from mud, muck, and other soft or unsuitable materials; and shall remain firm and intact under all construction operations. Where ENGINEER considers the existing subgrades unsuitable, remove same and replace it with Controlled Low Strength Material. Subgrades which are otherwise solid, but which become soft or mucky on top due to construction operations, shall be replaced with Controlled Low Strength Material. The finished elevation of stabilized subgrades shall not be above subgrade elevations required for the piping installation as herein specified
- I. Except at locations where excavation of rock from the bottom of the trench is required, care shall be taken not to excavate below the required depth. Unauthorized excavation below the specified grade line shall be refilled at CONTRACTOR'S expense with Controlled Low Strength Material.
- J. Whenever rock is encountered in the trench bottom, it shall be over excavated to a minimum depth of 6-inches below the O.D. of the pipe. This over excavation shall be filled with Controlled Low Strength Material.

3.2 EXCAVATION DRAINAGE AND DEWATERING

- A. General:
 - 1. Prevent surface and subsurface water from flowing into excavations and from flooding adjacent areas.
 - 2. Remove water from excavations as fast as it collects.

3. Maintain the ground water level at approximately three feet below the bottom of the excavation to provide a stable surface for construction operations, a stable subgrade for permanent work and to prevent damage to Work during all stages of construction.
 4. Provide and maintain pumps, sumps, suction and discharge lines and other dewatering system components necessary to convey water away from excavations. Adequate operational standby equipment shall be maintained on the site.
 5. Provide approved sediment traps when water is conveyed into water courses.
 6. Obtain ENGINEER'S approval before shutting down dewatering system for any reason.
- B. Standby Requirements for Dewatering:
1. Provide standby equipment to ensure continuity of dewatering operations.
- C. Disposal of Water Removed by Dewatering System:
1. Dispose of all water removed from the excavation in such a manner as not to endanger public health, property, or any portion of the Work under construction or completed.
 2. Dispose of water in such a manner as to cause no inconvenience to OWNER, ENGINEER or others involved in work about the site.
 3. Convey water from the excavation in a closed conduit. Do not use trench excavations as temporary drainage ditches.

3.3 SHEETING, SHORING, AND BRACING FOR STRUCTURE EXCAVATIONS

- A. General:
1. Used material shall be in good condition, not damaged or excessively pitted. All steel or wood sheeting designated to remain in place shall be new. New or used sheeting may be used for temporary work.
 2. All timber used for breast boards (lagging) shall be new or used, meeting the requirements for Douglas Fir Dense Construction grade with a bending strength not less than 1,500 psi or Southern Pine No. 2 Dense.
 3. All steel work for sheeting, shoring, bracing, cofferdams, etc., shall be designed in accordance with the provisions of the "Specifications for the Design, Fabrication and Erection of Structural Steel for Buildings" of the AISC, except that field welding will be permitted.
 4. Steel sheet piling shall be manufactured from steel conforming to ASTM A 328. Steel for soldier piles, wales, and braces shall be new or used and shall conform to ASTM A 36/ A 36M.
 5. Maintain shoring and bracing in excavations regardless of time period excavations will be open. Carry down shoring and bracing as excavation progresses.

6. Unless otherwise shown on the Drawings, specified, or ordered, all materials used for temporary construction shall be removed when Work is completed. Such removal shall be made in a manner not injurious to the structure or its appearance or to adjacent Work.
 7. Provide permanent steel sheet piling or pressure creosoted timber sheet piling wherever subsequent removal of sheet piling might permit lateral movement of soil under adjacent structures. Cut off tops as required, but at least 3 feet below grade, and leave permanently in place.
 8. The clearances and types of the temporary structures, insofar as they affect the character of the finished Work and the design of sheeting to be left in place, will be subject to the approval of ENGINEER; but, responsibility for the adequacy of all sheeting, shoring, bracing, coffer-damming, etc., belongs to CONTRACTOR.
 9. Safe and satisfactory sheeting, shoring, and bracing shall be the entire responsibility of CONTRACTOR.
 10. All Municipal, County, State, and Federal ordinances, codes, regulations, and laws shall be observed.
- B. Removal of Sheeting and Bracing:
1. Remove sheeting and bracing from excavations, unless otherwise directed in writing by ENGINEER. Removal shall be done so as to not cause injury to the Work. Removal shall be equal on both sides of excavation to ensure no unequal loads on pipe or structure.
 2. Defer removal of sheeting and bracing, where removal may cause soil to come into contact with concrete, until the following conditions are satisfied:
 - a. Concrete has cured a minimum of seven days.
 - b. Wall and floor framing up to and including grade level floors are in place.

3.4 TRENCH SHIELDS

- A. Excavation of earth material below the bottom of a shield shall not exceed the limits established by ordinances, codes, laws, and regulations.
- B. When Using a Shield for Pipe Installation:
1. Any portion of the shield that extends below the mid-diameter of an installed rigid pipe (e.g., RCP) shall be raised above this point prior to moving the shield ahead for the installation of the next length of pipe.
 2. The bottom of the shield shall not extend below the mid-diameter of installed flexible pipe (e.g., steel, DI, PVC, etc.) at any time.
- C. When using a shield for the installation of structures, the bottom of the shield shall not extend below the top of the bedding for the structures.

- D. When a shield is removed, extreme care shall be taken to prevent damage to the structures or the disturbance of the bedding for structures. Structures that are disturbed shall be removed and reinstalled as specified.

3.5 GENERAL REQUIREMENTS FOR PIPE BEDDING, BACKFILL AND COMPACTION

- A. Furnish, place, and compact all flowable backfill, backfill, and other materials required for pipelines as required to provide the finish grades as shown on the Drawings and as described herein.
- B. Fill containing organic materials or other unacceptable material shall be removed and replaced with approved fill material as specified.
- C. Placement Flowable Pipe Bedding and Backfill Material:
1. Bedding and backfill material as specified per section 2.1 shall be placed to the grades shown per City of Chandler Detail C-402. Bring pipe bedding around piping up evenly on all sides.
 2. Flowable backfill shall be placed in accordance with MAG Standard Specification 604. Mechanical vibration in accordance with ACI requirements shall be used to consolidate all flowable backfill material.
 3. Keep excavations dry during backfilling operations. No backfill or fill material shall be placed when free water is standing on the surface of the area where the fill is to be placed. No compaction of fill will be permitted with free water on any portion of the fill to be compacted. No fill shall be placed or compacted in a frozen condition or on top of frozen material. Any fill containing organic materials or other unacceptable material previously described shall be removed and replaced with approved fill material prior to compaction.
 4. Compaction shall be performed with equipment suitable for the type of backfill material being placed. CONTRACTOR shall select equipment that is capable of providing the minimum density required by these Specifications. Equipment shall be provided that is capable of compacting in restricted areas next to structures and around piping. The effectiveness of the equipment selected by CONTRACTOR shall be tested at the commencement of compacted fill work by construction of a small section of fill within the area where fill is to be placed. If tests on this section of fill show that the specified compaction is not obtained, CONTRACTOR shall increase the amount of coverages, decrease the lift thicknesses, or obtain a different type of compactor.
 5. The compaction requirements specified are predicated on the use of normal materials and compaction equipment. In order to establish criteria for the placement of a controlled fill so that it will have compressibility and strength characteristics compatible with the proposed structural loadings, a series of laboratory compaction and/or compressive strength tests shall be performed on the samples of materials submitted by CONTRACTOR. From the results of the

laboratory tests, the final values of the required percent compaction, the acceptable compaction moisture content range, and the maximum permissible lift thickness will be established for the fill material and construction equipment proposed.

6. If the specified densities are not obtained because of improper control of placement or compaction procedures, or because of inadequate or improperly functioning compaction equipment, CONTRACTOR shall perform whatever Work is required to provide the required densities. This Work shall include complete removal of unacceptable fill areas, and replacement and recompaction until acceptable fill is provided, at no additional cost to the OWNER.
7. CONTRACTOR shall repair, at their own expense, any after-settlement that occurs. CONTRACTOR shall make all repairs and replacements required within 30 days after notice from ENGINEER or OWNER.

D. Compaction Density Requirements: Refer to Article 3.9, below.

3.6 PIPE INSTALLATION

A. General:

1. All bedding shall be inspected by ENGINEER prior to laying pipe. Notify ENGINEER in advance of excavating, bedding and pipe laying operations.
2. The ENGINEER prior to installation shall inspect all piping. ENGINEER'S inspection will not relieve CONTRACTOR or manufacturer from responsibility for damaged products.
3. All piping shall be carefully examined for cracks, damage or other defects before installation. Any piping that is defective, including but not limited to, cracked, damaged, in poor condition, or with damaged linings or improper markings shall be rejected, unless the product can be repaired in a manner acceptable to the manufacturer and ENGINEER. Any piping found to be broken or defective after it has been installed shall be removed, replaced or repaired at CONTRACTOR'S expense.
4. Take field measurements, where required, prior to installation to ensure proper fitting of the Work. Uncover existing pipelines sufficiently in advance of the proposed Work in order that the type and location of the existing pipes and joints and other information required to fabricate the proposed piping can be determined. Responsibility to obtain whatever information is required to complete the connections of the proposed pipelines to the existing pipelines belongs to CONTRACTOR.
5. Present all conflicts between piping systems and equipment, structures or facilities to ENGINEER for determination of corrective measures before proceeding.
6. Request instructions from ENGINEER before proceeding if there is a conflict between the manufacturer's recommendations and the Contract Documents.

7. Installation of all pipe, fittings, valves, specials and appurtenances shall be subject to the review and approval of the ENGINEER.
8. Install piping as shown on the Drawings, specified and as recommended by the manufacturer and in conformance with referenced standards and approved Shop Drawings.
9. No pipe shall be brought into position until the preceding length has been bedded and secured in its final position.
10. Minimum earth cover over the piping shall be as shown on the Drawings, specified or directed by the ENGINEER, but in no case shall the earth cover be less than 3 feet-6 inches for all piping, except drains.
11. Interior of all piping and mating surfaces shall be inspected and all dirt, gravel, sand, debris or other foreign material shall be completely removed from the interior and mating surfaces before installation. Measures shall be taken to maintain the interior of all piping clean until acceptance of the completed Work. Care shall be taken to prevent foreign matter from entering joint space. Bell and spigot mating surfaces shall be wiped clean immediately before piping is laid. For ductile-iron pipe, the bell and spigot mating surfaces shall be thoroughly cleaned with a wire brush.
12. Install piping accurately to line and grade shown on the Drawings, specified or directed, unless otherwise approved by the ENGINEER. Accurate means of determining and checking the alignment and grade shall be used, which shall be subject to the approval of the ENGINEER. Any modifications to the Contract Documents to suit the pipe manufacturer's standard shall be approved by the ENGINEER. Remove and relay piping that is incorrectly installed, at CONTRACTOR'S expense.
13. Do not lay piping in water, unless otherwise specified in these Specifications or approved by the ENGINEER. Ensure that the water level in the trench is at least 6-inches below the bottom of piping. Maintain a dry trench until jointing and backfilling are complete, unless otherwise specified in these Specifications or approved by the ENGINEER.
14. Where unforeseen conditions will not permit the installation of piping as shown on the Drawings or specified, no piping shall be installed without approval of the ENGINEER. Do not modify structures or facilities without approval of the ENGINEER.
15. Start laying piping at lowest point and proceed toward the higher elevations, unless otherwise approved by the ENGINEER. Slope piping uniformly between elevations shown on the Drawings or as otherwise directed by the ENGINEER.
16. Place bell and spigot piping so that the bells face the direction of laying, unless otherwise approved by the ENGINEER.
17. Piping shall be installed so that the barrel of the piping, and not the joints, receives the bearing pressure from the trench bottom or other bedding condition.
18. No piping shall be brought into position until the preceding length, valve, fitting, or special has been bedded and secured in place.

19. Whenever pipe laying is not actively in progress, the open ends of the piping shall be closed by a temporary plug or cap to prevent soil, water and other foreign matter from entering the piping.
20. Field cutting of metallic piping, where required for inserting valves, fitting, specials, and closures, shall be made with a machine specially designed for cutting piping and in accordance with the manufacturer's instructions. Cuts shall be carefully done, without damage to piping, so as to leave a smooth end at right angles to the axis of the piping. Cut end shall be tapered and sharp edges filed off smooth. Flame cutting shall not be permitted. Any piping damaged by CONTRACTOR due to improper or careless methods of cutting shall be replaced or repaired at his expense.
21. Blocking under piping shall not be permitted, unless specifically approved by ENGINEER for special conditions.
22. Protective linings and coatings shall be touched up prior to installation, where required.
23. Except where bends, wyes or similar fittings are used, changes in alignment and grade of the piping shall be made by deflecting joints or with beveled pipe. Permissible joint deflection shall not exceed 75 percent of the amount allowed by the manufacturer.
24. All joints shall be made in the presence of the ENGINEER or his duly authorized representative, except as otherwise approved.
25. Special care shall be taken to ensure that each section of piping abuts against the next in such a manner that there will be not shoulder or unevenness of any kind along the piping invert.
26. Piping shall be rotated as required to place outlets in proper position.
27. Blind flanges and cleanouts shall be provided at locations shown on the Drawings, specified or required. Cleanouts on buried piping shall include all pipe, fittings and appurtenances required to bring cleanout to finished grade and terminate in a flange and blind flange or suitably capped piping as shown on the Drawings. Cleanout piping shall be same as that specified for the main run.
28. All gravity lines shall pitch uniformly at the grade shown on the Drawings or as specified or approved by the ENGINEER.
29. Short pipe stubs, maximum 4-feet-0-inch in length, shall be used at all manholes and other wall faces, except as otherwise specified.
30. Field painting shall be accomplished after joints are made.
31. All piping shall be plugged watertight with a suitable cap or plug securely fastened to the end of the piping at all contact interfaces.
32. On steep slopes, take measures acceptable to ENGINEER to prevent movement of the pipe during installation.
33. Exercise care to avoid flotation when installing pipe in cast-in-place concrete.
34. For copper tubing and thermoplastic piping, snake piping in trench to compensate for thermal expansion.

- B. Manufacturer's Installation Specialist:
 - 1. Provide the services of a competent installation specialist of the pipe manufacturer when pipe laying commences if CONTRACTOR is not experienced in laying and jointing a particular type of pipe for the following:
 - a. HOBAS pipe.
 - b. Thermoplastic pipe.
 - 2. Retain installation specialist at the site for a minimum of two days or until competency of the pipe laying crew has been satisfactorily demonstrated.
- C. Separation of Sewers and Potable Water Pipe Lines:
 - 1. Conform to requirements of MAG Specification Section 610.5.
- D. Plugs:
 - 1. Temporarily plug installed pipe at the end of each day's Work or other interruption to the installation of any pipeline. Plugging shall prevent the entry of animals, liquids or persons into the pipe or the entrance or insertion of deleterious materials.
 - 2. Install standard plugs into all bells at dead ends, tees or crosses. Cap all spigot ends.
 - 3. Fully secure and block all plugs and caps installed for pressure testing to withstand the specified test pressure.
 - 4. Where plugging is required for phasing of the Work or for subsequent connection of piping, install watertight, permanent type plugs.
- E. Laying Pipe:
 - 1. Conform to manufacturer's instructions and requirements of the standards listed below, where applicable:
 - a. Ductile Iron Pipe: AWWA C600, AWWA C105.
 - b. Concrete Pipe: AWWA M9, Concrete Pipe Handbook.
 - c. Steel Pipe: AWWA M11, AWWA C206.
 - d. Thermoplastic Pipe: ASTM D 2774.
 - e. Fiberglass Reinforced Polymer Mortar Pipe: ASTM D3262.
 - f. ASCE Manual of Practice No. 37.
- F. Jointing Pipe:
 - 1. CCFRP (HOBAS) Pipe:
 - a. Unless otherwise specified, the pipe shall be field connected with fiberglass sleeve couplings that utilize built-in double fin integral elastomeric seal gasket made of EPDM rubber compound, which must be mechanically locked to FRP backing, as the sole means to maintain joint water tightness. The joints must meet the performance requirements of ASTM D4161, and be leak free after field testing.
 - 2. Thermoplastic Pipe Joints:
 - a. Solvent Cement Joints:

- 1) Bevel pipe ends and remove all burrs before making joints. Clean both pipe and fittings thoroughly. Do not attempt to make solvent cement joints if temperature is below 40°F or above 90°F when exposed to direct sunlight or in wet conditions.
 - 2) Use solvent cement supplied or recommended by the pipe manufacturer.
 - 3) Apply joint primer and solvent cement and assemble joints in strict accordance with the recommendations and instructions of the manufacturer of the joint materials and the pipe manufacturer.
 - 4) Observe safety precautions with the use of joint primers and solvent cements. Allow air to circulate freely through pipelines to permit solvent vapors to escape. Slowly admit water when flushing or filling pipelines to prevent compression of gases within pipes.
- b. Push-On Joints:
- 1) Bevel all field-cut pipes, remove all burrs and provide a reference mark the correct distance from the pipe end.
 - 2) Clean the pipe end and the bell thoroughly before making the joint. Insert the O-ring gasket, making certain it is properly oriented. Lubricate the spigot well with an approved lubricant; do not lubricate the bell or O-ring. Insert the spigot end of the pipe carefully into the bell until the reference mark on the spigot is flush with the bell.
3. Mechanical Coupling Joints:
- a. Prior to the installation and assembly of mechanical couplings, the joint ends shall be cleaned thoroughly with a wire brush to remove foreign matter. Following this cleaning, lubricant shall be applied to the rubber gasket or inside of the coupling housing and to the joint ends. After lubrication, the gasket shall be installed around the joint end of the previously installed piece and the joint end of the subsequent piece shall be mated to the installed piece. The gasket shall be positioned and the coupling housing placed around the gasket and over the grooved or shouldered joint ends. The bolts shall be inserted and the nuts screwed up tightly by hand. The bolts shall then be tightened uniformly in order to produce an equal pressure on all parts of the housing. When the housing clamps meet metal to metal, the joint is complete and further tightening is not required.
- G. Transitions from One Type of Pipe to Another:
1. Provide all necessary adapters, specials and connection pieces required when connecting different types and sizes of pipe or connecting pipe made by different manufacturers.
- H. Closures:
1. Provide all closure pieces shown on the Drawings or required to complete the Work.

3.7 BACKFILL

A. General:

1. Backfill begins after the placement of the pipe bedding. Pipeline trenches may be backfilled prior to pressure testing, but no structure shall be constructed over any pipeline until it has been tested.
2. Place and compact backfill as construction progresses.
3. Compacted backfill shall be required for the full depth of the trench above the flowable pipe embedment material. Where the trench for one pipe passes beneath the trench for another pipe or electrical duct bank, the lower trench shall be compacted to the level of the bottom of the upper trench.
4. Each layer of backfill material shall be compacted by at least two complete coverages of all portions of the surface of each lift using approved compaction equipment. One coverage is defined as the conditions reached when all portions of the fill lift have been subjected to the direct contact of the compacting surface of the compactor.
5. The method of compaction and the equipment used shall be appropriate for the material to be compacted and shall not transmit damaging shocks to the pipe.
6. The degree of compaction required for all types of fills shall be as listed below. Material shall be moistened or aerated as necessary to provide the moisture content that will facilitate obtaining the specified compaction.
7. The trench backfill shall be thoroughly compacted to no less than the following densities when tested and determined by ASSHTO T-99 and T-191 or ASTM D 2922 and D 3017. When ASSHTO T-99, Method A or B, and T-191 are used for density determination, MAG Detail 190 will be used for rock correction. The minimum density required is identified below:

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Material Thickness (inches)	Required Minimum Density-Percent Compaction (ASTM D 698)	Maximum Uncompacted Lift (inches)
Aggregate Base Course	100	8
Trench Backfill above pipe – Deeper than 2 feet below pavement	95	8
Trench Backfill above pipe – Within 2 feet below pavement	100	8
Backfill Material	95	8
Flowable Backfill*	See below *	See below *

* Flowable backfill shall be in accordance with Paragraph 2.1.A., above. Testing shall be in accordance with MAG 728 and testing methods described in Section 03300, Cast-In-Place Concrete.

- a. All fill must be wetted and thoroughly mixed to achieve optimum moisture content, within two percent, with the following exceptions: For on-site clayey soils, the optimum moisture content may range from the target value to ± 2 percent.
- b. Natural undisturbed soils or compacted soil subsequently disturbed or removed by construction operations shall be replaced with materials compacted as specified above.
8. CONTRACTOR'S testing service shall perform tests required to provide data for selection of fill material and control of placement water content.
9. Field density tests, to ensure that the specified density is being obtained, shall be performed by CONTRACTOR'S testing service during each day of compaction Work.
10. If the tests indicate unsatisfactory compaction, provide the additional compaction necessary to obtain the specified degree of compaction. All additional compaction work shall be performed by CONTRACTOR, at no additional cost to the OWNER, until the specified compaction is obtained. This work shall include complete removal of unacceptable (as determined by the ENGINEER) fill areas and replacement and recompaction until acceptable fill is provided.

- B. Replacement of Unacceptable Excavated Materials: In cases where over-excavation for the replacement of unacceptable soil materials is required, the excavation shall be backfilled to the required subgrade with select backfill material and thoroughly compacted as specified in Paragraph 3.7., above. Sides of the excavation shall be sloped in accordance with the maximum inclinations specified for each structure location.
- C. Perform backfill around structures using the specified procedures, except that within ten feet of foundations and underground structures, light compaction equipment shall be used, with the gross weight of the equipment not exceeding 7,000 lbs. Provide equipment that is capable of the required compaction within restricted areas next to structures and around piping.

3.8 GRADING

- A. General: Uniformly grade areas within limits of grading shown on the Drawings or specified, including adjacent transition areas. Smooth subgrade surface within specified tolerances, compact with uniform levels or slopes between points where elevations are shown on the Drawings, or between such points and existing grades.
- B. Walks: Shape surface of areas under walks to line, grade and cross-section, with finish surface not more than 1-inch above or below the required subgrade elevation.
- C. Pavements: Shape surface of areas under pavements to line, grade and cross-section, with finish surface not more than 1/2-inch above or below the required subgrade elevation.
- D. Slabs: Grade smooth and even, free of voids, compacted as specified, and to required elevation. Provide final grades within a tolerance of 1/2-inch when tested with a 10-foot straightedge.
- E. Compaction: After grading, compact subgrade surfaces to the depth and percentage of maximum density required.

3.9 PAVEMENT SUBBASE COURSE

- A. Grade Control: During construction, maintain lines and grades including crown and cross-slope of subbase course.
- B. Shoulders: Place shoulders along edges of subbase course to prevent lateral movement. Construct shoulders of acceptable soil materials, placed in such quantity to compact to thickness of each subbase course layer. Compact and roll at least a 12-inch width of shoulder simultaneously with compacting and rolling of each layer of subbase course.

- C. Placing: Place subbase course material on prepared subgrade in layers of uniform thickness, conforming to indicated cross-section and thickness. Maintain optimum moisture content for compacting subbase material during placement operations.
 - 1. When a compacted subbase course is shown on the Drawings to be 6-inches thick or less, place material in a single layer. When shown on the Drawings to be more than 6-inches thick, place material in equal layers, except no single layer more than 6-inches or less than 3-inches in thickness when compacted.

3.10 DISPOSAL OF EXCAVATED MATERIALS

- A. Material removed from the excavations which does not conform to the requirements for fill or is in excess of that required for backfill shall be hauled away by CONTRACTOR and disposed of in compliance with municipal, county, state, federal or other applicable regulations at no additional cost to OWNER.

3.11 RESTORING AND RESURFACING EXISTING ROADWAYS AND FACILITIES

- A. Place 1-1/2-inches of temporary bituminous pavement immediately after backfilling trenches in paved roadways. Maintain the surface of the paved area over the trench in good and safe condition during progress of the entire Work, and promptly fill all depressions over and adjacent to the trench caused by settlement of backfill. Immediately prior to constructing the permanent paving and base, remove and dispose of temporary pavement. Permanent replacement pavement shall be equal to that of the existing roadways unless otherwise shown on the Drawings or specified.
- B. Pavement, gutters, curbs, walks, driveways and roadways disturbed or damaged by CONTRACTOR'S operations, except areas designated "New Pavement" or "Proposed Pavement", shall be restored or replaced at CONTRACTOR'S expense to as good condition as they were previous to the commencement of the Work and in accordance with applicable local and state highway specifications.

3.12 WORK AFFECTING EXISTING PIPING

- A. Location of Existing Piping:
 - 1. Locations of existing piping shown on the Drawings should be considered approximate.
 - 2. Determine the true location of existing piping to which connections are to be made, and location of other facilities which could be disturbed during earthwork operations, or which may be affected by CONTRACTOR'S Work already installed.
 - 3. Conform to applicable requirements of Division 1, General Requirements, pertaining to cutting and patching, and connections to existing facilities.

4. CONTRACTOR shall pothole all utility crossings as shown per the contract drawings for all new utilities, i.e., electrical conduits, piping, instrumentation conduits, irrigation, etc, crossing existing utilities prior to construction. CONTRACTOR shall provide as-builts showing elevations and northing and eastings of all crossings.
- B. Taking Existing Pipelines Out of Service:
1. Do not take pipelines out of service unless specifically listed below, or approved by ENGINEER.
- C. Work on Existing Pipelines:
1. Cut or tap pipes as shown on the Drawings or required with machines specifically designed for this Work. All taps shall be made at the spring line except for air release taps. Taps to buried piping 16-inches in diameter and larger shall be protected by an access vault to the surface or shall be 4-inches larger, with a shut off ball valve and a riser to the surface. Taps made to pipe smaller than 16-inches may be 2-inch taps. Taps shall not be located under slabs.
 2. Install temporary plugs to prevent entry of mud, dirt, water and debris.
 3. Provide all necessary adapters, fittings, pipe and appurtenances required to complete the Work. Provide all necessary adapters, fittings, pipe and appurtenances required to complete the Work.
 4. Existing pipelines which are cut and abandoned shall be adequately capped or filled with grout.

3.13 TESTING OF PIPING

- A. General:
1. Test all piping, except as otherwise authorized by ENGINEER.
 2. Notify ENGINEER and local authorities having jurisdiction at least 48 hours in advance of testing if their presence is required.
 3. Conduct all tests in the presence of the ENGINEER.
 4. Remove or protect any pipeline-mounted devices which may be damaged by the test pressure.
 5. Provide all apparatus and services required for testing, including but not limited to, the following:
 - a. Test pumps, bypass pumps, hoses, calibrated gauges, meters, test containers, valves and fittings.
 - b. Temporary bulkheads, bracing, blocking and thrust restraints.
 6. Provide air if an air test is required and power if pumping is required.
 7. Unless otherwise specified, OWNER will provide fluid required for testing.
 8. Repair and retest pipelines that fail to hold specified test pressure or which exceed the allowable leakage rate.
 9. Unless otherwise noted, pipelines shall hold specified test pressure for two hours.

10. Unless otherwise specified, test pressures required are at the lowest elevation of the pipeline section being tested.
- B. Schedule of Pipeline Tests:
1. Refer to Section 15050, Piping Systems, for the type of test required and the required hydrostatic test pressure.
 2. Unless otherwise specified, the required hydrostatic test pressures are at the lowest elevation of the pipeline.
 3. For piping not listed in Section 15050, Piping Systems:
 - a. Hydrostatically test pipe that will be operating at a pressure greater than five psig.
 - b. Use exfiltration testing or low-pressure air testing for all other piping.
 4. Hydrostatic Test Pressure:
 - a. Use test pressures listed in Section 15050, Piping Systems.
 - b. If a test pressure is not listed in Section 15050, Piping Systems, or if a hydrostatic test is required for piping not listed in Section 15050, Piping Systems, the test pressure will be determined by the ENGINEER based on the maximum anticipated sustained operating pressure and the methods described in the AWWA Manual or Standard which applies to the piping system.
 5. CONTRACTOR shall provide a test plan 14 days prior to performing the hydrostatic pressure tests on the pipe lines.
- C. Required Tests for Gravity Sewer:
1. Elect to test piping, using either air or water test procedures. Notify ENGINEER, in writing, in advance of all testing, which method he plans to utilize and must follow through with the same method on all pipeline testing.
 - a. Gravity sewers shall be tested with either air or water testing; however, storm drains may only be water tested.
 - b. Tests shall be performed after backfilling is completed, but shall be performed before final cleanup and acceptance of Work.
 - c. Tests shall be performed prior to final acceptance.
 - 1) Test all piping for leakage by means of the tests described below.
 - 2) Test to be performed between adjacent manholes or as approved by the ENGINEER.
 - d. Prior to making tests, submit details of his testing procedures, with a description of methods and equipment CONTRACTOR proposes to use, to the ENGINEER for approval. Furnish all necessary labor, equipment, water, watertight bulkheads, rodding machine, generator, pumps and all else necessary to carry out the required tests.
 2. Air Test:
 - a. Wet and thoroughly clean the inside of the pipe before test is performed.
 - b. Insert test plugs in ends of pipe to be tested.
 - c. Securely brace test plugs.

- d. Measure and record groundwater height above the pipe invert. All gage pressures in the test shall be increased by the amount of the back pressure due to groundwater submergence.
- e. Slowly fill the pipe with air to a pressure of four psig. Maintain pressure between 4 and 3.5 psig for at least two minutes for temperature stabilization.
- f. Check all plugs for tightness.
- g. With a pressure of approximately four psig in pipe, disconnect air supply.
- h. Allow pressure to decrease to 3.5 psig.
- i. When the pressure reaches 3.5 psig, record the time required to decrease to 2.5 psig using a stopwatch.
- j. The line is considered acceptable if the time for the pressure to decrease from 3.5 psig to 2.5 psig is not less than the amount shown on the following table for the respective pipe diameters.

Minimum Acceptance Times	
Pipe Diameter (Inches)	Time, min/100 ft
48	8.5 min.
66	12.1 min.

- k. If the leakage in the section tested exceeds the specified amount, make the necessary repairs or replacements required to reduce the leakage to within the specified limits and the test shall be repeated until the leakage requirement is met.
 - l. No one shall be allowed in the manhole during air testing.
3. Water Test:
- a. When water test is performed the sewer pipe test section shall be filled with water and allowed to stand for 24 hours. The water shall then be replenished and the test performed.
 - b. Insert test plugs and securely brace.
 - c. Fill the pipe and manhole with water to provide a positive differential head on the top of the pipe at the highest point of the pipeline under test of at least the test pressure specified in Section 15050, Piping Systems.
 - d. The amount of water added to maintain this head shall be the leakage.
 - e. Test for a period of at least four hours.
 - f. Total leakage of any section tested shall not exceed the following rates:
 - 1) Gravity Sewer: 0.5 gallons per hour per 100 feet of pipe per inch diameter of pipe.

- g. If the leakage in the section tested exceeds the specified amount, make the necessary repairs or replacements required to reduce the leakage to within the specified limits and the test shall be repeated until the leakage requirements is met.
 - h. On steep grades it may be necessary to place plugs in the pipe between manholes to avoid excessive pressures in the pipe.
 - 4. Visual Inspection:
 - a. Prior to final acceptance, a visual inspection by ENGINEER of all appurtenant structures, (e.g., manholes, chambers, etc.), shall be required. Any visual leaks, regardless of their magnitude shall be repaired by CONTRACTOR.
 - 5. Watertight Sewers:
 - a. It is imperative that all sewers and appurtenant structures be constructed as watertight as practicable. Adhere rigidly to all requirements of the Contract Documents and follow all directions of the ENGINEER to secure a watertight sewer. If, during the Work or after its completion, any leaks are discovered, they shall be repaired in a satisfactory manner at the expense of CONTRACTOR even though the pipe and appurtenant structures may have already successfully passed the leakage tests.
- D. Sanitary Sewer Manhole Testing:
 - 1. Manholes shall be tested for water tightness per MAG Standard Specification Section 611.
- E. Vertical Deflection Test for Thermoplastic Pipe:
 - 1. The vertical deflection test shall be conducted after the final backfill has been in place at least 30 days.
 - 2. Manually pull a pin-type vertical gauge mounted on a sled through the pipe. Gauge shall be set so that if vertical deflection of pipe exceeds five percent, it will stop. Excavate and reinstall all such piping. Gauge shall be as manufactured by Quality Test Products, or equal.

3.14 DISPOSAL OF WATER

- A. Provide suitable means for disposal of test and flushing water so that no damage results to facilities or waterways.
- B. Means of disposal of test and flushing water shall be subject to the approval of ENGINEER, local governing authorities and regulatory agencies.
- C. Responsibility belongs to CONTRACTOR for any damage caused by water disposal operations.

CITY OF CHANDLER: Public Works and Utilities Department
PROJECT NAME: Loop 101 and 202 Freeway 66-inch Redundant Sewer Project
PROJECT NUMBER: WW2402.401

3.15 CLEANING

- A. Cleaning:
1. Thoroughly clean all piping and flush in a manner approved by ENGINEER, prior to placing in service.
 2. Piping 24-inches in diameter and larger shall be inspected from inside and all debris, dirt and foreign matter removed.

+ + END OF SECTION + +

SECTION 15106

THERMOPLASTIC PIPE

PART 1 - GENERAL

1.1 DESCRIPTION

- A. Scope:
1. Provide all labor, materials, equipment and incidentals as shown on the Drawings, specified and required to furnish and install and place in satisfactory service polyvinyl chloride (PVC) piping, fitting and specials.
 2. The extent of piping is shown on the Drawings and Section 15050, Piping Systems.
- B. Coordination:
1. Review installation procedures under other Sections and coordinate with the Work that is related to this Section.

1.2 QUALITY ASSURANCE

- A. Manufacturer's Qualifications:
1. Manufacturers of thermoplastic pipe and fittings shall have a minimum of five years of experience producing thermoplastic pipe and fittings, and shall be able to show evidence of at least five installations in satisfactory operation.
- B. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown or specified.
1. ASTM D 1598, Test Method for Time-to-Failure of Plastic Pipe Under Constant Internal Pressure.
 2. ASTM D 1599, Test Method for Short-Time Hydraulic Failure Pressure of Plastic Pipe, Tubing and Fittings.
 3. ASTM D 1784, Specification for Rigid Poly (Vinyl Chloride) (PVC) Compounds and Chlorinated Poly (Vinyl Chloride) (CPVC) Compounds.
 4. ASTM D 2122, Test Method for Determining Dimensions of Thermoplastic Pipe and Fittings.
 5. ASTM D 2774, Practice for Underground Installation of Thermoplastic Pressure Piping.
 6. ASTM D 2846, Specification for Chlorinated Poly (Vinyl Chloride) (CPVC) Plastic Hot- and Cold-Water Distribution Systems.
 7. ASTM D 3034, Specification for Type PSM Poly (Vinyl Chloride) (PVC) Sewer Pipe and Fittings.
 8. ASTM D 3139 Standard Specification for Joints for Plastic Pressure Pipes Using Flexible Elastomeric Seals

9. ASTM F 437, Specification for Threaded Chlorinated Poly (Vinyl Chloride) (CPVC) Plastic Pipe Fittings, Schedule 80.
10. ASTM F 439, Specification for Chlorinated Poly (Vinyl Chloride) (CPVC) Plastic Pipe Fittings, Schedule 80.
11. ASTM F 441, Specification for Chlorinated Poly (Vinyl Chloride) (CPVC) Plastic Pipe, Schedules 40 and 80.
12. ASTM F 477, Specification for Elastomeric Seals (Gaskets) for Joining Plastic Pipe.
13. ASTM F 493, Specification for Solvent Cements for Chlorinated Poly (Vinyl Chloride) (CPVC) Plastic Pipe and Fittings.
14. ASTM F 1417, Standard Practice for Installation Acceptance of Plastic Non-pressure Sewer Lines Using Low-Pressure Air
15. Standard No. 14, National Sanitation Foundation.
16. American National Standards Institute.

C. Shop Tests:

1. Piping manufacturer shall maintain a continuous quality control program. All PVC plastic molding materials used to manufacture pipe and fittings under this Section shall be tested for conformance to the requirements of ASTM D 3034.

1.3 SUBMITTALS

A. Shop Drawings: Submit for approval the following:

1. Detailed procedures to be used in jointing and installing piping system including manufacturer's recommendations.
2. Interfacing of piping system to equipment and appurtenances.
3. Detail requirements for burial, supports, anchors, guides, expansion joints, and all accessories required for a satisfactory piping system.
4. Bill of materials, indicating material composition of pipe, fittings and solvent, pressure rating, nominal size and its location on the piping installation drawings.
5. Certifications letter from pipe manufacturer confirming that the materials to be used are suitable for the intended service.
6. Comply with the requirements of Section 01332, Shop Drawing Procedures.
7. CONTRACTOR shall submit an electronic copy per Specification Section 01332, Shop Drawing Procedures.
8. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final

authority for determining acceptability of requested deviations. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration.

- B. Certificates: Submit certificates of compliance with referenced standards.

1.4 PRODUCT DELIVERY, HANDLING AND STORAGE

- A. Delivery: All necessary precautions shall be taken to prevent damage to pipe fittings and other materials during shipment and delivery. All materials shall be securely fastened to truck or rail car to prevent movement or damage during shipment. All materials shall be inspected by CONTRACTOR, upon delivery to the site.
- B. Handling: All pipe materials shall be handled to prevent damage. Pipe and fittings shall not be dropped, rolled, or pushed off from any height on delivery, storage or installation.
- C. Storage: All pipe materials shall be stored off the ground. Pipe ends shall be secured by caps or plugs. Do not store pipe or fittings in sunlight. Pipe shall be stored to prevent sagging or bending. Store off the ground, under cover, and in a dry location.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. PVC Pipe:
1. PVC pipe shall be SDR 26, conforming to ASTM D 3034.
 2. Pipe Compound shall conform to ASTM D1784 Cell Class 12454/12364, gaskets shall conform to ASTM F477, and integral bell joints shall conform to ASTM D3212.

2.2 DETAILED REQUIREMENTS

- A. Workmanship: The pipe shall be homogeneous throughout and free from visible cracks, holes, foreign inclusions or other defects. The pipe shall be uniform in color, opacity, density, and other physical properties.
- B. Dimensions and Tolerances: Dimensions and tolerances shall be measured in accordance with ASTM D 2122. The eccentricity of the inside and outside circumferences of the pipe walls shall not exceed 12 percent.

- C. Sustained Pressure: The pipe shall not fail, balloon, burst, or weep as defined in ASTM D 1598.
- D. Burst Pressure: The minimum burst pressure shall be determined in accordance with ASTM D 1599.
- E. Meets or exceeds ASTM D3219 for joint tightness, including a 22-inch Hg vacuum and a 25-foot head pressure test. When testing/ qualifying the line, low pressure air testing is acceptable in accordance with ASTM F1417.
- F. Marking: Marking on the pipe shall include the following, spaced at intervals of not more than five feet.
 - 1. Pipe nominal size.
 - 2. Pipe schedule.
 - 3. Specification of plastic material.
 - 4. Type and grade of plastic.
 - 5. Date and place of manufacture.
- G. Piping and fittings shall be manufactured with a minimum of two percent of titanium oxide for ultraviolet protection.

2.3 ADAPTERS

- A. Where required to join piping of different materials, provide the required adapters, as recommended by the thermoplastic pipe manufacturer.

PART 3 - EXECUTION

3.1 INSTALLATION

- A. General:
 - 1. Refer to Section 15050, Piping Systems, Section 15051, Buried Piping Installation for piping installation, testing, cleaning and acceptance.
 - 2. Request instructions from ENGINEER before proceeding if there is a conflict between Contract Documents and manufacturer's recommendations.
 - 3. Pipe, fittings and accessories that are cracked, damaged, not identified or in poor condition will be rejected.

+ + END OF SECTION + +

SECTION 15108.1

CENTRIFUGALLY CAST, FIBERGLASS-REINFORCED
POLYMER MORTAR PIPE

PART 1 - GENERAL

1.1 DESCRIPTION

A. Scope:

1. Provide all labor, materials, equipment and incidentals as shown on the Drawings, as specified and as required to furnish and install centrifugally cast, fiberglass-reinforced polymer mortar piping.
2. The extent of centrifugally cast fiberglass-reinforced polymer mortar pipe to be furnished is shown on the Drawings.

1.2 QUALITY ASSURANCE

A. Manufacturer's Qualifications:

1. Manufacturers of centrifugally cast, fiberglass-reinforced polymer mortar pipe and fittings shall have a minimum of five years' experience of producing centrifugally cast, fiberglass-reinforced polymer mortar pipe and fittings, and shall be able to show evidence of at least five installations in satisfactory operation.

B. Reference Standards: Comply with applicable provisions and recommendations of the following, except as otherwise shown or specified.

1. ASTM D 2412, Test Method for Determination of External Loading Characteristics of Plastic Pipe by Parallel-Plate Loading.
2. ASTM D 3262, Specification for "Fiberglass" (Glass-Fiber-Reinforced Thermo-setting-Resin) Sewer Pipe.
3. ASTM D 3681, Test Method for Chemical Resistance of "Fiberglass" (Glass-Fiber-Reinforced Thermosetting-Resin Pipe) in a Deflected Condition.
4. ASTM D 4161, Specification for "Fiberglass" (Glass-Fiber-Reinforced Thermo-setting-Resin) Pipe Joints Using Flexible Elastomeric Seals.

C. Shop Tests:

1. Manufacturer shall maintain a continuous quality control program and shall furnish the ENGINEER with certified test reports.
2. Tests shall be witnessed by a Registered Professional Engineer or a member of ASTM, who may be an employee of the manufacturer. The Registered Professional Engineer shall seal and sign all test reports.
3. Cut a three-foot section from each size pipe furnished, and perform the following tests using methods recommended by the ASTM:

- a. ASTM D 2412, Test Method for Determination of External Loading Characteristics of Plastic Pipe by Parallel-Plate Loading.
- b. ASTM D 3262, Specification for "Fiberglass" (Glass-Fiber-Reinforced Thermosetting-Resin) Sewer Pipe.
- c. ASTM D 3681, Test Method for Chemical Resistance of "Fiberglass" (Glass-Fiber-Reinforced Thermosetting-Resin) Pipe in a Deflected Condition.

1.3 SUBMITTALS

- A. Shop Drawings: Submit for approval the following:
 1. Details of construction, fabrication, and specifications for pipe laminate construction.
 2. Details of piping system including: Location of supports, fittings, anchors, and all accessories necessary for piping system.
 3. Pipe laying schedules.
 - a. Submit these with Shop Drawings required under Section 15051, Buried Piping Installation.
 4. A copy of this specification section, with addendum updates included, and all referenced and applicable sections, with addendum updates included, with each paragraph check-marked to indicate specification compliance or marked to indicate requested deviations from specification requirements. Check marks (✓) shall denote full compliance with a paragraph as a whole. If deviations from the specifications are indicated, and therefore requested by the CONTRACTOR, each deviation shall be underlined and denoted by a number in the margin to the right of the identified paragraph, referenced to a detailed written explanation of the reasons for requesting the deviation. The ENGINEER shall be the final authority for determining acceptability of requested deviations. The CONTRACTOR shall furnish equipment and/or services as specified if an exception and/or deviation is rejected. The remaining portions of the paragraph not underlined will signify compliance on the part of the CONTRACTOR with the specifications. Failure to include a copy of the marked-up specification sections, along with justification(s) for any requested deviations to the specification requirements, with the submittal shall be sufficient cause for rejection of the entire submittal with no further consideration. The CONTRACTOR shall submit the preventive maintenance information package as part of the shop drawing submittal package to the ENGINEER for review and approval.
- B. Test Reports: Submit reports for any tests required above with test specimens.
- C. Certificates: Submit certificates of compliance with referenced standards.

1.4 PRODUCT DELIVERY, STORAGE AND HANDLING

- A. Refer to Sections 15051, Buried Piping Installation.

PART 2 - PRODUCTS

2.1 SERVICE CONDITIONS

- A. Centrifugally cast, fiberglass-reinforced polymer mortar piping system shall be specifically designed, constructed, and installed for the service intended and shall comply with the following service conditions.
- B. Service Conditions:

Location:	New Gravity Sewer Pipes (Open Cut)	New Gravity Sewer Pipe inside 86" Dia. (Outside Dia.) Steel Casing (Casing Carrier)
Nominal Pipe Size (in):	66	66
Joint Type	FWC Coupling	Flush Reline
Pipe Pressure Class (psi):	25	25
Pipe Stiffness (psi):	46	71
Min. Wall Thickness (in):	1.27	1.45
Nominal Pipe Length (ft.):	20	20
Fluid Temperature (°F max):	100	100
Fluid Flow Max. (MGD):	28	28
Fluid Chemical Composition	Raw Wastewater	Raw Wastewater

2.2 MATERIALS

- A. Resin Systems: The manufacturer shall use only polyester resin systems.

- B. Glass Reinforcements: The reinforcing glass fibers used to manufacture the components shall be of the highest quality commercial grade E-glass filaments with binder and sizing compatible with impregnating resins.
- C. Silica Sand: Sand shall be minimum 98 percent silica with a maximum moisture content of 0.2 percent.
- D. Additives: Resin additives, such as curing agents, pigments, dyes, fillers, thixotropic agents, etc., when used, shall not detrimentally affect the performance of the product.
- E. Elastomeric Gaskets: Gasket shall be a full face EPDM gasket with double fin on each side of center register. Overall coupling width and sealing surface shall be: 10" wide for pipe sizes 24" thru 54" in diameter and 11.5" wide for pipe sizes 57" thru 78" in diameter. Gaskets shall be supplied by qualified gasket manufacturers and be suitable for the service intended.

2.3 DETAILS OF CONSTRUCTION

- A. Pipes: Manufacture pipe by the centrifugal casting process only, so it will result in a dense, non-porous, corrosion-resistant, consistent composite structure. Non-reinforced, unlined or statically cast polymer products are not acceptable. Pipes made with filament-winding construction either by continuous mandrel winding or reciprocating winding process are not allowed. The pipe shall meet the ASTM D3262 Standard Specification CELL LIMITS Type1, Liner 2, Grade 3 and Pipe Stiffness Class C or higher. The manufacturer of pipe materials must demonstrate a 5-year recent history of successful installations with over 2.5 million lineal feet in the United States for sanitary sewer service.
- B. Joints: Unless otherwise specified, the pipe shall be field connected with fiberglass sleeve couplings that utilize built-in double fin integral elastomeric seal gasket made of EPDM rubber compound, which must be mechanically locked to FRP backing, as the sole means to maintain joint water tightness. The joints must meet the performance requirements of ASTM D4161, and be leak free after field testing.
- C. Corrosion Liner: CCFRPM pipe liner shall be a non-reinforced layer of nominal 0.040 inches (40 mil) and maximum 0.100 inches (100 mil) flexibilized polyester resin as an integral part of pipe wall construction. No fiberglass material in the form of glass fabric, filament winding or otherwise shall be present in the liner layer
- D. Fittings: Flanges, elbows, reducers, tees, wyes, laterals and other fittings shall be capable of withstanding all operating conditions when installed. They may be contact molded or manufactured from mitered sections of pipe joined by glass-fiber-reinforced overlays. Properly protected standard ductile iron, fusion-bonded epoxy-coated steel and stainless steel fittings may also be used.

- E. Diameters: The actual outside diameter of the pipes shall be in accordance with ASTM D 3262.
- F. Lengths: Pipe shall be supplied in nominal lengths of 20 feet. Actual laying length shall be nominal +1, -4 inches. At least 90% of the total footage of each size and class of pipe, excluding special order lengths, shall be furnished in nominal length sections
- G. Wall thickness: The minimum wall thickness shall be the stated design thickness.
- H. End Squareness: Pipe ends shall be square to the pipe axis with a maximum tolerance of 1/8-inch.
- I. Product and Manufacturer: Provide one of the following:
 - 1. Hobas Pipe USA, Inc.
 - 2. Or equal.

2.4 MAINTENANCE TOOLS AND SPARE MATERIALS

- A. If required, furnish and deliver the following tools and maintenance materials carefully boxed or packaged and plainly marked for recording:
 - 1. Names and addresses of all manufacturers of fiberglass reinforcements, resins, hardeners and components used to repair and maintain centrifugally cast, fiberglass-reinforced polymer mortar piping system.
 - 2. Contact Hobas Pipe USA Field Service department for review and inspection as required prior to ordering field repair materials.
- B. Store and safeguard tools and materials until completion of the Work, at which time they shall be inventoried, delivered and placed in an area designated by OWNER.

2.5 MARKING FOR IDENTIFICATION

- A. All pipe line materials shall be permanently marked with the following:
 - 1. Name or trademark of manufacturer.
 - 2. Pipe class and specification designation.
 - 3. Size and length dimensions.
 - 4. Date and place of manufacture.

PART 3 - EXECUTION

3.1 INSTALLATION

- A. For buried piping installation, refer to Section 15051, Buried Piping Installation.

- B. Pipe Handling: Use textile slings, other suitable materials or a forklift. Use of chains or cables is not recommended.
- C. Jointing:
 - 1. Clean ends of pipe and coupling components.
 - 2. Apply joint lubricant to pipe ends and elastomeric seals of coupling. Use only lubricants approved by the pipe manufacturer.
 - 3. Use suitable equipment and end protection to push or pull the pipes together.
 - 4. Do not exceed forces recommended by the manufacturer for coupling pipe.
 - 5. Join pipes in straight alignment then deflect to required angle. Do not allow the deflection angle to exceed the deflection permitted by the manufacturer.
- D. For pipe testing, refer to Section 15050, Piping Systems.

3.2 LEAKAGE TESTS

- A. For buried piping leakage tests, refer to Section 15051, Buried Piping Installation.
- B. Low Pressure Air Test: Each reach may be tested with air pressure (max 5 psi). The system passes the test if the pressure drop due to leakage through the pipe or pipe joints is less than or equal to the specified amount over the prescribed time period.
- C. Individual Joint Testing: For pipes large enough to enter, individual joints may be pressure tested with a portable tester to 5 psi max. with air or water in lieu of line infiltration, exfiltration or air testing.
- D. Deflection: Maximum allowable long-term deflection is normally 5% of the initial diameter.

+ + END OF SECTION + +

EXHIBIT D

ACCEPTED GMP/PRICE PROPOSAL

INDEX OF EXHIBITS
LOOP 101 AND 202 FREEWAY
66-INCH REDUNDANT SEWER PROJECT
COC Project No.: WW2402.401
Guaranteed Maximum Price

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- Exhibit D2: GMP Coversheet
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- Exhibit D7: Risk Register
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EXHIBIT D1: SCOPE OF WORK
LOOP 101 AND 202 FREEWAY
66-INCH REDUNDANT SEWER PROJECT
 COC Project No.: WW2402.401
 Guaranteed Maximum Price

PROJECT OVERVIEW

The project includes installing a redundant 66" diameter gravity sewer line that crosses the Loop 202 Santan Freeway. The new 66" diameter gravity sewer line installations consist of approximately 776 linear feet of open cut installation, 639 linear feet of trenchless 66-inch sewer line installation within an 88-inch steel casing that crosses the Loop 202, 4 new junction structures, and 4 each new 60" polymer concrete manholes with 96" precast bases. The new gravity sewer system also includes installing 203 linear feet of 48" PVC SDR-26 sewer line, removing sections of existing 48" sewer lines for construction of the new system, and abandonment of those existing 48" sewer lines and corresponding structures. In addition, this project includes rehabilitating existing 30", 48", and 66" diameter sewer lines, which includes installation of cured-in-place pipe liners and polymer concrete manhole inserts.

SCOPE OF WORK

Below chart demonstrates the direct activities of the project to complete the work.

1	Mobilization And Demobilization	1	LS
2	SWPPP	15	MO
3	Install Pipe Plug Per MAG Standard Detail 427	8	EACH
4	Remove Existing 48" Sanitary Sewer As Needed For Construction Of New 66" (HOBAS) Pipe	63	LF
5	Grout Fill Existing 48" Sanitary Sewer To Extents Shown	387	LF
6	Abandon Existing 108" Dia Manhole 756009379	1	EACH
7	Abandon Existing Junction Structure No. 2	1	EACH
8	Furnish And Install 48" Sanitary Sewer (PVC SDR-26)	203	LF
9	Remove Existing Plug And Connect To Existing 66" Stub Out	1	EACH
10	Furnish & Install 66" Sanitary Sewer (Hobas FWC Coupling Joint)	776	LF
11	Remove Existing 66" Sanitary Sewer And Replace With 66" Hobas (FWC Coupling Joint Type)	110	LF
12	Furnish And Install 60" Polymer Concrete Sewer Manhole With 96" Precast Base	3	EACH
13	Remove Existing Manhole 756004803 And Replace With 60" Polymer Concrete Sewer Manhole With 96" Precast Base (MH No. 4)	1	EACH
14	Furnish & Install 66" Sanitary Sewer (Hobas Flush Reline Joint) Inside 86" Steel Casing	639	LF
15	Remove Existing 48" Sanitary Sewer Prior To Steel Casing	2	EACH

16	Furnish & Install New Junction Structure B	1	EACH
17	Furnish & Install New Junction Structure C	1	EACH
18	Furnish & Install Junction Structure D (60" Polymer Concrete Manhole With 84" Base)	1	EACH
19	Furnish & Install New Junction Structure E	1	EACH
20	Install 60" Polymer Concrete Manhole Insert And 30" Diameter Composite Frame/Cover (Inserts)	8	EACH
21	Install 60" Polymer Concrete Manhole And 30" Diameter Composite Frame/Cover (Tee-Style)	1	EACH
22	Install 48" Polymer Concrete Manhole And 30" Diameter Composite Frame/Cover (Tee Style)	4	EACH
23	Existing Junction Structure No. 1 (New 60" Polymer MH Stackout & Abandonment Of South MH)	1	EACH
24	Clean Existing 30" Sanitary Sewer	938	LF
25	Clean Existing 48" Sanitary Sewer	1,997	LF
26	Clean Existing 66" Sanitary Sewer	652	LF
27	Pipe Man Entry Cleaning & Detached Liner Removal	2,669	LF
28	Install 30" CIPP Liner	938	LF
29	Install 48" CIPP Liner	2,019	LF
30	Install 66" CIPP Liner	652	LF
31	48" East Bypass Pump Setup & Teardown	1	EACH
32	48" East Bypass Pump Operation	289	DY
33	Furnish & Install 84" Polymer Concrete Sewer Manhole On CIP Base	1	EACH
34	48" West Bypass Pump Setup & Teardown	1	EACH
35	48" West Bypass Pump Operation	122	DY
36	General Conditions	15	MO

BID ITEM INCLUSIONS:

1. Mobilization/Demobilization

- a. Includes a onetime round trip for personnel, equipment, and supplies. Project delays caused by Owner that result in demobilization of personnel and/or equipment are not included, and any additional costs incurred are a change in scope.
- b. Monthly internet fees and connex box rental.
- c. Yard maintenance and clean-up crew.
- d. Water meters for construction water.
- e. 4 each project signs.
- f. Jack and bore subcontractor mobilization and demobilization. Project delays that result in demobilization of personnel and/or equipment are not included, and any additional costs incurred are a change in scope.

- g. 1 each CIPP subcontractor mobilization.
 - h. Material delivery/freight for stop logs, 66" Hobas Pipe, and Armorock manholes and inserts.
 - i. Crew costs for unloading and stringing new 48" and 66" sewerline pipe.
 - j. Crew and crane costs for offloading Armorock structures, staging material in yard, handling material, and transporting material to on-site location.
 - k. Large excavator transport costs for mobilizing, on-site equipment moves from one site location to another, and demobilizing from the site.
2. SWPPP
- a. B&F utilizes a third-party service to provide the SWPPP project book and complete the inspections and record keeping. This cost is included in this item.
 - b. BMP materials to protect drainage inlets from storm water runoff.
 - c. Street sweeping for 168 hours which will be expected at 12 hours per month during construction.
 - d. NOI and Dust Control Permit
 - e. Labor and equipment to perform setup of SWPPP and routine maintenance of the site cleaning and repairing and replacing BMPs weekly as needed.
3. Install Pipe Plug Per MAG Standard Detail 427
- a. Includes all labor, equipment, and shoring to install 8 each pipe plugs on to-be abandoned existing pipe sections called out on Sheet No. D-1.0 Note 3 (Plug Pipe Per MAG Standard Detail 427) and Note 4 (Plug Existing 48" Dia. Pipe From Within MH 7560009379 Per MAG Standard Detail 427 to Avoid Removal of ADOT Irrigation Ditch).
 - b. Includes materials for brick and mortar pipe plugs per MAG Standard Detail 427.
4. Remove Existing 48" Sanitary Sewer As Needed For Construction Of New 66" Sewer
- a. Labor, equipment, and shoring to cut and remove existing 48" sanitary sewer between existing manholes 756004803 and 756007413.
 - b. Trucking costs for hauling off and disposing of spoils and existing pipe to be removed.
 - c. Subcontractor costs for cutting pipe ends prior to removing pipe.
5. Grout Fill Existing 48" Sanitary Sewer
- a. Scope includes grout filling the existing 48-inch sewerline to-be abandoned, which is called out on Sheet D-1.0
 - b. Labor, equipment, and shoring to assemble pipe plugs with grout connection and connect grout pump.
 - c. Grout pump and labor to fill each pipe to be abandoned with grout.
 - d. Grout or CLSM material to fill existing pipes with.
6. Abandon Existing 108" Dia. Manhole 756009379
- a. Labor and equipment to abandon existing Manhole 756009379 in place.
 - b. Material and crew costs for filling manhole with ½ sack ABC slurry in lifts.
7. Abandon Existing Junction Structure No. 2
- a. Labor and equipment to abandon existing junction structure no. 2 by removing the manhole lids, concrete rings, and riser and grout filling.
 - b. Includes trucking and disposals costs for removing lids, concrete rings, and riser.
 - c. ½ Sack ABC slurry for filling existing junction structure no. 2.
8. Furnish & Install 48" Sanitary Sewer (PVC SDR-26)

- a. Labor, equipment, and shoring for excavating, installing, and backfilling new 48" sewerline per Project Drawings.
 - b. Trucking and dump fees for haul-off and disposal of spoils.
 - c. Sewerline testing.
 - d. Compacted MAG Spec ABC for pipe bedding.
 - e. ½ Sack ABC Slurry backfill from top of bedding to 12" above top of pipe.
 - f. MAG spec ABC backfill from 12" above top of pipe to 1 foot below finished grade.
 - g. Material purchase for 48" PVC SDR-26 sewerline pipe.
9. Remove Existing Plug And Connect To Existing 66" Stub Out
 - a. Labor, equipment, and shoring to excavate, expose, and remove pipe plug from existing 66" sewer stub-out.
 - b. Trucking and dump fees for spoil materials.
 - c. Hydro excavation crew time for soft digging around existing sewer stub-out.
10. Furnish & Install 66" Sanitary Sewer (Hobas FWC Coupling Joint)
 - a. Labor, equipment, and shoring for excavating, installing, and backfilling new 66" Hobas sewerline per Project Drawings.
 - b. Trucking and dump fees for haul-off and disposal of spoils.
 - c. Sewerline testing.
 - d. MAG Spec ABC for pipe bedding
 - e. ½ Sack ABC Slurry backfill from top of bedding to 12" above top of pipe.
 - f. MAG spec ABC backfill from 12" above top of pipe to 1 foot below finished grade.
 - g. Material purchase for 66" Hobas sewerline pipe.
11. Remove Existing 66" Sanitary Sewer and Replace With 66" HObas (FWC Coupling Joint)
 - a. Labor, equipment, haul-off, dump fees, and shoring for removing existing existing 66" RCP sewer between new Price Junction Structure on existing alignment, excavating, installing, and backfilling new 66" Hobas sewerline per Project Drawings.
 - b. Trucking and dump fees for haul-off and disposal of spoils.
 - c. Sewerline testing.
 - d. MAG Spec ABC for pipe bedding
 - e. ½ Sack ABC Slurry backfill from top of bedding to 12" above top of pipe.
 - f. MAG spec ABC backfill from 12" above top of pipe to 1 foot below finished grade.
 - g. Material purchase for 66" Hobas sewerline pipe.
12. Furnish & Install 60" Polymer Concrete Sewer Manhole With 96" Precast Base (New 66" Sewer Line)
 - a. Includes manhole number 1, 2, and 3.
 - b. Labor, equipment, and shoring for excavating, installing, and backfilling 2 each new 60" polymer concrete manholes with 96" precast bases per Project Drawings.
 - c. Trucking and dump fees for haul-off and disposal of spoils.
 - d. Vacuum testing structure after backfill.
 - e. Final adjustments and concrete collars.
 - f. ½ Sack ABC Slurry backfill.
 - g. Purchase of precast manhole material from Armorock.
 - h. Crane to set manholes.
 - i. Owner supplies manhole frame and covers.
13. Remove Existing Manhole 756004803 And Replace With 60" Polymer Concrete Sewer Manhole With 96" Precast Base

- a. Includes costs for removing existing manhole 756004803 and installing new manhole no. 4 per Project Drawings.
 - b. Labor, equipment, and shoring for excavating, installing, and backfilling 1 each new 60" polymer concrete manholes with 96" precast base per Project Drawings.
 - c. Trucking and dump fees for haul-off and disposal of spoils.
 - d. Sawcut and removal of existing pipe.
 - e. Vacuum testing structure after backfill.
 - f. Final adjustments and concrete collars.
 - g. ½ Sack ABC Slurry backfill.
 - h. Purchase of precast manhole material from Armorock.
 - i. 48" PVC SDR-26 sewerline pipe material for stub-out and 1 each pipe collar to connect to existing 48" on west side of new manhole. No end treatment of cut pipe included since pipe collar will be CIPP lined through.
 - j. Crane to set manhole.
 - k. Owner supplies manhole frame and cover.
14. Furnish & Install 66" Sanitary Sewer (Hobas Flush Reline Joint) Inside 86" Steel Casing
- a. Labor and equipment to excavate jack and bore pits, set shoring, remove shoring, and back fill pits.
 - b. Trucking and dump fees for hauling off jack and bore pits spoils and casing installation spoils.
 - c. MAG specs for ABC backfill jack and bore pits.
 - d. Jack and bore subcontractor costs for furnishing, installing, and welding 86" O.D. steel casing (84" I.D.), installing 66" Hobas pipe through steel casing, grouting void outside of casing, and filling annular space with low density cellular grout.
 - e. Crew costs for loading trucks to haul off spoils from casing installation.
 - f. Trench shore protection rental.
 - g. Temporary fence for securing jack and bore pits.
 - h. Purchase of 66" Hobas Pipe to be pushed through casing.
15. Remove Existing 48" Sanitary Sewer Prior To Steel Casing
- a. Labor, equipment, and shoring for excavating and exposing existing 48" sewerlines to be removed for steel casing installation at stations 107+96 and 108+65.
 - b. Sawcutting and removing pipe
 - c. Trucking and dump fees for haul-off of spoils and removed sewer pipe.
 - d. ½ Sack ABC Slurry backfill off 2 each excavations.
16. Furnish & Install New Junction Structure B
- a. Labor and equipment to excavate and set slide rail for cast-in-place structure installation and set and prep subgrade for structure.
 - b. Crane for setting and removing slide rail.
 - c. MAG Spec ABC for structure subgrade/foundation installation and preparation.
 - d. Trucking and dump fees for spoil haul-off.
 - e. ½ Sack ABC Slurry backfill around structure.
 - f. Slide rail shoring system rental and Engineering.
 - g. Furnishing and installing PVC T-Lock per Project Drawings.
 - h. Cast-in-place Structure Subcontractor costs for reinforcing, forming, and pouring structure.
 - i. Material purchase costs for Plastifab stop logs, davit crane, access hatch, and grating. Note material deviation, per Plastifab, guide frames are surface mounted

with unseating head, so they will need to be stainless steel in lieu of the specified vinyl ester FRP.

- j. Owner supplies manhole frames and covers.
17. Furnish & Install New Junction Structure C
- a. Labor and equipment to excavate, set slide rail, install, and backfill 192" round precast polymer structure installation.
 - b. Hydro excavation around corners for slide rail installation.
 - c. Crane for setting/removing slide rail and setting structure.
 - d. MAG Spec ABC for structure subgrade.
 - e. Trucking and dump fees for spoil haul-off.
 - f. ½ Sack ABC Slurry backfill around structure.
 - g. Slide rail shoring system rental and Engineering.
 - h. Vacuum testing of new structure.
 - i. Purchase of 192" Armorock polymer precast structure, hatches, FRP grating, stop logs, and davit crane.
 - j. Note material deviation, per Plastifab, guide frames are surface mounted with unseating head, so they will need to be stainless steel in lieu of the specified vinyl ester FRP.
 - k. Labor and equipment for installing FRP grating.
 - l. Owner supplies manhole frames and covers.
18. Furnish & Install Junction Structure D (60" Polymer Concrete Manhole With 84" Base)
- a. Labor, equipment, and shoring for excavating, installing, and backfilling 1 each new 60" polymer concrete manhole with an 84" precast base per Project Drawings.
 - b. Trucking and dump fees for haul-off and disposal of spoils.
 - c. Sawcut and removal of existing pipe.
 - d. Vacuum testing structure after backfill.
 - e. Final adjustments and concrete collars.
 - f. ½ Sack ABC Slurry backfill.
 - g. Purchase of precast manhole material from Armorock.
 - h. 48" PVC SDR-26 sewerline pipe material for stub-out and 1 each pipe collar to connect to existing 48" sewerline on east side of new structure.
 - i. FRP end treatment on existing pipe end to be connected to and collar poured around.
 - j. Crane to set manhole.
 - k. Owner supplies manhole frame and cover.
19. Furnish & Install New Junction Structure E
- a. Labor and equipment to excavate, demo and remove existing MH 756007413, set slide rail, install new junction structure, and backfill around 192" round precast polymer structure installation.
 - b. Hydro excavation around corners for slide rail installation.
 - c. Crane for setting/removing slide rail and setting structure.
 - d. Sawcutting and removing existing pipe.
 - e. MAG Spec ABC for structure subgrade.
 - f. Trucking and dump fees for spoil haul-off.
 - g. ½ Sack ABC Slurry backfill around structure.
 - h. Slide rail shoring system rental and Engineering.
 - i. Vacuum testing of new structure.
 - j. Purchase of 192" Armorock polymer precast structure, hatches, and FRP grating.

- k. Labor and equipment for installing FRP grating.
 - l. Owner supplies manhole frames and covers.
20. Install 60" Polymer Concrete Manhole Insert and 32" Diameter Smart Composite Cover
- a. Labor and equipment to excavate around existing manhole, remove and haul off existing manhole cones, and install 60" polymer concrete manhole inserts.
 - b. Trucking and dump fees for cone and spoil disposal.
 - c. Purchase 60" Armorock polymer manhole inserts.
 - d. Preparation of bench to accept composite insert.
 - e. Bench rehabilitation includes rebuilding bench with C-120, installing FRP, and overlapping FRP onto polymer concrete manhole insert.
 - f. Final adjustments and concrete collars.
 - g. Hydrophilic grout between annular space between existing manhole barrel sections and new polymer concrete manhole insert.
 - h. Slurry backfill around manhole.
 - i. 1 each location, specifically MH 756005216 along Price Road, for concrete flatwork replacement.
 - j. 1 each location, specifically MH 756005162 in Pecos Diversion structure facility, for asphalt replacement.
 - k. Owner supplies manhole frames and covers.
21. Install 60" Polymer Concrete Manhole and 30" Diameter Composite Frame/Cover (Tee-Style)
- a. Labor and equipment to excavate and remove existing manhole to tee-style base, prep tee-style base for wet set of new bottom polymer section, install new 60" polymer risers and cone/flat top, and slurry backfill with ½ Sack ABC Slurry.
 - b. Trucking and dump fees for manhole removal and excavation.
 - c. Purchase 60" Armorock polymer manhole materials.
 - d. Installing FRP to overlap CIPP lining and new polymer manhole riser bottom section.
 - e. Final adjustments and concrete collars.
 - f. Slurry backfill around manhole.
 - g. Owner supplies manhole frames and covers.
22. Install 48" Polymer Concrete Manhole and 30" Diameter Composite Frame/Cover (Tee-Style)
- a. Labor and equipment to excavate and remove existing manhole to tee-style base, prep tee-style base for wet set of new bottom polymer sections, install new 48" polymer risers and cone/flat top, and slurry backfill with ½ Sack ABC Slurry.
 - b. Trucking and dump fees for manhole removal and excavation.
 - c. Purchase 48" Armorock polymer manhole materials.
 - d. Installing FRP to overlap CIPP lining and new polymer manhole riser bottom section.
 - e. Final adjustments and concrete collars.
 - f. Slurry backfill around manhole.
 - g. Owner supplies manhole frames and covers.
23. Existing Junction Structure No. 1 (New 60" Polymer MH Stackout & Abandonment of South MH)
- a. Labor and equipment to excavate and remove existing manhole riser sections on existing Junction Structure No. 1, prep tee-style base for wet set of new bottom

- polymer section, install new 60" polymer riser and cone/flat top, and slurry backfill with ½ Sack ABC Slurry.
- b. FRP will be installed to overlap CIPP to-be lined through structure and overlap onto new polymer bottom riser section per Project Drawings.
 - c. South manhole will be completely removed, CIPP lined through, and backfilled.
 - d. Trucking and dump fees for manhole removal and excavation.
 - e. Purchase 60" Armorock polymer manhole materials.
 - f. Final adjustment and concrete collar.
 - g. Slurry backfill structure.
 - h. Owner supplies manhole frames and covers.
24. Clean Existing 30" Sanitary Sewer
- a. Sewer cleaning vactor crew and CCTV crew for performing 3-pass cleaning of existing 30" sewerline.
25. Clean Existing 48" Sanitary Sewer
- a. Sewer cleaning vactor crew and CCTV crew for performing 3-pass cleaning of existing 48" sewerline.
26. Clean Existing 66" Sanitary Sewer
- a. Sewer cleaning vactor crew and CCTV crew for performing 3-pass cleaning of existing 66" sewerline.
27. Man Entry Cleaning & Detached Liner Removal
- a. Labor and confined space equipment to complete entry to remove any detached T-Lock from pipe.
 - b. Any grouting to be compensated through Construction Contingency.
28. Install 30" CIPP Liner
- a. Furnish and install 30" CIPP Liner per project documents.
 - b. CIPP lining support crew.
 - c. Post-Pano sewer CCTV inspection.
 - d. Modify manhole bench for CIPP liner installation.
29. Install 48" CIPP Liner
- a. Furnish and install 48" CIPP Liner per project documents.
 - b. Lining water usage to be compensated through allowance item.
 - c. CIPP lining support crew.
 - d. Post-Pano sewer CCTV inspection.
30. Install 66" CIPP Liner
- a. Furnish and install 66" CIPP Liner per project documents.
 - b. CIPP lining support crew.
 - c. Lining water usage to to be compensated through allowance item.
 - d. If required, chilled water setups and/or styrene mitigation shall be compensated through Construction Contingency.
 - e. Post-Pano sewer CCTV inspection.
31. 48" East Bypass Pump Setup & Teardown
- a. Complete bypass setup costs for 48" bypass from suction pit to discharge into existing manhole 756009379.
 - b. Excavation of suction pit, cutting coupon, installing temporary CMP tank for suction pit, installing concrete in suction pit, restoring coupon, removing CMP tank, backfilling, and restoring.
 - c. Labor, equipment, and transport costs for mobilizing and demobilizing bypass pumps, pipe, and equipment.

- d. Bypass pump station setup with 3 each 12" pumps.
 - e. Trenching and burying bypass lines to maintain SRP, ADOT, and construction access.
 - f. Fuse and install 1 each 18" discharge lines with each line being approximately 700 linear feet and pass hydrostatic test HDPE discharge line.
 - g. Bypass pump station and pipe removal.
 - h. Bypass discharge location prep and setup for discharge line installation.
 - i. Bypass Engineering Stamp.
32. 48" East Bypass Pump Operation
- a. All equipment rental for 289 calendar days.
 - b. Fuel for pumps for 289 calendar days based on \$4.00 per gallon. Fuel escalation will be based on the EIA.gov Rock Mountain market rate and compensation will be paid through Contractor's Contingency based of area average fuel rate index, if market increases above \$4.00 per gallon for over a 30-day period.
 - c. Pump operators and bypass monitors for 24 hours per day. 1 operator/monitor during day-time work hours and 2 each operators/monitors for after hours.
 - d. Includes disposal of rags that are removed from pumps.
 - e. Includes maintenance of equipment.
 - f. Includes secondary containment for all equipment.
 - g. All valves, manifolds, pipe, fittings, fencing/barriers, and equipment rental.
 - h. Includes light tower.
33. Furnish & Install 84" Polymer Concrete Sewer Manhole on Cast-In-Place Base
- a. Pricing is based on preliminary and conceptual discussions only. Pricing includes installing a new 84" manhole on a cast-in-place base north of the Pecos Diversion Structure, which will also be utilized as the suction manhole for the west bypass.
 - b. Includes labor, equipment, and shoring to excavate down to existing pipe, cut coupon, and pour cast-in-place base around existing 48" sewerline.
 - c. Concrete cast-in-place structure crew to perform reinforcing, forming, and pouring of manhole bases per detail provided in 90% Design Review Comments.
 - d. Includes concrete material for cast-in-place bases.
 - e. FRP bench treatment for cast-in-place base.
 - f. ½ Sack ABC Slurry backfill around structures.
 - g. Hydro excavation around existing pipe
 - h. Final manhole adjustments and concrete collars.
 - i. Owner supplies manhole frames and covers.
34. 48" West Bypass Pump Setup & Teardown
- a. Complete bypass setup costs for 48" bypass from new manhole north of Pecos Diversion Structure to discharge into existing manhole 756009379.
 - b. Labor, equipment, and transport costs for mobilizing and demobilizing bypass pumps, pipe, and equipment.
 - c. Bypass pump station setup with 3 each 12" pumps.
 - d. Trenching and burying bypass lines to maintain SRP, ADOT, and construction access.
 - e. Fuse and install 1 each 18" discharge lines with each line being approximately 2,550 linear feet and pass hydrostatic test HDPE discharge line.
 - f. Bypass pump station and pipe removal.
 - g. Bypass discharge location prep and setup for discharge line installation.
 - h. Bypass Engineering Stamp.

35. 48" West Bypass Pump Operation

- a. All equipment rental for 122 calendar days.
- b. Fuel for pumps for 122 calendar days based on \$4.00 per gallon. Fuel escalation will be based on the EIA.gov Rock Mountain market rate and compensation will be paid through Contractor's Contingency based on area average fuel rate index, if market increases above \$4.00 per gallon for over a 30-day period.
- c. Pump operators and bypass monitors for 24 hours per day. 1 operator/monitor during day-time work hours and 2 each operators/monitors for after hours.
- d. Includes disposal of rags that are removed from pumps.
- e. Includes maintenance of equipment.
- f. Includes secondary containment for all equipment.
- g. All valves, manifolds, pipe, fittings, fencing/barriers, and equipment rental.
- h. Includes light tower.

ALLOWANCES

100. Traffic Control (Allowance)

- a. This will be all costs associated with Traffic Control including but not limited to traffic control plans, permits, devices, labor, equipment, temporary pedestrian relocation, light bagging, etc.
- b. Visual Message Boards

101. Police Officer (Allowance)

- a. This will cover all cost associated with police officers' presence on the site during work near a lighted intersection.
- b. The intent will always be to offer the work to Chandler PD prior to allowing other officers to work on the project.

102. Quality Control Testing (Allowance)

- a. All costs associated with testing on the project including but not limited to compaction, concrete cylinder, sieve analysis, proctor, etc.

103. Construction Water (Allowance)

- a. Cost for water on the project for all activating including but not limited to dust mitigation, testing, CIPP, pipe cleaning, etc.
- b. This will be reimbursed with the bills backing up the charges with all associated markups.

104. Public Outreach (Allowance)

- a. Cost for all required public outreach efforts including but not limited to public meetings, websites, hotline management, flyers, door hanger, labor for distribution, email correspondence, data collection, etc.

105. T-Lock Grouting (Allowance)

- a. The grouting behind deteriorated concrete inside pipe during inspections and cleaning efforts.

106. Heavy Cleaning (Allowance)

- a. Cleaning of heavy debris inside pipe with man entry and extraction to waste.

107. Landscape Restoration (Allowance)

- a. Landscape restoration due to work in landscape areas.

108. SRP Observer (Allowance)

- a. Allowance to reimburse any costs associated with SRP observers required for construction activities near SRP easement or powerlines.
- 109. Survey (Allowance)
 - a. Allowance to capture all GIS survey data for the city.
- 110. Odor Control (Allowance)
 - a. Allowance to capture all costs associated with additional Odor Control required beyond tarping bypass suction and discharge locations.
- 111. Pecos Diversion Structure Rehab & Sluice Gates (Allowance)
 - a. Allowance shall be utilized to reimburse costs for stripping and coating existing Pecos Diversion Structure, removing and replacing 2 each sluice gates, costs for sluice gates and hardware, motor operated sluice gate valves, stems, anchors, electrical connections for motor operated sluice gate valves, etc.

CONTINGENCY

- 500. Contractor's Contingency
 - a. Proposed Contractor's Contingency amount is based on risk matrix included in this GMP.
- 501. Owner Contingency
 - i. The Owner's Contingency is based on 5% of the contract value.

EXHIBIT D

CM@RISK GMP SUMMARY (PAGE 2 of 2)

Project Name: 101/202 66-inch Redundant Sewer GMP
 Project Location: Price Road and 202
 City of Chandler Project No.: WW2402.401

Date: 8/12/2025

				Amount
A. Cost of Work				
A1 Cost of Work (self perform, no mark-up)				\$10,879,284.64
A2 Cost of Work (subcontractors, suppliers, material)				\$13,052,205.73
TOTAL COST OF WORK:				\$23,931,490.37
B. General Conditions				
				\$1,357,246.35
SUBTOTAL 1 (Cost of Work) (A + B):				\$25,288,736.72
C. CM@Risk's Fee				
				\$2,149,542.62
SUBTOTAL 2 (A + B + C):				\$27,438,279.34
D. Bonds and Insurance (on Subtotal 1)				
D1 Bonds (Payment and Performance) @ 1.0%				\$252,887.37
D2 Insurance @ 1.1%				\$278,176.10
D3 Builder's Risk Insurance				\$84,000.00
TOTAL BONDS & INSURANCE:				\$615,063.47
SUBTOTAL 3 (SUBTOTAL 2 + BONDS & INS):				\$28,053,342.81
E. Sales Tax				
E1 Sales Tax (7.8% x 0.65)				\$1,422,304.48
E2 Tax Credits				\$0.00
TOTAL SALES TAX:				\$1,422,304.48
F. Approved Allowances				
	QTY	UOM	Unit Price	Allowance Total
100 Traffic Control	1	AL	\$ 46,000.00	\$46,000.00
101 Police Officer	1	AL	\$ 14,490.00	\$14,490.00
102 Quality Control Testing	1	AL	\$ 86,250.00	\$86,250.00
103 Construction Water	1	AL	\$ 57,500.00	\$57,500.00
104 Public Outreach	1	AL	\$ 15,000.00	\$15,000.00
105 T-Lock Grouting Allowance	40	HR	\$ 704.90	\$28,196.08
106 Heavy Cleaning Allowance	1	AL	\$ 66,125.00	\$66,125.00
107 Landscape Restoration	1	AL	\$ 15,000.00	\$15,000.00
108 SRP Observer	1	AL	\$ 52,900.00	\$52,900.00
109 Survey	1	AL	\$ 54,050.00	\$54,050.00
110 Odor Control	1	AL	\$ 197,000.00	\$197,000.00
111 Pecos Diversion Structure Rehab & Sluice Gates	1	AL	\$ 466,900.00	\$466,900.00
TOTAL ALLOWANCES:				\$1,099,411.08
G. Contingencies				
G1 Contractors Contingency				\$2,545,652.66
G2 Owner Contingency				\$1,528,752.92
TOTAL CONTINGENCIES:				\$4,074,405.57
GUARANTEED MAXIMUM PRICE TOTAL:				\$34,649,463.94

Establishment of Values:

- Cost of Work (A) and Allowances (F) to be submitted with GMP Proposal.
- General Conditions Cost (B) to be established in Baseline GMP.
- CM@Risk Fee as set forth in Section 3.7.3 of this Contract

CITY OF CHANDLER
WW2402.401 LOOP 101 AND 202 FREEWAY 66-INCH REDUNDANT SEWER PROJECT
GMP SCHEDULE OF VALUES

A	B	C	D	E	F
ITEM NO.	DESCRIPTION OF WORK	QTY	UNIT OF MEASURE	UNIT PRICE	SCHEDULED VALUE
1	Mobilization And Demobilization	1	LS	\$ 1,070,395.41	\$ 1,070,395.41
2	SWPPP	15	MO	\$ 6,621.58	\$ 99,323.70
3	Install Pipe Plug Per MAG Standard Detail 427	8	EACH	\$ 10,050.84	\$ 80,406.72
4	Pipe	63	LF	\$ 1,039.65	\$ 65,497.95
5	Grout Fill Existing 48" Sanitary Sewer To Extents Shown	387	LF	\$ 212.93	\$ 82,403.91
6	Abandon Existing 108" Dia Manhole 756009379	1	EACH	\$ 27,020.66	\$ 27,020.66
7	Abandon Existing Junction Structure No. 2	1	EACH	\$ 65,995.34	\$ 65,995.34
8	Furnish And Install 48" Sanitary Sewer (PVC SDR-26)	203	LF	\$ 1,377.18	\$ 279,567.54
9	Remove Existing Plug And Connect To Existing 66" Stub Out	1	EACH	\$ 15,331.12	\$ 15,331.12
10	Furnish & Install 66" Sanitary Sewer (Hobas FWC Coupling Joint)	776	LF	\$ 2,061.63	\$ 1,599,824.88
11	Type)	110	LF	\$ 4,489.87	\$ 493,885.70
12	Furnish And Install 60" Polymer Concrete Sewer Manhole With 96" Precast Base	3	EACH	\$ 189,772.58	\$ 569,317.74
13	Manhole With 96" Precast Base (MH No. 4)	1	EACH	\$ 348,571.50	\$ 348,571.50
14	Furnish & Install 66" Sanitary Sewer (Hobas Flush Reline Joint) Inside 86" Steel Casing	639	LF	\$ 5,973.99	\$ 3,817,379.61
15	Remove Existing 48" Sanitary Sewer Prior To Steel Casing	2	EACH	\$ 83,716.80	\$ 167,433.60
16	Furnish & Install New Junction Structure B	1	EACH	\$ 3,098,028.38	\$ 3,098,028.38
17	Furnish & Install New Junction Structure C	1	EACH	\$ 1,263,349.01	\$ 1,263,349.01
18	Furnish & Install Junction Structure D (60" Polymer Concrete Manhole With 84" Base)	1	EACH	\$ 313,292.32	\$ 313,292.32
19	Furnish & Install New Junction Structure E	1	EACH	\$ 1,176,438.77	\$ 1,176,438.77
20	(Inserts)	8	EACH	\$ 69,859.71	\$ 558,877.68
21	Style)	1	EACH	\$ 200,635.08	\$ 200,635.08
22	Style)	4	EACH	\$ 132,459.19	\$ 529,836.76
23	South MH)	1	EACH	\$ 95,414.07	\$ 95,414.07
24	Clean Existing 30" Sanitary Sewer	938	LF	\$ 10.28	\$ 9,642.64
25	Clean Existing 48" Sanitary Sewer	1,997	LF	\$ 14.49	\$ 28,936.53
26	Clean Existing 66" Sanitary Sewer	652	LF	\$ 18.82	\$ 12,270.64
27	Pipe Man Entry Cleaning & Detached Liner Removal	2,669	LF	\$ 7.74	\$ 20,658.06
28	Install 30" CIPP Liner	938	LF	\$ 292.97	\$ 274,805.86
29	Install 48" CIPP Liner	2,019	LF	\$ 427.40	\$ 862,920.60
30	Install 66" CIPP Liner	652	LF	\$ 1,036.46	\$ 675,771.92
31	48" East Bypass Pump Setup & Teardown	1	EACH	\$ 274,833.54	\$ 274,833.54
32	48" East Bypass Pump Operation	289	DY	\$ 10,884.80	\$ 3,145,707.20
33	Furnish & Install 84" Polymer Concrete Sewer Manhole On CIP Base	1	EACH	\$ 278,133.06	\$ 278,133.06
34	48" West Bypass Pump Setup & Teardown	1	EACH	\$ 238,901.81	\$ 238,901.81
35	48" West Bypass Pump Operation	122	DY	\$ 17,136.73	\$ 2,090,681.06
					\$ 23,931,490.37
	General Conditions				
36	General Conditions	15	MO	\$ 90,483.09	\$ 1,357,246.35
	Subtotal with General Conditions				\$ 25,288,736.72
	8.5% Markup				\$ 2,149,542.62
	Total with OH&P				\$ 27,438,279.34
	Insurance and Bond on Subtotal of CMAR Direct Work + Overhead & Profit				
	Bond Cost (1%)				\$ 252,887.37
	Insurance Cost (1.1%)				\$ 278,176.10
	Builder's Risk Insurance				\$ 84,000.00
	Subtotal Bond and Insurance				\$ 615,063.47
	Subtotal Bond and Insurance+ CMAR Direct Work + Overhead & Profit				\$ 28,053,342.81
	Sales Tax				
	Sales Tax				\$ 1,422,304.48
	Combined Total with Insurance, Bonds, & Tax				\$ 29,475,647.29

CITY OF CHANDLER
WW2402.401 LOOP 101 AND 202 FREEWAY 66-INCH REDUNDANT SEWER PROJECT
GMP SCHEDULE OF VALUES

A	B	C	D	E	F
ITEM NO.	DESCRIPTION OF WORK	QTY	UNIT OF MEASURE	UNIT PRICE	SCHEDULED VALUE
	ALLOWANCES				
100	Traffic Control	1	AL	\$ 46,000.00	\$ 46,000.00
101	Police Officer	1	AL	\$ 14,490.00	\$ 14,490.00
102	Quality Control Testing	1	AL	\$ 86,250.00	\$ 86,250.00
103	Construction Water	1	AL	\$ 57,500.00	\$ 57,500.00
104	Public Outreach	1	AL	\$ 15,000.00	\$ 15,000.00
105	T-Lock Grouting Allowance	40	HR	\$ 704.90	\$ 28,196.08
106	Heavy Cleaning Allowance	1	AL	\$ 66,125.00	\$ 66,125.00
107	Landscape Restoration	1	AL	\$ 15,000.00	\$ 15,000.00
108	SRP Observer	1	AL	\$ 52,900.00	\$ 52,900.00
109	Survey	1	AL	\$ 54,050.00	\$ 54,050.00
110	Odor Control	1	AL	\$ 197,000.00	\$ 197,000.00
111	Pecos Diversion Structure Rehab & Sluice Gates	1	AL	\$ 466,900.00	\$ 466,900.00
	Subtotal with Allowances				\$ 1,099,411.08
500	Contractors Contingency	1	LS	\$ 2,545,652.66	\$ 2,545,652.66
501	Owner's Contingency	1	LS	\$ 1,528,752.92	\$ 1,528,752.92
	Subtotal of Contingency				\$ 4,074,405.57
	ORIGINAL CONTRACT TOTALS				\$ 34,649,463.94



CONTRACTING
Since 1990

Proposal

Corporate:
11011 N. 23rd Ave. Phoenix, AZ 85029
Office: 623-582-1170 | Fax: 623-582-3761

Tucson:
1904 W Prince Rd. Tucson, 85705
Office: 520-207-8228 | Fax: 520-305-3323

AZ Licenses:
ROC089744 A | General Engineering
ROC111282 B-04 | General Engineering Contractor

Cost Detail With Man Hours

Project Name:	101/202 66 Inch Reduntant Sewer 90% GMP Rev1	Customer:	City of Chandler
Job Number:	24-4018-00	Bid Number:	24-4018-00
Bid As:	General Contractor	Billing Address:	Mail Stop 407, P.O. Box 4008 Chandler, AZ 85244
Estimator:	Travis Monk	Phone:	480-782-3310
Project Address:	Price Road And 202	Contact:	Raymond Potts
Completion Date:	4/10/2025		

Pay Items

Description	Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
D 1 - Mobilization And Demobilization	1.00	LS	1,146.00	1,146.00	\$1,070,395.41	\$1,070,395.41
D Mobilize/ Demobilize Equipment	1.00	LS	336.00	336.00	\$66,432.96	\$66,432.96
 B&F Equipment Mobilization (1.00 EACH/DY, 42.00 DY)	42.00	EACH	336.00	8.00	\$1,388.88	\$58,332.96
 Transport Lowboy	336.00	HR			\$33.43	\$11,232.48
 Transport Truck Semi	336.00	HR			\$83.31	\$27,992.16
 Transport Truck Driver	336.00	HR			\$56.87	\$19,108.32
S Shoring Deliveries/Pick-ups/Moves (Slide Rail Delivery Is Under Separate Item)	36.00	EACH			\$225.00	\$8,100.00
D Office Startup / Establishment	1.00	EACH	0.00	0.00	\$12,750.00	\$12,750.00
P Internet	15.00	MO			\$300.00	\$4,500.00
P Connex	15.00	MO			\$550.00	\$8,250.00
D Yard Crew	15.00	MO	330.00	22.00	\$3,515.60	\$52,734.00
 Yard Clean-up & Maintenance (2.75 DY/MO, 41.25 DY)	15.00	MO	330.00	22.00	\$3,515.60	\$52,734.00
 Cat 950	330.00	HR			\$104.46	\$34,471.80
 Operator: Small Equipment	330.00	HR			\$55.34	\$18,262.20
P Water Meters	2.00	EACH			\$2,400.00	\$4,800.00
D Project Signs	4.00	EACH	32.00	8.00	\$1,527.36	\$6,109.44
P Project Signs	4.00	EACH			\$750.00	\$3,000.00
 Sign Installation Crew (3.00 EACH/DY, 1.33 DY)	4.00	EACH	32.00	8.00	\$777.36	\$3,109.44
 F-250 to F-450	10.67	HR			\$39.86	\$425.17
 Foreman	10.67	HR			\$77.15	\$822.93
 Laborer	10.67	HR			\$44.30	\$472.53
 Cat 420 Small Backhoe	10.67	HR			\$74.86	\$798.51
 Operator: Small Equipment	10.67	HR			\$55.34	\$590.29
D Unload & String Pipe	20.00	DY	240.00	12.00	\$1,395.36	\$27,907.20
 Unload And String Pipe (2.00 DY/DY, 10.00 DY)	20.00	DY	240.00	12.00	\$1,395.36	\$27,907.20
 F-250 to F-450	80.00	HR			\$39.86	\$3,188.80

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
(Item 1 - Mobilization And Demobilization continued)							
 Foreman		80.00	HR			\$77.15	\$6,172.00
 Laborer		80.00	HR			\$44.30	\$3,544.00
 Operator: Large Equipment		80.00	HR			\$60.68	\$4,854.40
 Cat TL1255 (12K Telehandler Forklift)		80.00	HR			\$126.85	\$10,148.00
 Large Excavator(s) Mobilization/Site Moves/ Demobilization		12.00	EACH			\$4,218.00	\$50,616.00
 Structures, Manholes, Inserts, & Pipe Material Handling At Yard And Deliver To Site		12.00	EACH	128.00	10.67	\$7,338.49	\$88,061.92
 B&F Equipment Mobilization (3.00 EACH/DY, 4.00 DY)		12.00	EACH	32.00	2.67	\$462.96	\$5,555.52
 Transport Lowboy		32.00	HR			\$33.43	\$1,069.76
 Transport Truck Semi		32.00	HR			\$83.31	\$2,665.92
 Transport Truck Driver		32.00	HR			\$56.87	\$1,819.84
 Support & Spotter Crew (3.00 EACH/DY, 4.00 DY)		12.00	EACH	96.00	8.00	\$870.53	\$10,446.40
 Cat 950		32.00	HR			\$104.46	\$3,342.72
 Laborer		32.00	HR			\$44.30	\$1,417.60
 Operator: Large Equipment		32.00	HR			\$60.68	\$1,941.76
 Foreman		32.00	HR			\$77.15	\$2,468.80
 F-250 to F-450		32.00	HR			\$39.86	\$1,275.52
 Crane For Offloading Armorock Manhole Bases		3.00	EACH			\$9,500.00	\$28,500.00
 Crane For Offloading Armorock Structures		2.00	EACH			\$21,780.00	\$43,560.00
 Potholing		15.00	EACH	80.00	5.33	\$894.83	\$13,422.40
 GC-Combo Hydro Truck Crew (3.00 EACH/DY, 5.00 DY)		15.00	EACH	80.00	5.33	\$894.83	\$13,422.40
 Combo Vactor Truck		40.00	HR			\$225.00	\$9,000.00
 Combo Truck Assistant		40.00	HR			\$48.46	\$1,938.40
 Combo Truck Operator		40.00	HR			\$62.10	\$2,484.00
 Material Delivery		1.00	LS	0.00	0.00	\$445,790.65	\$445,790.65
 Stop Logs		1.00	LS			\$15,525.00	\$15,525.00
 66" Hobas Pipe Delivery		1.00	LS			\$117,600.00	\$117,600.00
 Armorock Polymer Structure & Insert Deliveries		1.00	EACH			\$309,200.00	\$309,200.00
 Access Hatch Delivery Fees		1.00	LS			\$3,465.65	\$3,465.65
 Subcontractor Mobilizations		1.00	LS	0.00	0.00	\$296,770.84	\$296,770.84
 CIPP Mob (3 Each - 30", 48", And 66") W/ 1.5% Bond (Insituform Technologies LLC)		1.00	LS			\$38,680.64	\$38,680.64
 Jack & Bore Subcontractor Mobilization (W/ 1.5% Bond)		1.00	EACH			\$203,000.00	\$203,000.00
 CIP Structure B Subcontractor Mobilization (w/ 2% Bond) (CS Construction Inc)		1.00	LS			\$55,090.20	\$55,090.20
 General Site Shoring Engineered Fees		2.00	EACH			\$2,500.00	\$5,000.00
 2 - SWPPP		15.00	MO	432.00	28.80	\$6,621.58	\$99,323.68
 SWPPP Permit And Book		1.00	LS			\$6,500.00	\$6,500.00
 Inspections		44.00	EACH			\$200.00	\$8,800.00
 BMP		1.00	LS			\$10,500.00	\$10,500.00
 SWPPP Manitenance		15.00	MO	360.00	24.00	\$2,427.76	\$36,416.40
 SWPPP Maintenance Crew (1.00 DY/MO, 15.00 DY)		15.00	MO	360.00	24.00	\$2,427.76	\$36,416.40
 F-250 to F-450		120.00	HR			\$39.86	\$4,783.20
 John Deere 210 Gannon / Skip Loader		120.00	HR			\$86.82	\$10,418.40

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 2 - SWPPP continued)</i>							
	Foreman	120.00	HR			\$77.15	\$9,258.00
	Laborer	120.00	HR			\$44.30	\$5,316.00
	Operator: Small Equipment	120.00	HR			\$55.34	\$6,640.80
	Street Sweeping	168.00	HR			\$140.00	\$23,520.00
	Dust Permit	2.00	EACH			\$1,130.00	\$2,260.00
	On-site Trackout Rock	120.00	TON	72.00	0.60	\$94.39	\$11,327.28
	Trackout Rock	120.00	TON			\$33.70	\$4,044.00
	Setup & Remove Trackout (40.00 TON/DY, 3.00 DY)	120.00	TON	72.00	0.60	\$60.69	\$7,283.28
	F-250 to F-450	24.00	HR			\$39.86	\$956.64
	John Deere 210 Gannon / Skip Loader	24.00	HR			\$86.82	\$2,083.68
	Foreman	24.00	HR			\$77.15	\$1,851.60
	Laborer	24.00	HR			\$44.30	\$1,063.20
	Operator: Small Equipment	24.00	HR			\$55.34	\$1,328.16
	3 - Install Pipe Plug Per MAG Standard Detail 427	8.00	EACH	576.00	72.00	\$10,050.84	\$80,406.72
	Install Pipe Plug	8.00	EACH	576.00	72.00	\$9,175.84	\$73,406.72
	Large Diameter Pipe Plug Crew (1.00 EACH/DY, 8.00 DY)	8.00	EACH	576.00	72.00	\$8,925.84	\$71,406.72
	F-250 to F-450	64.00	HR			\$39.86	\$2,551.04
	Cat 950	64.00	HR			\$104.46	\$6,685.44
	Excavator - 160 K LB	64.00	HR			\$304.86	\$19,511.04
	Water Truck 4000Gal	64.00	HR			\$75.36	\$4,823.04
	Backhoe Light	64.00	HR			\$86.11	\$5,511.04
	Foreman	64.00	HR			\$77.15	\$4,937.60
	Operator: Large Equipment [3]	64.00	HR			\$60.68	\$11,650.56
	Pipelayer [2]	64.00	HR			\$51.05	\$6,534.40
	Laborer [2]	64.00	HR			\$44.30	\$5,670.40
	Water Truck Driver	64.00	HR			\$55.19	\$3,532.16
	Cement - Concrete Mix 60# (Arizona Materials LLC)	160.00	UNIT			\$8.00	\$1,280.00
	Concrete Block 8x8x16	288.00	UNIT			\$2.50	\$720.00
	Shoring	14.00	DY			\$500.00	\$7,000.00
	4 - Remove Existing 48" Sanitary Sewer As Needed For Construction Of New 66" (HOBAS) Pipe	63.00	LF	264.00	4.19	\$1,039.65	\$65,497.64
	Dig, Remove Pipe & Backfill	63.00	LF	264.00	4.19	\$982.50	\$61,897.64
	Large Diameter Removal Crew (21.00 LF/DY, 3.00 DY)	63.00	LF	264.00	4.19	\$720.04	\$45,362.64
	F-250 to F-450	24.00	HR			\$39.86	\$956.64
	Water Truck 4000Gal	24.00	HR			\$75.36	\$1,808.64
	Cat 395 (Weight 190K)	24.00	HR			\$398.75	\$9,570.00
	Excavator - 160 K LB	24.00	HR			\$304.86	\$7,316.64
	Backhoe Light	24.00	HR			\$86.11	\$2,066.64
	Hydraulic Hammer 10k FT LB	24.00	HR			\$88.45	\$2,122.80
	Dump Truck - 10 Wheel [2]	24.00	HR			\$88.40	\$4,243.20
	Cat 950	24.00	HR			\$104.46	\$2,507.04

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
(Item 4 - Remove Existing 48" Sanitary Sewer As Needed For Construction Of New 66" (HOBAS) Pipe)							
	Foreman	24.00	HR			\$77.15	\$1,851.60
	Laborer [2]	24.00	HR			\$44.30	\$2,126.40
	Operator: Large Equipment [3]	24.00	HR			\$60.68	\$4,368.96
	Pipelaye [2]	24.00	HR			\$51.05	\$2,450.40
	Truck Driver [2]	24.00	HR			\$55.19	\$2,649.12
	Water Truck Driver	24.00	HR			\$55.19	\$1,324.56
	DUMP FEES	27.00	LOAD			\$75.00	\$2,025.00
	Dump Fee - Pipe Disposal	6.00	EACH			\$100.00	\$600.00
	Trucking (1.00 DY, 8.25 Trucks, 4.13 EACH/HR)	33.00	EACH			\$270.00	\$8,910.00
	Shoring	10.00	DY			\$500.00	\$5,000.00
	Cut Pipe	2.00	EACH			\$1,800.00	\$3,600.00
	5 - Grout Fill Existing 48" Sanitary Sewer To Extents Shown	387.00	LF	288.00	0.74	\$212.93	\$82,403.56
	Grout Fill And Abandon Pipe	387.00	EACH	288.00	0.74	\$200.01	\$77,403.56
	Large Diameter Abandonment Crew (96.75 EACH/DY, 4.00 DY)	387.00	EACH	288.00	0.74	\$100.10	\$38,738.56
	F-250 to F-450	32.00	HR			\$39.86	\$1,275.52
	Water Truck 4000Gal	32.00	HR			\$75.36	\$2,411.52
	Cat 395 (Weight 190K)	32.00	HR			\$218.28	\$6,984.96
	Cat 950	32.00	HR			\$79.64	\$2,548.48
	Backhoe Light	32.00	HR			\$86.11	\$2,755.52
	Grout Pump	32.00	HR			\$206.25	\$6,600.00
	Foreman	32.00	HR			\$77.15	\$2,468.80
	Operator: Large Equipment [3]	32.00	HR			\$60.68	\$5,825.28
	Pipelaye [2]	32.00	HR			\$51.05	\$3,267.20
	Laborer [2]	32.00	HR			\$44.30	\$2,835.20
	Water Truck Driver	32.00	HR			\$55.19	\$1,766.08
	Pumpable Grout Fill	209.00	UNIT			\$185.00	\$38,665.00
	Shoring	10.00	DY			\$500.00	\$5,000.00
	6 - Abandon Existing 108" Dia Manhole 756009379	1.00	EACH	168.00	168.00	\$27,020.66	\$27,020.66
	Abandon Existing Manhole In Place	1.00	EACH	168.00	168.00	\$21,803.76	\$21,803.76
	Structure Abandonment Crew (0.33 EACH/DY, 3.00 DY)	1.00	EACH	168.00	168.00	\$21,803.76	\$21,803.76
	F-250 to F-450	24.00	HR			\$39.86	\$956.64
	Backhoe Light	24.00	HR			\$86.11	\$2,066.64
	Excavator - 160K LB	24.00	HR			\$304.86	\$7,316.64
	Hydraulic Hammer 10k FT LB	24.00	HR			\$88.45	\$2,122.80
	Foreman	24.00	HR			\$77.15	\$1,851.60
	Operator: Large Equipment [2]	24.00	HR			\$60.68	\$2,912.64
	Pipelaye [2]	24.00	HR			\$51.05	\$2,450.40
	Laborer [2]	24.00	HR			\$44.30	\$2,126.40
	1/2 Sack Slurry	50.40	CY			\$103.51	\$5,216.90
	7 - Abandon Existing Junction Structure No. 2	1.00	EACH	232.00	232.00	\$65,995.34	\$65,995.34
	Remove Lids, Concrete Rings, Riser, And Grout Fill	1.00	EACH	232.00	232.00	\$33,563.12	\$33,563.12
	Demo, Grout Fill, Backfill & Restore Crew (0.25 EACH/DY, 4.00 DY)	1.00	EACH	232.00	232.00	\$33,563.12	\$33,563.12

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 7 - Abandon Existing Junction Structure No. 2 continued)</i>							
	F-250 to F-450	32.00	HR			\$39.86	\$1,275.52
	Excavator - 160 K LB	32.00	HR			\$304.86	\$9,755.52
	Cat 950	32.00	HR			\$104.46	\$3,342.72
	Backhoe Light	32.00	HR			\$86.11	\$2,755.52
	Dump Truck - 10 Wheel	8.00	HR			\$88.40	\$707.20
	Hydraulic Hammer 10k FT LB	32.00	HR			\$88.45	\$2,830.40
	Foreman	32.00	HR			\$77.15	\$2,468.80
	Operator: Large Equipment [2]	32.00	HR			\$60.68	\$3,883.52
	Pipelayer [2]	32.00	HR			\$51.05	\$3,267.20
	Laborer [2]	32.00	HR			\$44.30	\$2,835.20
	Truck Driver	8.00	HR			\$55.19	\$441.52
	Trucking (2.00 DY, 0.13 Trucks, 0.06 EACH/HR)	1.00	EACH			\$270.00	\$270.00
	DUMP FEES	1.00	LOAD			\$100.00	\$100.00
	1/2 Sack Slurry	309.75	CY			\$103.51	\$32,062.22
	8 - Furnish And Install 48" Sanitary Sewer (PVC SDR-26)	203.00	LF	609.00	3.00	\$1,377.18	\$279,567.56
	48" Sewer Installation	203.00	LF	609.00	3.00	\$728.36	\$147,857.66
	48" Mainline Sewer Crew (29.00 LF/DY, 7.00 DY)	203.00	LF	560.00	2.76	\$461.11	\$93,604.56
	F-250 to F-450	56.00	HR			\$39.86	\$2,232.16
	Dump Truck - 10 Wheel	56.00	HR			\$88.40	\$4,950.40
	Water Truck 4000Gal	56.00	HR			\$75.36	\$4,220.16
	Cat 395 (Weight 190K)	56.00	HR			\$398.75	\$22,330.00
	Excavator - 160 K LB	56.00	HR			\$304.86	\$17,072.16
	Cat 950	56.00	HR			\$104.46	\$5,849.76
	Backhoe Light	56.00	HR			\$86.11	\$4,822.16
	36" Compaction Wheel 320/325/335/349	56.00	HR			\$13.44	\$752.64
	Foreman	56.00	HR			\$77.15	\$4,320.40
	Operator: Large Equipment [3]	56.00	HR			\$60.68	\$10,194.24
	Pipelayer [2]	56.00	HR			\$51.05	\$5,717.60
	Laborer [2]	56.00	HR			\$44.30	\$4,961.60
	Truck Driver	56.00	HR			\$55.19	\$3,090.64
	Water Truck Driver	56.00	HR			\$55.19	\$3,090.64
	Trucking (1.00 DY, 27.00 Trucks, 13.50 EACH/HR)	108.00	EACH			\$270.00	\$29,160.00
	DUMP FEES	108.00	LOAD			\$100.00	\$10,800.00
	Shoring	10.00	DY			\$750.00	\$7,500.00
	Sewerline Testing	203.00	LF	49.00	0.24	\$33.46	\$6,793.10
	Sewerline Testing Crew (203.00 LF/DY, 1.00 DY)	203.00	LF	40.00	0.20	\$24.89	\$5,053.60
	42-78 Inch Plug [2]	8.00	HR			\$19.88	\$318.08
	185 Compressor	8.00	HR			\$51.69	\$413.52
	Backhoe Light	8.00	HR			\$86.11	\$688.88
	24-inch Trailer Mounted Blower	8.00	HR			\$51.25	\$410.00
	Confined Space Equipment	8.00	HR			\$15.63	\$125.04
	Flatbed Trailers 12 to 18 Fort Trailer	8.00	HR			\$12.75	\$102.00
	Vac Trailer 500 Gallon	8.00	HR			\$56.36	\$450.88
	F-250 to F-450	8.00	HR			\$39.86	\$318.88

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
(Item 8 - Furnish And Install 48" Sanitary Sewer (PVC SDR-26) continued)							
	Foreman	8.00	HR			\$77.15	\$617.20
	Laborer [2]	8.00	HR			\$44.30	\$708.80
	Leadman	8.00	HR			\$51.86	\$414.88
	Operator: Large Equipment	8.00	HR			\$60.68	\$485.44
	GC-Combo Hydro Truck Crew (812.00 LF/DY, 0.25 DY)	203.00	LF	4.00	0.02	\$3.31	\$671.12
	Combo Vactor Truck	2.00	HR			\$225.00	\$450.00
	Combo Truck Assistant	2.00	HR			\$48.46	\$96.92
	Combo Truck Operator	2.00	HR			\$62.10	\$124.20
	CCTV Inspection (4 Hour Minimum Or 1000 LF) (406.00 LF/DY, 0.50 DY)	203.00	LF	5.00	0.02	\$5.26	\$1,068.38
	Camera Truck	4.00	HR			\$165.00	\$660.00
	CCTV Technician	5.00	HR			\$81.68	\$408.38
	Material Buy	1.00	EACH	0.00	0.00	\$131,709.91	\$131,709.91
	1/2 Sack Slurry	190.28	CY			\$103.51	\$19,695.37
	Aggregate - ABC MAG Spec	1,738.28	TON			\$20.00	\$34,765.50
	Pipe - PVC SDR 26 Pipe 48in	224.00	LF			\$344.68	\$77,208.54
	Detectable Tape	1.00	ROLL			\$40.50	\$40.50
	9 - Remove Existing Plug And Connect To Existing 66" Stub Out	1.00	EACH	60.00	60.00	\$15,331.12	\$15,331.12
	Expose Existing Pipe Stub And Remove Plug	1.00	EACH	60.00	60.00	\$15,331.12	\$15,331.12
	66" Sewer Crew (2.00 EACH/DY, 0.50 DY)	1.00	EACH	44.00	44.00	\$7,206.64	\$7,206.64
	F-250 to F-450	4.00	HR			\$39.86	\$159.44
	Dump Truck - 10 Wheel [2]	4.00	HR			\$88.40	\$707.20
	Water Truck 4000Gal	4.00	HR			\$75.36	\$301.44
	Cat 395 (Weight 190K)	4.00	HR			\$398.75	\$1,595.00
	Excavator - 160 K LB	4.00	HR			\$304.86	\$1,219.44
	Cat 950	4.00	HR			\$104.46	\$417.84
	Backhoe Light	4.00	HR			\$86.11	\$344.44
	Foreman	4.00	HR			\$77.15	\$308.60
	Operator: Large Equipment [3]	4.00	HR			\$60.68	\$728.16
	Laborer [2]	4.00	HR			\$44.30	\$354.40
	Pipelayer [2]	4.00	HR			\$51.05	\$408.40
	Truck Driver [2]	4.00	HR			\$55.19	\$441.52
	Water Truck Driver	4.00	HR			\$55.19	\$220.76
	GC-Combo Hydro Truck Crew (1.00 EACH/DY, 1.00 DY)	1.00	EACH	16.00	16.00	\$2,684.48	\$2,684.48
	Combo Vactor Truck	8.00	HR			\$225.00	\$1,800.00
	Combo Truck Assistant	8.00	HR			\$48.46	\$387.68
	Combo Truck Operator	8.00	HR			\$62.10	\$496.80
	Trucking (1.00 DY, 3.00 Trucks, 1.50 EACH/HR)	12.00	EACH			\$270.00	\$3,240.00
	DUMP FEES	12.00	LOAD			\$100.00	\$1,200.00
	Shoring	2.00	DY			\$500.00	\$1,000.00
	10 - Furnish & Install 66" Sanitary Sewer (Hobas FWC Coupling Joint)	776.00	LF	3,840.89	4.95	\$2,061.63	\$1,599,824.82
	66" Sewerline Installation	776.00	LF	3,840.89	4.95	\$1,277.02	\$990,968.16
	66" Sewer Crew (18.00 LF/DY, 43.11 DY)	776.00	LF	3,448.89	4.44	\$782.18	\$606,973.40
	F-250 to F-450	344.89	HR			\$39.86	\$13,747.27

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 10 - Furnish & Install 66" Sanitary Sewer (Hobas FWC Coupling Joint) continued)</i>							
 Dump Truck - 10 Wheel [2]		344.89	HR			\$88.40	\$60,976.36
 Water Truck 4000Gal		344.89	HR			\$75.36	\$25,990.83
 Cat 395 (Weight 190K)		344.89	HR			\$398.75	\$137,524.44
 Excavator - 160 K LB		344.89	HR			\$304.86	\$105,142.83
 Cat 950		344.89	HR			\$104.46	\$36,027.09
 Backhoe Light		344.89	HR			\$86.11	\$29,698.38
 36" Compaction Wheel 320/325/335/349		344.89	HR			\$13.44	\$4,635.31
 Foreman		344.89	HR			\$77.15	\$26,608.18
 Operator: Large Equipment [3]		344.89	HR			\$60.68	\$62,783.57
 Laborer [2]		344.89	HR			\$44.30	\$30,557.16
 Pipelayer [2]		344.89	HR			\$51.05	\$35,213.16
 Truck Driver		344.89	HR			\$55.19	\$19,034.42
 Water Truck Driver		344.89	HR			\$55.19	\$19,034.42
 Trucking (1.00 DY, 148.75 Trucks, 74.38 EACH/HR)		595.00	EACH			\$270.00	\$160,650.00
 DUMP FEES		595.00	LOAD			\$100.00	\$59,500.00
 Shoring		60.00	DY			\$1,825.00	\$109,500.00
 Sewerline Testing		776.00	LF	392.00	0.51	\$70.03	\$54,344.76
 Sewerline Testing Crew (97.00 LF/DY, 8.00 DY)		776.00	LF	320.00	0.41	\$52.10	\$40,428.80
 42-78 Inch Plug [2]		64.00	HR			\$19.88	\$2,544.64
 185 Compressor		64.00	HR			\$51.69	\$3,308.16
 Backhoe Light		64.00	HR			\$86.11	\$5,511.04
 24-inch Trailer Mounted Blower		64.00	HR			\$51.25	\$3,280.00
 Confined Space Equipment		64.00	HR			\$15.63	\$1,000.32
 Flatbed Trailers 12 to 18 Fort Trailer		64.00	HR			\$12.75	\$816.00
 Vac Trailer 500 Gallon		64.00	HR			\$56.36	\$3,607.04
 F-250 to F-450		64.00	HR			\$39.86	\$2,551.04
 Foreman		64.00	HR			\$77.15	\$4,937.60
 Laborer [2]		64.00	HR			\$44.30	\$5,670.40
 Leadman		64.00	HR			\$51.86	\$3,319.04
 Operator: Large Equipment		64.00	HR			\$60.68	\$3,883.52
 GC-Combo Hydro Truck Crew (388.00 LF/DY, 2.00 DY)		776.00	LF	32.00	0.04	\$6.92	\$5,368.96
 Combo Vactor Truck		16.00	HR			\$225.00	\$3,600.00
 Combo Truck Assistant		16.00	HR			\$48.46	\$775.36
 Combo Truck Operator		16.00	HR			\$62.10	\$993.60
 CCTV Inspection (4 Hour Minimum Or 1000 LF) (194.00 LF/DY, 4.00 DY)		776.00	LF	40.00	0.05	\$11.01	\$8,547.00
 Camera Truck		32.00	HR			\$165.00	\$5,280.00
 CCTV Technician		40.00	HR			\$81.68	\$3,267.00
 Material Buy		1.00	EACH	0.00	0.00	\$608,856.66	\$608,856.66
 Aggregate - ABC MAG Spec		8,980.55	TON			\$20.00	\$179,611.00
 1/2 Sack Slurry		1,467.38	CY			\$103.51	\$151,887.99
 66" Hobas Pipe		840.00	LF			\$330.00	\$277,200.00
 Detectable Tape		2.00	ROLL			\$40.50	\$81.00
 Misc. - Tracer Wire		1.00	LF			\$76.67	\$76.67

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
D	11 - Remove Existing 66" Sanitary Sewer And Replace With 66" Hobas (FWC Coupling Joint Type)	110.00	LF	1,673.00	15.21	\$4,489.87	\$493,885.30
D	66" Sewerline Removal & Replacement	110.00	LF	1,624.00	14.76	\$3,476.19	\$382,380.73
	66" Remove & Replace Sewer Crew (7.33 LF/DY, 15.00 DY)	110.00	LF	1,200.00	10.91	\$2,016.39	\$221,803.20
	F-250 to F-450	120.00	HR			\$39.86	\$4,783.20
	Dump Truck - 10 Wheel [2]	120.00	HR			\$88.40	\$21,216.00
	Water Truck 4000Gal	120.00	HR			\$75.36	\$9,043.20
	Cat 395 (Weight 190K)	120.00	HR			\$398.75	\$47,850.00
	Excavator - 160 K LB	120.00	HR			\$304.86	\$36,583.20
	Cat 950	120.00	HR			\$104.46	\$12,535.20
	Backhoe Light	120.00	HR			\$86.11	\$10,333.20
	Hydraulic Hammer 10k FT LB	120.00	HR			\$88.45	\$10,614.00
	36" Compaction Wheel 320/325/335/349	120.00	HR			\$13.44	\$1,612.80
	Foreman	120.00	HR			\$77.15	\$9,258.00
	Operator: Large Equipment [3]	120.00	HR			\$60.68	\$21,844.80
	Laborer [2]	120.00	HR			\$44.30	\$10,632.00
	Pipelaye [2]	120.00	HR			\$51.05	\$12,252.00
	Truck Driver	120.00	HR			\$55.19	\$6,622.80
	Water Truck Driver	120.00	HR			\$55.19	\$6,622.80
	Locate, Remove Existing Collar, And Tie-in (0.33 EACH/DY, 3.00 DY)	1.00	EACH	216.00	216.00	\$40,691.28	\$40,691.28
	F-250 to F-450	24.00	HR			\$39.86	\$956.64
	Dump Truck - 10 Wheel	24.00	HR			\$88.40	\$2,121.60
	Water Truck 4000Gal	24.00	HR			\$75.36	\$1,808.64
	Cat 395 (Weight 190K)	24.00	HR			\$398.75	\$9,570.00
	Excavator - 160 K LB	24.00	HR			\$304.86	\$7,316.64
	Cat 950	24.00	HR			\$104.46	\$2,507.04
	Backhoe Light	24.00	HR			\$86.11	\$2,066.64
	Hydraulic Hammer 10k FT LB	24.00	HR			\$88.45	\$2,122.80
	Foreman	24.00	HR			\$77.15	\$1,851.60
	Laborer [2]	24.00	HR			\$44.30	\$2,126.40
	Operator: Large Equipment [3]	24.00	HR			\$60.68	\$4,368.96
	Pipelaye	24.00	HR			\$51.05	\$1,225.20
	Truck Driver	24.00	HR			\$55.19	\$1,324.56
	Water Truck Driver	24.00	HR			\$55.19	\$1,324.56
	Trucking (1.00 DY, 39.50 Trucks, 19.75 EACH/HR)	158.00	EACH			\$270.00	\$42,660.00
	DUMP FEES	158.00	LOAD			\$100.00	\$15,800.00
	Shoring	20.00	DY			\$1,250.00	\$25,000.00
D	Hydro Excavate Around Pipe	2.00	EACH	128.00	64.00	\$10,737.92	\$21,475.84
	GC-Combo Hydro Truck Crew (0.25 EACH/DY, 8.00 DY)	2.00	EACH	128.00	64.00	\$10,737.92	\$21,475.84
	Combo Vactor Truck	64.00	HR			\$225.00	\$14,400.00
	Combo Truck Assistant	64.00	HR			\$48.46	\$3,101.44
	Combo Truck Operator	64.00	HR			\$62.10	\$3,974.40
D	Install Pipe Collar	1.00	EACH	80.00	80.00	\$11,350.41	\$11,350.41
	Install Pipe Collar 66" Pipe (0.50 EACH/DY, 2.00 DY)	1.00	EACH	80.00	80.00	\$9,977.76	\$9,977.76

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 11 - Remove Existing 66" Sanitary Sewer And Replace With 66" Hobas (FWC Coupling Joint Type) continued)</i>							
 F-250 to F-450		16.00	HR			\$39.86	\$637.76
 Excavator - 160 K LB		16.00	HR			\$304.86	\$4,877.76
 Foreman		16.00	HR			\$77.15	\$1,234.40
 Pipelayer [2]		16.00	HR			\$51.05	\$1,633.60
 Laborer		16.00	HR			\$44.30	\$708.80
 Operator: Small Equipment		16.00	HR			\$55.34	\$885.44
 Forms And Bar		1.00	EACH			\$750.00	\$750.00
 Ready Mix Concrete - MAG C 2000		4.20	CY			\$148.25	\$622.65
 Cut Pipe		2.00	EACH			\$1,800.00	\$3,600.00
 Material Buy		1.00	EACH	0.00	0.00	\$104,711.47	\$104,711.47
 Aggregate - ABC MAG Spec		1,651.20	TON			\$20.00	\$33,024.00
 1/2 Sack Slurry		245.10	CY			\$103.51	\$25,370.30
 66" Hobas Pipe		140.00	LF			\$330.00	\$46,200.00
 Detectable Tape		1.00	ROLL			\$40.50	\$40.50
 Misc. - Tracer Wire		1.00	LF			\$76.67	\$76.67
 Sewerline Testing		110.00	LF	49.00	0.45	\$61.76	\$6,793.10
 Sewerline Testing Crew (110.00 LF/DY, 1.00 DY)		110.00	LF	40.00	0.36	\$45.94	\$5,053.60
 42-78 Inch Plug [2]		8.00	HR			\$19.88	\$318.08
 185 Compressor		8.00	HR			\$51.69	\$413.52
 Backhoe Light		8.00	HR			\$86.11	\$688.88
 24-inch Trailer Mounted Blower		8.00	HR			\$51.25	\$410.00
 Confined Space Equipment		8.00	HR			\$15.63	\$125.04
 Flatbed Trailers 12 to 18 Fort Trailer		8.00	HR			\$12.75	\$102.00
 Vac Trailer 500 Gallon		8.00	HR			\$56.36	\$450.88
 F-250 to F-450		8.00	HR			\$39.86	\$318.88
 Foreman		8.00	HR			\$77.15	\$617.20
 Laborer [2]		8.00	HR			\$44.30	\$708.80
 Leadman		8.00	HR			\$51.86	\$414.88
 Operator: Large Equipment		8.00	HR			\$60.68	\$485.44
 GC-Combo Hydro Truck Crew (440.00 LF/DY, 0.25 DY)		110.00	LF	4.00	0.04	\$6.10	\$671.12
 Combo Vactor Truck		2.00	HR			\$225.00	\$450.00
 Combo Truck Assistant		2.00	HR			\$48.46	\$96.92
 Combo Truck Operator		2.00	HR			\$62.10	\$124.20
 CCTV Inspection (4 Hour Minimum Or 1000 LF) (220.00 LF/DY, 0.50 DY)		110.00	LF	5.00	0.05	\$9.71	\$1,068.38
 Camera Truck		4.00	HR			\$165.00	\$660.00
 CCTV Technician		5.00	HR			\$81.68	\$408.38
 12 - Furnish And Install 60" Polymer Concrete Sewer Manhole With 96" Precast Base		3.00	EACH	1,056.00	352.00	\$189,772.58	\$569,317.75
 Install 60" Manhole		3.00	EACH	1,056.00	352.00	\$180,272.58	\$540,817.75
 Excavate And Backfill Manhole		3.00	EACH	960.00	320.00	\$86,202.06	\$258,606.19
 Manhole Crew (0.25 EACH/DY, 12.00 DY)		3.00	EACH	960.00	320.00	\$53,058.24	\$159,174.72
 F-250 to F-450		96.00	HR			\$39.86	\$3,826.56
 Dump Truck - 10 Wheel		96.00	HR			\$88.40	\$8,486.40
 Water Truck 4000Gal		96.00	HR			\$75.36	\$7,234.56
 Cat 395 (Weight 190K)		96.00	HR			\$398.75	\$38,280.00

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 12 - Furnish And Install 60" Polymer Concrete Sewer Manhole With 96" Precast Base continued)</i>							
 Excavator - 160 K LB		96.00	HR			\$304.86	\$29,266.56
 Cat 950		96.00	HR			\$104.46	\$10,028.16
 Backhoe Light		96.00	HR			\$86.11	\$8,266.56
 Foreman		96.00	HR			\$77.15	\$7,406.40
 Operator: Large Equipment [3]		96.00	HR			\$60.68	\$17,475.84
 Pipelayer [2]		96.00	HR			\$51.05	\$9,801.60
 Laborer [2]		96.00	HR			\$44.30	\$8,505.60
 Truck Driver		96.00	HR			\$55.19	\$5,298.24
 Water Truck Driver		96.00	HR			\$55.19	\$5,298.24
 1/2 Sack Slurry		403.13	CY			\$103.51	\$41,727.47
 Aggregate - ABC MAG Spec		56.70	TON			\$20.00	\$1,134.00
 Trucking (1.00 DY, 11.50 Trucks, 5.75 EACH/HR)		46.00	EACH			\$270.00	\$12,420.00
 DUMP FEES		46.00	LOAD			\$75.00	\$3,450.00
 Shoring		22.00	DY			\$1,850.00	\$40,700.00
 Testing Structure		3.00	EACH	48.00	16.00	\$2,325.00	\$6,975.00
 Manhole Testing (2.00 EACH/DY, 1.50 DY)		3.00	EACH	48.00	16.00	\$2,325.00	\$6,975.00
 F-250 to F-450		12.00	HR			\$39.86	\$478.32
 42-78 Inch Plug [2]		12.00	HR			\$19.88	\$477.12
 185 Compressor		12.00	HR			\$51.69	\$620.28
 Backhoe Light		12.00	HR			\$86.11	\$1,033.32
 24-inch Trailer Mounted Blower		12.00	HR			\$51.25	\$615.00
 Confined Space Equipment		12.00	HR			\$15.63	\$187.56
 Flatbed Trailers 12 to 18 Fort Trailer		12.00	HR			\$12.75	\$153.00
 Vac Trailer 500 Gallon		12.00	HR			\$56.36	\$676.32
 Foreman		12.00	HR			\$77.15	\$925.80
 Pipelayer		12.00	HR			\$51.05	\$612.60
 Laborer		12.00	HR			\$44.30	\$531.60
 Operator: Small Equipment		12.00	HR			\$55.34	\$664.08
 MH Adjustment		3.00	EACH	48.00	16.00	\$1,745.52	\$5,236.56
 RH-Manhole Raised Crew (2.00 EACH/DY, 1.50 DY)		3.00	EACH	48.00	16.00	\$1,460.60	\$4,381.80
 F-250 to F-450		12.00	HR			\$39.86	\$478.32
 Backhoe Light		12.00	HR			\$86.11	\$1,033.32
 Flatbed Trailers 12 to 18 Fort Trailer		12.00	HR			\$12.75	\$153.00
 Foreman		12.00	HR			\$77.15	\$925.80
 Laborer [2]		12.00	HR			\$44.30	\$1,063.20
 Operator: Large Equipment		12.00	HR			\$60.68	\$728.16
 CONCRETE - MAG AA (4000 PSI)		4.00	CY			\$151.19	\$604.76
 Concrete - Short Load Fee		1.00	EACH			\$250.00	\$250.00
 Buy Manholes		3.00	EACH	0.00	0.00	\$90,000.00	\$270,000.00
 Manhole - Polymer Manhole - 60"		3.00	EACH			\$90,000.00	\$270,000.00
 Crane For Manhole Installation		3.00	EACH			\$9,500.00	\$28,500.00
 13 - Remove Existing Manhole 756004803 And Replace With 60" Polymer Concrete Sewer Manhole With 96" Precast Base (MH No. 4)		1.00	EACH	896.00	896.00	\$348,571.50	\$348,571.50
 Install 60" Manhole		1.00	EACH	896.00	896.00	\$339,071.50	\$339,071.50
 Hydro Excavate Around Pipe		1.00	EACH	64.00	64.00	\$10,737.92	\$10,737.92

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
(Item 13 - Remove Existing Manhole 756004803 And Replace With 60" Polymer Concrete Sewer Manhole With 96" Precast Base							
 GC-Combo Hydro Truck Crew (0.25 EACH/DY, 4.00 DY)		1.00	EACH	64.00	64.00	\$10,737.92	\$10,737.92
 Combo Vactor Truck		32.00	HR			\$225.00	\$7,200.00
 Combo Truck Assistant		32.00	HR			\$48.46	\$1,550.72
 Combo Truck Operator		32.00	HR			\$62.10	\$1,987.20
 Cut Pipe		2.00	EACH			\$1,800.00	\$3,600.00
 MH Adjustment		1.00	EACH	16.00	16.00	\$2,095.60	\$2,095.60
 RH-Manhole Raised Crew (2.00 EACH/DY, 0.50 DY)		1.00	EACH	16.00	16.00	\$1,460.60	\$1,460.60
 F-250 to F-450		4.00	HR			\$39.86	\$159.44
 Backhoe Light		4.00	HR			\$86.11	\$344.44
 Flatbed Trailers 12 to 18 Fort Trailer		4.00	HR			\$12.75	\$51.00
 Foreman		4.00	HR			\$77.15	\$308.60
 Laborer [2]		4.00	HR			\$44.30	\$354.40
 Operator: Large Equipment		4.00	HR			\$60.68	\$242.72
 CONCRETE - MAG AA (4000 PSI)		4.20	CY			\$151.19	\$635.00
 Excavate And Backfill Manhole		1.00	EACH	720.00	720.00	\$184,833.01	\$184,833.01
 SL-Manhole Coated 7ft Diameter Appurtenance Crew (0.10 EACH/DY, 10.00 DY)		1.00	EACH	720.00	720.00	\$135,637.60	\$135,637.60
 F-250 to F-450		80.00	HR			\$39.86	\$3,188.80
 Dump Truck - 10 Wheel		80.00	HR			\$88.40	\$7,072.00
 Water Truck 4000Gal		80.00	HR			\$75.36	\$6,028.80
 Cat 395 (Weight 190K)		80.00	HR			\$398.75	\$31,900.00
 Excavator - 160 K LB		80.00	HR			\$304.86	\$24,388.80
 Cat 950		80.00	HR			\$104.46	\$8,356.80
 Backhoe Light		80.00	HR			\$86.11	\$6,888.80
 Hydraulic Hammer 10k FT LB		80.00	HR			\$88.45	\$7,076.00
 Foreman		80.00	HR			\$77.15	\$6,172.00
 Laborer [2]		80.00	HR			\$44.30	\$7,088.00
 Operator: Large Equipment [3]		80.00	HR			\$60.68	\$14,563.20
 Pipelayer		80.00	HR			\$51.05	\$4,084.00
 Truck Driver		80.00	HR			\$55.19	\$4,415.20
 Water Truck Driver		80.00	HR			\$55.19	\$4,415.20
 1/2 Sack Slurry		288.75	CY			\$103.51	\$29,888.51
 Aggregate - ABC MAG Spec		23.10	TON			\$20.00	\$462.00
 Trucking (1.00 DY, 8.75 Trucks, 4.38 EACH/HR)		35.00	EACH			\$270.00	\$9,450.00
 Dump Fee - Concrete		4.00	LOAD			\$195.00	\$780.00
 DUMP FEES		31.00	LOAD			\$100.00	\$3,100.00
 Pipe - PVC SDR 26 Pipe 48in		16.00	LF			\$344.68	\$5,514.90
 Testing Structure		1.00	EACH	16.00	16.00	\$2,328.24	\$2,328.24
 Manhole Testing (2.00 EACH/DY, 0.50 DY)		1.00	EACH	16.00	16.00	\$2,328.24	\$2,328.24
 F-250 to F-450		4.00	HR			\$39.86	\$159.44
 42-78 Inch Plug [2]		4.00	HR			\$19.88	\$159.04
 185 Compressor		4.00	HR			\$51.69	\$206.76
 Backhoe Light		4.00	HR			\$86.11	\$344.44
 24-inch Trailer Mounted Blower		4.00	HR			\$51.25	\$205.00
 Confined Space Equipment		4.00	HR			\$15.63	\$62.52

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
(Item 13 - Remove Existing Manhole 756004803 And Replace With 60" Polymer Concrete Sewer Manhole With 96" Precast Base							
 Flatbed Trailers 12 to 18 Fort Trailer		4.00	HR			\$12.75	\$51.00
 Vac Trailer 500 Gallon		4.00	HR			\$56.36	\$225.44
 Foreman		4.00	HR			\$77.15	\$308.60
 Laborer		4.00	HR			\$44.30	\$177.20
 Leadman		4.00	HR			\$51.86	\$207.44
 Operator: Small Equipment		4.00	HR			\$55.34	\$221.36
 Install Pipe Collar		1.00	EACH	80.00	80.00	\$11,661.74	\$11,661.74
 Install Pipe Collar 66" Pipe (0.50 EACH/DY, 2.00 DY)		1.00	EACH	80.00	80.00	\$9,977.76	\$9,977.76
 F-250 to F-450		16.00	HR			\$39.86	\$637.76
 Excavator - 160 K LB		16.00	HR			\$304.86	\$4,877.76
 Foreman		16.00	HR			\$77.15	\$1,234.40
 Pipelayer [2]		16.00	HR			\$51.05	\$1,633.60
 Laborer		16.00	HR			\$44.30	\$708.80
 Operator: Small Equipment		16.00	HR			\$55.34	\$885.44
 Forms And Bar		1.00	EACH			\$750.00	\$750.00
 Ready Mix Concrete - MAG C 2000		6.30	CY			\$148.25	\$933.98
 Buy Manholes		1.00	EACH	0.00	0.00	\$96,065.00	\$96,065.00
 Manhole - Polymer Manhole - 60"		1.00	EACH			\$96,065.00	\$96,065.00
 Shoring		15.00	DY			\$1,850.00	\$27,750.00
 Crane For Manhole Installation		1.00	EACH			\$9,500.00	\$9,500.00
 14 - Furnish & Install 66" Sanitary Sewer (Hobas Flush Reline Joint) Inside 86" Steel Casing		639.00	LF	1,909.40	2.99	\$5,973.99	\$3,817,381.41
 Excavate Jack & Bore Pits And Set Shoring		2.00	EACH	1,008.00	504.00	\$110,520.62	\$221,041.24
 J&B Excavation Crew (0.14 EACH/DY, 14.00 DY)		2.00	EACH	1,008.00	504.00	\$89,993.12	\$179,986.24
 F-250 to F-450		112.00	HR			\$39.86	\$4,464.32
 Backhoe Light		112.00	HR			\$86.11	\$9,644.32
 Cat 395 (Weight 190K)		112.00	HR			\$398.75	\$44,660.00
 Cat 950		112.00	HR			\$104.46	\$11,699.52
 Dump Truck - 10 Wheel		112.00	HR			\$88.40	\$9,900.80
 Excavator - 160 K LB		112.00	HR			\$304.86	\$34,144.32
 Water Truck 4000Gal		112.00	HR			\$75.36	\$8,440.32
 Foreman		112.00	HR			\$77.15	\$8,640.80
 Laborer [2]		112.00	HR			\$44.30	\$9,923.20
 Operator: Large Equipment [3]		112.00	HR			\$60.68	\$20,388.48
 Pipelayer		112.00	HR			\$51.05	\$5,717.60
 Truck Driver		112.00	HR			\$55.19	\$6,181.28
 Water Truck Driver		112.00	HR			\$55.19	\$6,181.28
 Trucking (1.00 DY, 29.75 Trucks, 14.88 EACH/HR)		119.00	EACH			\$270.00	\$32,130.00
 DUMP FEES		119.00	LOAD			\$75.00	\$8,925.00
 Backfill Jack & Bore Pits And Set Shoring		2.00	EACH	616.00	308.00	\$85,310.23	\$170,620.46
 J&B Backfill Crew (0.29 EACH/DY, 7.00 DY)		2.00	EACH	616.00	308.00	\$53,037.60	\$106,075.20
 Backhoe Light		56.00	HR			\$86.11	\$4,822.16
 Cat 395 (Weight 190K)		56.00	HR			\$398.75	\$22,330.00
 Cat 950		56.00	HR			\$104.46	\$5,849.76
 Dump Truck - 10 Wheel [3]		56.00	HR			\$88.40	\$14,851.20

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
(Item 14 - Furnish & Install 66" Sanitary Sewer (Hobas Flush Reline Joint) Inside 86" Steel Casing continued)							
 Excavator - 160 K LB		56.00	HR			\$304.86	\$17,072.16
 F-250 to F-450		56.00	HR			\$39.86	\$2,232.16
 Water Truck 4000Gal		56.00	HR			\$75.36	\$4,220.16
 Foreman		56.00	HR			\$77.15	\$4,320.40
 Laborer [2]		56.00	HR			\$44.30	\$4,961.60
 Operator: Large Equipment [3]		56.00	HR			\$60.68	\$10,194.24
 Pipelayer		56.00	HR			\$51.05	\$2,858.80
 Truck Driver [3]		56.00	HR			\$55.19	\$9,271.92
 Water Truck Driver		56.00	HR			\$55.19	\$3,090.64
 Aggregate - ABC MAG Spec		1,227.45	TON			\$20.00	\$24,549.00
 1/2 Sack Slurry		386.40	CY			\$103.51	\$39,996.26
 J&B Casing Installation		639.00	LF	256.40	0.40	\$4,931.21	\$3,151,043.27
 J&B Sub Furnish & Install Casing (W/ 1.5% Bond)		639.00	LF			\$4,308.68	\$2,753,246.52
 Trucking (13.75 DY, 2.00 Trucks, 1.00 EACH/HR)		110.00	EACH			\$270.00	\$29,700.00
 DUMP FEES		110.00	LOAD			\$75.00	\$8,250.00
 J&B Support Crew & Spoil Haul - Off (9.97 LF/DY, 64.10 DY)		639.00	LF	256.40	0.40	\$167.55	\$107,066.75
 Cat 950		512.80	HR			\$104.46	\$53,567.35
 Water Truck 4000Gal		512.80	HR			\$75.36	\$38,644.80
 Operator: Large Equipment [0.25]		512.80	HR			\$60.68	\$7,779.21
 Water Truck Driver [0.25]		512.80	HR			\$55.19	\$7,075.39
 66" Hobas Pipe Flush Reline		660.00	LF			\$383.00	\$252,780.00
 Temp Fence Around Pits		7.00	MO			\$350.00	\$2,450.00
 Shoring		7.00	MO			\$38,280.02	\$267,960.14
 Sewerline Testing		639.00	LF	29.00	0.05	\$6.68	\$4,266.30
 Sewerline Testing Crew (1,278.00 LF/DY, 0.50 DY)		639.00	LF	20.00	0.03	\$3.95	\$2,526.80
 42-78 Inch Plug [2]		4.00	HR			\$19.88	\$159.04
 185 Compressor		4.00	HR			\$51.69	\$206.76
 Backhoe Light		4.00	HR			\$86.11	\$344.44
 24-inch Trailer Mounted Blower		4.00	HR			\$51.25	\$205.00
 Confined Space Equipment		4.00	HR			\$15.63	\$62.52
 Flatbed Trailers 12 to 18 Fort Trailer		4.00	HR			\$12.75	\$51.00
 Vac Trailer 500 Gallon		4.00	HR			\$56.36	\$225.44
 F-250 to F-450		4.00	HR			\$39.86	\$159.44
 Foreman		4.00	HR			\$77.15	\$308.60
 Laborer [2]		4.00	HR			\$44.30	\$354.40
 Leadman		4.00	HR			\$51.86	\$207.44
 Operator: Large Equipment		4.00	HR			\$60.68	\$242.72
 GC-Combo Hydro Truck Crew (2,556.00 LF/DY, 0.25 DY)		639.00	LF	4.00	0.01	\$1.05	\$671.12
 Combo Vactor Truck		2.00	HR			\$225.00	\$450.00
 Combo Truck Assistant		2.00	HR			\$48.46	\$96.92
 Combo Truck Operator		2.00	HR			\$62.10	\$124.20
 CCTV Inspection (4 Hour Minimum Or 1000 LF) (1,278.00 LF/DY, 0.50 DY)		639.00	LF	5.00	0.01	\$1.67	\$1,068.38
 Camera Truck		4.00	HR			\$165.00	\$660.00
 CCTV Technician		5.00	HR			\$81.68	\$408.38

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
D	15 - Remove Existing 48" Sanitary Sewer Prior To Steel Casing	2.00	EACH	512.00	256.00	\$83,716.80	\$167,433.60
D	Remove Existing 48" Sewer & Backfill	2.00	EACH	512.00	256.00	\$83,716.80	\$167,433.60
	Sewer Removal Crew (0.25 EACH/DY, 8.00 DY)	2.00	EACH	512.00	256.00	\$39,553.28	\$79,106.56
	F-250 to F-450	64.00	HR			\$39.86	\$2,551.04
	Backhoe Light	64.00	HR			\$86.11	\$5,511.04
	Cat 950	64.00	HR			\$104.46	\$6,685.44
	Dump Truck - 10 Wheel	64.00	HR			\$88.40	\$5,657.60
	Excavator - 160 K LB	64.00	HR			\$304.86	\$19,511.04
	Hydraulic Hammer 10k FT LB	64.00	HR			\$88.45	\$5,660.80
	Water Truck 4000Gal	64.00	HR			\$75.36	\$4,823.04
	Foreman	64.00	HR			\$77.15	\$4,937.60
	Laborer [2]	64.00	HR			\$44.30	\$5,670.40
	Operator: Large Equipment [2]	64.00	HR			\$60.68	\$7,767.04
	Pipelayer	64.00	HR			\$51.05	\$3,267.20
	Truck Driver	64.00	HR			\$55.19	\$3,532.16
	Water Truck Driver	64.00	HR			\$55.19	\$3,532.16
	Trucking (1.00 DY, 10.00 Trucks, 5.00 EACH/HR)	40.00	EACH			\$270.00	\$10,800.00
	DUMP FEES	40.00	LOAD			\$75.00	\$3,000.00
	Shoring For Sta. 107+96	6.00	DY			\$500.00	\$3,000.00
	Slide Rail For Sta. 108+65 (Pipe Is 5' From ADOT Canal)	2.00	WK			\$14,936.87	\$29,873.74
	Cut Pipe	4.00	EACH			\$1,800.00	\$7,200.00
	1/2 Sack Slurry	332.85	CY			\$103.51	\$34,453.30
D	16 - Furnish & Install New Junction Structure B	1.00	EACH	4,304.00	4,304.00	\$3,098,028.38	\$3,098,028.38
D	Excavate Pit, Set Slide Rail/Shoring, & Prep Subgrade	1.00	EACH	2,400.00	2,400.00	\$513,495.57	\$513,495.57
	Excavation (0.04 EACH/DY, 25.00 DY)	1.00	EACH	2,200.00	2,200.00	\$360,332.00	\$360,332.00
	F-250 to F-450	200.00	HR			\$39.86	\$7,972.00
	Foreman	200.00	HR			\$77.15	\$15,430.00
	Laborer [2]	200.00	HR			\$44.30	\$17,720.00
	Pipelayer [2]	200.00	HR			\$51.05	\$20,420.00
	Water Truck Driver	200.00	HR			\$55.19	\$11,038.00
	Dump Truck - 10 Wheel [2]	200.00	HR			\$88.40	\$35,360.00
	Water Truck 4000Gal	200.00	HR			\$75.36	\$15,072.00
	Cat 395 (Weight 190K)	200.00	HR			\$398.75	\$79,750.00
	Excavator - 160 K LB	200.00	HR			\$304.86	\$60,972.00
	Cat 950	200.00	HR			\$104.46	\$20,892.00
	Backhoe Light	200.00	HR			\$86.11	\$17,222.00
	Operator: Large Equipment [3]	200.00	HR			\$60.68	\$36,408.00
	Truck Driver [2]	200.00	HR			\$55.19	\$22,076.00
	Structure Subgrade Prep Crew (441.80 SF/DY, 5.00 DY)	2,209.00	SF	200.00	0.09	\$9.08	\$20,048.00
	F-250 to F-450	40.00	HR			\$39.86	\$1,594.40
	Water Truck 4000Gal	40.00	HR			\$75.36	\$3,014.40
	Cat 239 Skid Steer	40.00	HR			\$71.84	\$2,873.60
	Trench Compactor Remote Control	40.00	HR			\$31.11	\$1,244.40
	Foreman	40.00	HR			\$77.15	\$3,086.00

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 16 - Furnish & Install New Junction Structure B continued)</i>							
 Laborer		40.00	HR			\$44.30	\$1,772.00
 Operator: Small Equipment		40.00	HR			\$55.34	\$2,213.60
 Pipelayer		40.00	HR			\$51.05	\$2,042.00
 Water Truck Driver		40.00	HR			\$55.19	\$2,207.60
 Trucking (1.00 DY, 66.25 Trucks, 33.13 EACH/HR)		265.00	EACH			\$270.00	\$71,550.00
 DUMP FEES		265.00	LOAD			\$100.00	\$26,500.00
 Aggregate - ABC MAG Spec		242.55	TON			\$20.00	\$4,851.00
 1/2 Sack Slurry		291.90	CY			\$103.51	\$30,214.57
 Soft Dig Corners / Pipe		15.00	DY	240.00	16.00	\$2,684.48	\$40,267.20
 GC-Combo Hydro Truck Crew (1.00 DY/DY, 15.00 DY)		15.00	DY	240.00	16.00	\$2,684.48	\$40,267.20
 Combo Vactor Truck		120.00	HR			\$225.00	\$27,000.00
 Combo Truck Assistant		120.00	HR			\$48.46	\$5,815.20
 Combo Truck Operator		120.00	HR			\$62.10	\$7,452.00
 Remove Slide Rail & Backfill		1.00	EACH	576.00	576.00	\$213,795.55	\$213,795.55
 Remove Slide Rail & Backfill Crew (0.13 EACH/DY, 8.00 DY)		1.00	EACH	576.00	576.00	\$91,415.68	\$91,415.68
 Cat 395 (Weight 190K)		64.00	HR			\$398.75	\$25,520.00
 Cat 950		64.00	HR			\$104.46	\$6,685.44
 Excavator - 160 K LB		64.00	HR			\$304.86	\$19,511.04
 F-250 to F-450		64.00	HR			\$39.86	\$2,551.04
 Water Truck 4000Gal		64.00	HR			\$75.36	\$4,823.04
 Foreman		64.00	HR			\$77.15	\$4,937.60
 Laborer [2]		64.00	HR			\$44.30	\$5,670.40
 Operator: Large Equipment [3]		64.00	HR			\$60.68	\$11,650.56
 Pipelayer [2]		64.00	HR			\$51.05	\$6,534.40
 Water Truck Driver		64.00	HR			\$55.19	\$3,532.16
 1/2 Sack Slurry		1,182.30	CY			\$103.51	\$122,379.87
 Slide Rail Shoring		8.00	MO			\$67,054.72	\$536,437.76
 Crane For Slide Rail Installation & Remove		5.00	DY			\$9,500.00	\$47,500.00
 PVC T-Lock Installation (Sonco) (Sonco Construction, Inc.)		1.00	LS			\$223,760.00	\$223,760.00
 CIP Structure B Subcontractor (CS Construction Inc)		1.00	LS			\$931,382.40	\$931,382.40
 CIP Structure Material Buy		1.00	EACH	0.00	0.00	\$478,157.74	\$478,157.74
 Stop Logs		1.00	LS			\$270,034.00	\$270,034.00
 Davit Crane		1.00	LS			\$10,258.50	\$10,258.50
 Access Hatch		1.00	LS			\$41,305.95	\$41,305.95
 FRP Grating		1.00	LS			\$148,343.72	\$148,343.72
 Misc Hardware, Anchors, Kor-N-Seal Boots, Braided Rope, Signs, Etc.		1.00	LS			\$8,215.57	\$8,215.57
 Structure Leak Test		1.00	LS	320.00	320.00	\$49,680.80	\$49,680.80
 Manhole Testing (0.10 EACH/DY, 10.00 DY)		1.00	EACH	320.00	320.00	\$49,680.80	\$49,680.80
 F-250 to F-450		80.00	HR			\$39.86	\$3,188.80
 42-78 Inch Plug [4]		80.00	HR			\$19.88	\$6,361.60
 185 Compressor		80.00	HR			\$51.69	\$4,135.20
 Backhoe Light		80.00	HR			\$86.11	\$6,888.80

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
(Item 16 - Furnish & Install New Junction Structure B continued)							
	24-inch Trailer Mounted Blower	80.00	HR			\$51.25	\$4,100.00
	Confined Space Equipment	80.00	HR			\$15.63	\$1,250.40
	Flatbed Trailers 12 to 18 Foot Trailer	80.00	HR			\$12.75	\$1,020.00
	Vac Trailer 500 Gallon	80.00	HR			\$56.36	\$4,508.80
	Foreman	80.00	HR			\$77.15	\$6,172.00
	Pipelayer	80.00	HR			\$51.05	\$4,084.00
	Laborer	80.00	HR			\$44.30	\$3,544.00
	Operator: Small Equipment	80.00	HR			\$55.34	\$4,427.20
	Install Grating	1.00	EACH	480.00	480.00	\$37,969.60	\$37,969.60
	Structure Assembly Crew (0.10 EACH/DY, 10.00 DY)	1.00	EACH	480.00	480.00	\$37,969.60	\$37,969.60
	Backhoe Light	80.00	HR			\$86.11	\$6,888.80
	F-250 to F-450	80.00	HR			\$39.86	\$3,188.80
	Confined Space Equipment	80.00	HR			\$15.63	\$1,250.40
	Foreman	80.00	HR			\$77.15	\$6,172.00
	Laborer	80.00	HR			\$44.30	\$3,544.00
	Operator: Small Equipment	80.00	HR			\$55.34	\$4,427.20
	Pipelayer	80.00	HR			\$51.05	\$4,084.00
	Concrete Finisher [2]	80.00	HR			\$52.59	\$8,414.40
	Install Stop Logs	3.00	EACH	288.00	96.00	\$7,593.92	\$22,781.76
	Structure Assembly Crew (0.50 EACH/DY, 6.00 DY)	3.00	EACH	288.00	96.00	\$7,593.92	\$22,781.76
	Backhoe Light	48.00	HR			\$86.11	\$4,133.28
	F-250 to F-450	48.00	HR			\$39.86	\$1,913.28
	Confined Space Equipment	48.00	HR			\$15.63	\$750.24
	Foreman	48.00	HR			\$77.15	\$3,703.20
	Laborer	48.00	HR			\$44.30	\$2,126.40
	Operator: Small Equipment	48.00	HR			\$55.34	\$2,656.32
	Pipelayer	48.00	HR			\$51.05	\$2,450.40
	Concrete Finisher [2]	48.00	HR			\$52.59	\$5,048.64
	Temp Fence Around Pits	8.00	MO			\$350.00	\$2,800.00
	17 - Furnish & Install New Junction Structure C	1.00	EACH	1,792.00	1,792.00	\$1,263,349.01	\$1,263,349.01
	Excavate/Set Slide Rail And Remove Slide Rail/Backfill Of Structure	1.00	EACH	1,200.00	1,200.00	\$253,237.48	\$253,237.48
	Structure Excavation & Backfill Crew (0.07 EACH/DY, 15.00 DY)	1.00	EACH	1,200.00	1,200.00	\$198,968.40	\$198,968.40
	Backhoe Light	120.00	HR			\$86.11	\$10,333.20
	Cat 395 (Weight 190K)	120.00	HR			\$398.75	\$47,850.00
	Cat 950	120.00	HR			\$104.46	\$12,535.20
	Dump Truck - 10 Wheel	120.00	HR			\$88.40	\$10,608.00
	Excavator - 160 K LB	120.00	HR			\$304.86	\$36,583.20
	F-250 to F-450	120.00	HR			\$39.86	\$4,783.20
	Water Truck 4000Gal	120.00	HR			\$75.36	\$9,043.20
	Foreman	120.00	HR			\$77.15	\$9,258.00
	Laborer [2]	120.00	HR			\$44.30	\$10,632.00
	Operator: Large Equipment [3]	120.00	HR			\$60.68	\$21,844.80
	Pipelayer [2]	120.00	HR			\$51.05	\$12,252.00

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 17 - Furnish & Install New Junction Structure C continued)</i>							
 Truck Driver		120.00	HR			\$55.19	\$6,622.80
 Water Truck Driver		120.00	HR			\$55.19	\$6,622.80
 1/2 Sack Slurry		320.25	CY			\$103.51	\$33,149.08
 Aggregate - ABC MAG Spec		38.50	TON			\$20.00	\$770.00
 DUMP FEES		55.00	LOAD			\$100.00	\$5,500.00
 Trucking (6.25 DY, 2.20 Trucks, 1.10 EACH/HR)		55.00	EACH			\$270.00	\$14,850.00
 Soft Dig Corners / Pipe		10.00	DY	160.00	16.00	\$2,684.48	\$26,844.80
 GC-Combo Hydro Truck Crew (1.00 DY/DY, 10.00 DY)		10.00	DY	160.00	16.00	\$2,684.48	\$26,844.80
 Combo Vactor Truck		80.00	HR			\$225.00	\$18,000.00
 Combo Truck Assistant		80.00	HR			\$48.46	\$3,876.80
 Combo Truck Operator		80.00	HR			\$62.10	\$4,968.00
 Crane For Structure Installation		1.00	EACH			\$25,938.00	\$25,938.00
 Crane For Slide Rail Installation & Remove		2.00	EACH			\$9,500.00	\$19,000.00
 Testing Structure		1.00	EACH	80.00	80.00	\$12,475.28	\$12,475.28
 Manhole Testing (0.50 EACH/DY, 2.00 DY)		1.00	EACH	80.00	80.00	\$10,425.28	\$10,425.28
 F-250 to F-450		16.00	HR			\$39.86	\$637.76
 42-78 Inch Plug [3]		16.00	HR			\$19.88	\$954.24
 185 Compressor		16.00	HR			\$51.69	\$827.04
 Backhoe Light		16.00	HR			\$86.11	\$1,377.76
 24-inch Trailer Mounted Blower		16.00	HR			\$51.25	\$820.00
 Confined Space Equipment		16.00	HR			\$15.63	\$250.08
 Flatbed Trailers 12 to 18 Fort Trailer		16.00	HR			\$12.75	\$204.00
 Vac Trailer 500 Gallon		16.00	HR			\$56.36	\$901.76
 Foreman		16.00	HR			\$77.15	\$1,234.40
 Operator: Large Equipment		16.00	HR			\$60.68	\$970.88
 Laborer [2]		16.00	HR			\$44.30	\$1,417.60
 Leadman		16.00	HR			\$51.86	\$829.76
 Grout Exterior Joint		100.00	EACH			\$20.50	\$2,050.00
 Material Buy		1.00	EACH	0.00	0.00	\$830,218.21	\$830,218.21
 Junction Structure C		1.00	EACH			\$561,245.00	\$561,245.00
 Stop Logs		1.00	LS			\$186,689.00	\$186,689.00
 Access Hatch		1.00	LS			\$31,027.00	\$31,027.00
 Davit Crane		1.00	LS			\$10,258.50	\$10,258.50
 FRP Grating		1.00	LS			\$32,783.14	\$32,783.14
 Misc Hardware, Anchors, Kor-N-Seal Boots, Braided Rope, Signs, Etc.		1.00	LS			\$8,215.57	\$8,215.57
 Slide Rail Shoring System		3.00	MO			\$21,540.00	\$64,620.00
 Install Grating		1.00	EACH	160.00	160.00	\$14,777.40	\$14,777.40
 CP-Concrete Replace Crew (0.20 EACH/DY, 5.00 DY)		1.00	EACH	160.00	160.00	\$14,777.40	\$14,777.40
 Backhoe Light		40.00	HR			\$86.11	\$3,444.40
 F-250 to F-450		40.00	HR			\$39.86	\$1,594.40
 Confined Space Equipment		5.00	DY			\$125.00	\$625.00
 Foreman		40.00	HR			\$77.15	\$3,086.00
 Laborer		40.00	HR			\$44.30	\$1,772.00
 Operator: Small Equipment		40.00	HR			\$55.34	\$2,213.60

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
(Item 17 - Furnish & Install New Junction Structure C continued)							
	Pipelayer	40.00	HR			\$51.05	\$2,042.00
	Install Stop Logs	2.00	EACH	192.00	96.00	\$7,593.92	\$15,187.84
	Structure Assembly Crew (0.50 EACH/DY, 4.00 DY)	2.00	EACH	192.00	96.00	\$7,593.92	\$15,187.84
	Backhoe Light	32.00	HR			\$86.11	\$2,755.52
	F-250 to F-450	32.00	HR			\$39.86	\$1,275.52
	Confined Space Equipment	32.00	HR			\$15.63	\$500.16
	Foreman	32.00	HR			\$77.15	\$2,468.80
	Laborer	32.00	HR			\$44.30	\$1,417.60
	Operator: Small Equipment	32.00	HR			\$55.34	\$1,770.88
	Pipelayer	32.00	HR			\$51.05	\$1,633.60
	Concrete Finisher [2]	32.00	HR			\$52.59	\$3,365.76
	Temp Fence Around Pits	3.00	MO			\$350.00	\$1,050.00
	18 - Furnish & Install Junction Structure D (60" Polymer Concrete Manhole With 84" Base)	1.00	EACH	888.00	888.00	\$313,292.32	\$313,292.32
	Install 60" Manhole On 84" Precast Base	1.00	EACH	888.00	888.00	\$303,792.32	\$303,792.32
	Excavate And Backfill Manhole	1.00	EACH	720.00	720.00	\$183,585.21	\$183,585.21
	Structure Excavation & Backfill Crew (0.10 EACH/DY, 10.00 DY)	1.00	EACH	720.00	720.00	\$135,637.60	\$135,637.60
	F-250 to F-450	80.00	HR			\$39.86	\$3,188.80
	Dump Truck - 10 Wheel	80.00	HR			\$88.40	\$7,072.00
	Water Truck 4000Gal	80.00	HR			\$75.36	\$6,028.80
	Cat 395 (Weight 190K)	80.00	HR			\$398.75	\$31,900.00
	Excavator - 160 K LB	80.00	HR			\$304.86	\$24,388.80
	Cat 950	80.00	HR			\$104.46	\$8,356.80
	Backhoe Light	80.00	HR			\$86.11	\$6,888.80
	Hydraulic Hammer 10k FT LB	80.00	HR			\$88.45	\$7,076.00
	Foreman	80.00	HR			\$77.15	\$6,172.00
	Operator: Large Equipment [3]	80.00	HR			\$60.68	\$14,563.20
	Pipelayer	80.00	HR			\$51.05	\$4,084.00
	Laborer [2]	80.00	HR			\$44.30	\$7,088.00
	Truck Driver	80.00	HR			\$55.19	\$4,415.20
	Water Truck Driver	80.00	HR			\$55.19	\$4,415.20
	1/2 Sack Slurry	281.40	CY			\$103.51	\$29,127.71
	Aggregate - ABC MAG Spec	22.00	TON			\$20.00	\$440.00
	Trucking (1.00 DY, 9.25 Trucks, 4.63 EACH/HR)	37.00	EACH			\$270.00	\$9,990.00
	Dump Fee - Concrete	4.00	UNIT			\$100.00	\$400.00
	DUMP FEES	33.00	LOAD			\$75.00	\$2,475.00
	Pipe - PVC SDR 26 Pipe 48in	16.00	LF			\$344.68	\$5,514.90
	Cut Pipe	2.00	EACH			\$1,800.00	\$3,600.00
	MH Adjustment	1.00	EACH	16.00	16.00	\$1,778.10	\$1,778.10
	RH-Manhole Raised Crew (2.00 EACH/DY, 0.50 DY)	1.00	EACH	16.00	16.00	\$1,460.60	\$1,460.60
	F-250 to F-450	4.00	HR			\$39.86	\$159.44
	Backhoe Light	4.00	HR			\$86.11	\$344.44
	Flatbed Trailers 12 to 18 Fort Trailer	4.00	HR			\$12.75	\$51.00
	Foreman	4.00	HR			\$77.15	\$308.60
	Laborer [2]	4.00	HR			\$44.30	\$354.40

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
(Item 18 - Furnish & Install Junction Structure D (60" Polymer Concrete Manhole With 84" Base) continued)							
 Operator: Large Equipment		4.00	HR			\$60.68	\$242.72
 CONCRETE - MAG AA (4000 PSI)		2.10	CY			\$151.19	\$317.50
 Testing Structure		1.00	EACH	32.00	32.00	\$4,118.40	\$4,118.40
 Manhole Testing (1.00 EACH/DY, 1.00 DY)		1.00	EACH	32.00	32.00	\$4,118.40	\$4,118.40
 F-250 to F-450		8.00	HR			\$39.86	\$318.88
 42-78 Inch Plug [2]		8.00	HR			\$19.88	\$318.08
 185 Compressor		8.00	HR			\$29.85	\$238.80
 Backhoe Light		8.00	HR			\$40.69	\$325.52
 24-inch Trailer Mounted Blower		8.00	HR			\$51.25	\$410.00
 Confined Space Equipment		8.00	HR			\$15.63	\$125.04
 Flatbed Trailers 12 to 18 Fort Trailer		8.00	HR			\$12.75	\$102.00
 Vac Trailer 500 Gallon		8.00	HR			\$56.36	\$450.88
 Foreman		8.00	HR			\$77.15	\$617.20
 Laborer		8.00	HR			\$44.30	\$354.40
 Leadman		8.00	HR			\$51.86	\$414.88
 Operator: Small Equipment		8.00	HR			\$55.34	\$442.72
 Install Pipe Collar		1.00	EACH	120.00	120.00	\$31,650.62	\$31,650.62
 Install Pipe Collar 66" Pipe (0.33 EACH/DY, 3.00 DY)		1.00	EACH	120.00	120.00	\$14,966.64	\$14,966.64
 F-250 to F-450		24.00	HR			\$39.86	\$956.64
 Excavator - 160 K LB		24.00	HR			\$304.86	\$7,316.64
 Foreman		24.00	HR			\$77.15	\$1,851.60
 Pipelayer [2]		24.00	HR			\$51.05	\$2,450.40
 Laborer		24.00	HR			\$44.30	\$1,063.20
 Operator: Small Equipment		24.00	HR			\$55.34	\$1,328.16
 Forms And Bar		1.00	EACH			\$750.00	\$750.00
 Ready Mix Concrete - MAG C 2000		6.30	CY			\$148.25	\$933.98
 FRP End Treatment		1.00	EACH			\$15,000.00	\$15,000.00
 Buy Manholes		1.00	EACH	0.00	0.00	\$53,160.00	\$53,160.00
 Manhole - Polymer Manhole - 60"		1.00	EACH			\$53,160.00	\$53,160.00
 Shoring		14.00	DY			\$1,850.00	\$25,900.00
 Crane For Manhole Installation		1.00	EACH			\$9,500.00	\$9,500.00
 19 - Furnish & Install New Junction Structure E		1.00	EACH	2,380.00	2,380.00	\$1,176,438.77	\$1,176,438.77
 Excavate/Set Slide Rail And Remove Slide Rail/Backfill Of Structure		1.00	EACH	2,000.00	2,000.00	\$408,466.27	\$408,466.27
 Structure Excavation & Backfill Crew (0.04 EACH/DY, 25.00 DY)		1.00	EACH	2,000.00	2,000.00	\$349,304.00	\$349,304.00
 Backhoe Light		200.00	HR			\$86.11	\$17,222.00
 Cat 395 (Weight 190K)		200.00	HR			\$398.75	\$79,750.00
 Cat 950		200.00	HR			\$104.46	\$20,892.00
 Dump Truck - 10 Wheel		200.00	HR			\$88.40	\$17,680.00
 Excavator - 160 K LB		200.00	HR			\$304.86	\$60,972.00
 F-250 to F-450		200.00	HR			\$39.86	\$7,972.00
 Hydraulic Hammer 10k FT LB		200.00	HR			\$88.45	\$17,690.00
 Water Truck 4000Gal		200.00	HR			\$75.36	\$15,072.00
 Foreman		200.00	HR			\$77.15	\$15,430.00
 Laborer [2]		200.00	HR			\$44.30	\$17,720.00

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 19 - Furnish & Install New Junction Structure E continued)</i>							
	Operator: Large Equipment [3]	200.00	HR			\$60.68	\$36,408.00
	Pipelayer [2]	200.00	HR			\$51.05	\$20,420.00
	Truck Driver	200.00	HR			\$55.19	\$11,038.00
	Water Truck Driver	200.00	HR			\$55.19	\$11,038.00
	1/2 Sack Slurry	349.65	CY			\$103.51	\$36,192.27
	Aggregate - ABC MAG Spec	38.50	TON			\$20.00	\$770.00
	DUMP FEES	60.00	LOAD			\$100.00	\$6,000.00
	Trucking (6.25 DY, 2.40 Trucks, 1.20 EACH/HR)	60.00	EACH			\$270.00	\$16,200.00
	Soft Dig Corners / Pipe	10.00	DY	160.00	16.00	\$2,684.48	\$26,844.80
	GC-Combo Hydro Truck Crew (1.00 DY/DY, 10.00 DY)	10.00	DY	160.00	16.00	\$2,684.48	\$26,844.80
	Combo Truck Assistant	80.00	HR			\$48.46	\$3,876.80
	Combo Truck Operator	80.00	HR			\$62.10	\$4,968.00
	Combo Vactor Truck	80.00	HR			\$225.00	\$18,000.00
	Saw Cut Pipe	2.00	EACH			\$1,500.00	\$3,000.00
	Crane For Structure Installation	1.00	EACH			\$25,938.00	\$25,938.00
	Crane For Slide Rail Installation & Remove	2.00	DY			\$9,500.00	\$19,000.00
	Testing Structure	1.00	EACH	60.00	60.00	\$9,868.96	\$9,868.96
	Manhole Testing (0.67 EACH/DY, 1.50 DY)	1.00	EACH	60.00	60.00	\$7,818.96	\$7,818.96
	F-250 to F-450	12.00	HR			\$39.86	\$478.32
	42-78 Inch Plug [3]	12.00	HR			\$19.88	\$715.68
	185 Compressor	12.00	HR			\$51.69	\$620.28
	Backhoe Light	12.00	HR			\$86.11	\$1,033.32
	24-inch Trailer Mounted Blower	12.00	HR			\$51.25	\$615.00
	Confined Space Equipment	12.00	HR			\$15.63	\$187.56
	Flatbed Trailers 12 to 18 Fort Trailer	12.00	HR			\$12.75	\$153.00
	Vac Trailer 500 Gallon	12.00	HR			\$56.36	\$676.32
	Foreman	12.00	HR			\$77.15	\$925.80
	Operator: Large Equipment	12.00	HR			\$60.68	\$728.16
	Laborer [2]	12.00	HR			\$44.30	\$1,063.20
	Leadman	12.00	HR			\$51.86	\$622.32
	Grout Exterior Joint	100.00	EACH			\$20.50	\$2,050.00
	Material Buy	1.00	EACH	0.00	0.00	\$602,873.14	\$602,873.14
	Junction Structure E (Armorock Polymer)	1.00	EACH			\$552,735.00	\$552,735.00
	Access Hatch	1.00	LS			\$17,355.00	\$17,355.00
	FRP Grating	1.00	LS			\$32,783.14	\$32,783.14
	Slide Rail Shoring System	3.00	MO			\$21,540.00	\$64,620.00
	Install Grating	1.00	EACH	160.00	160.00	\$14,777.60	\$14,777.60
	CP-Concrete Replace Crew (0.20 EACH/DY, 5.00 DY)	1.00	EACH	160.00	160.00	\$14,777.60	\$14,777.60
	Backhoe Light	40.00	HR			\$86.11	\$3,444.40
	F-250 to F-450	40.00	HR			\$39.86	\$1,594.40
	Foreman	40.00	HR			\$77.15	\$3,086.00
	Laborer	40.00	HR			\$44.30	\$1,772.00
	Operator: Small Equipment	40.00	HR			\$55.34	\$2,213.60
	Pipelayer	40.00	HR			\$51.05	\$2,042.00
	Confined Space Equipment	40.00	HR			\$15.63	\$625.20

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 19 - Furnish & Install New Junction Structure E continued)</i>							
	Temp Fence Around Pits	3.00	MO			\$350.00	\$1,050.00
	20 - Install 60" Polymer Concrete Manhole Insert And 30" Diameter Composite Frame/Cover (Inserts)	8.00	EACH	1,613.33	201.67	\$69,859.71	\$558,877.72
	Install Inserts	8.00	EACH	1,109.33	138.67	\$21,930.36	\$175,442.88
	RH-Manhole Insert Install Crew (0.50 EACH/DY, 16.00 DY)	8.00	EACH	512.00	64.00	\$7,480.96	\$59,847.68
	Dump Truck - 10 Wheel	128.00	HR			\$88.40	\$11,315.20
	Excavator - 35K LB	128.00	HR			\$105.46	\$13,498.88
	F-250 to F-450	128.00	HR			\$39.86	\$5,102.08
	Flatbed Trailers 12 to 18 Fort Trailer	128.00	HR			\$12.75	\$1,632.00
	Foreman	128.00	HR			\$77.15	\$9,875.20
	Laborer [2]	128.00	HR			\$44.30	\$11,340.80
	Operator: Small Equipment	128.00	HR			\$55.34	\$7,083.52
	Mountain Grout	62.00	EACH			\$450.00	\$27,900.00
	DUMP FEES	16.00	LOAD			\$100.00	\$1,600.00
	RH-Manhole Insert Backfill Crew (0.50 EACH/DY, 16.00 DY)	8.00	EACH	512.00	64.00	\$7,480.96	\$59,847.68
	Dump Truck - 10 Wheel	128.00	HR			\$88.40	\$11,315.20
	Excavator - 35K LB	128.00	HR			\$105.46	\$13,498.88
	F-250 to F-450	128.00	HR			\$39.86	\$5,102.08
	Flatbed Trailers 12 to 18 Fort Trailer	128.00	HR			\$12.75	\$1,632.00
	Foreman	128.00	HR			\$77.15	\$9,875.20
	Laborer [2]	128.00	HR			\$44.30	\$11,340.80
	Operator: Small Equipment	128.00	HR			\$55.34	\$7,083.52
	1/2 Sack Slurry	140.70	CY			\$103.51	\$14,563.86
	RH-Manhole Adjust Crew (3.00 EACH/DY, 2.67 DY)	8.00	EACH	85.33	10.67	\$1,097.33	\$8,778.67
	F-250 to F-450	21.33	HR			\$39.86	\$850.35
	Flatbed Trailers 12 to 18 Fort Trailer	21.33	HR			\$12.75	\$272.00
	Foreman	21.33	HR			\$77.15	\$1,645.87
	Laborer [2]	21.33	HR			\$44.30	\$1,890.13
	Operator: Small Equipment	21.33	HR			\$55.34	\$1,180.59
	Backhoe Light	21.33	HR			\$86.11	\$1,837.01
	185 Compressor	21.33	HR			\$51.69	\$1,102.72
	CONCRETE - MAG AA (4000 PSI)	12.60	CY			\$151.19	\$1,904.99
	Concrete - Short Load Fee	4.00	EACH			\$250.00	\$1,000.00
	Bench Rebuild And FRP Channel, Bench, And Tie Into Armorock	8.00	EACH	384.00	48.00	\$30,454.52	\$243,636.16
	Manhole Prep & Rebuild Crew (0.50 EACH/DY, 16.00 DY)	8.00	EACH	384.00	48.00	\$12,807.52	\$102,460.16
	12-inch Electric Blower	128.00	HR			\$1.50	\$192.00
	185 Compressor	128.00	HR			\$51.69	\$6,616.32
	Coating Truck	128.00	HR			\$550.00	\$70,400.00
	Confined Space Entry	128.00	HR			\$15.63	\$2,000.64
	Foreman	128.00	HR			\$77.15	\$9,875.20
	Manhole Coating Assistant	128.00	HR			\$48.84	\$6,251.52
	Manhole Coating Technician	128.00	HR			\$55.66	\$7,124.48

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 20 - Install 60" Polymer Concrete Manhole Insert And 30" Diameter Composite Frame/Cover (Inserts) continued)</i>							
 Ready Mix Concrete - C - 120		56.00	EACH			\$21.00	\$1,176.00
 FRP Treatment Channel, Bench, And Tie Into To Armorock		8.00	EACH			\$17,500.00	\$140,000.00
 Manhole Insert Material Buy		8.00	EACH	0.00	0.00	\$15,560.00	\$124,480.00
 Armorock 60" Polymer Inserts		8.00	EACH			\$15,560.00	\$124,480.00
 Concrete Restoration		1.00	EACH	120.00	120.00	\$8,568.68	\$8,568.68
 CP-Concrete Sidewalk Crew (0.33 EACH/DY, 3.00 DY)		1.00	EACH	120.00	120.00	\$7,963.92	\$7,963.92
 F-250 to F-450		24.00	HR			\$39.86	\$956.64
 Box Trailer Pump Watch		24.00	HR			\$12.75	\$306.00
 Concrete Finisher [3]		24.00	HR			\$52.59	\$3,786.48
 Foreman		24.00	HR			\$77.15	\$1,851.60
 Laborer		24.00	HR			\$44.30	\$1,063.20
 CONCRETE - MAG AA (4000 PSI)		4.00	CY			\$151.19	\$604.76
 Asphalt Min. Mob		1.00	LS			\$3,250.00	\$3,250.00
 ADOT Chainlink Fence Removal & Replacements		1.00	EACH	0.00	0.00	\$1,500.00	\$1,500.00
 Fence Subcontractor		1.00	LS			\$1,500.00	\$1,500.00
 Manhole Inspections		8.00	EACH			\$250.00	\$2,000.00
 21 - Install 60" Polymer Concrete Manhole And 30" Diameter Composite Frame/Cover (Tee-Style)		1.00	EACH	608.00	608.00	\$200,635.08	\$200,635.08
 Install 60" Manhole		1.00	EACH	608.00	608.00	\$166,385.08	\$166,385.08
 Excavate And Backfill Manhole		1.00	EACH	480.00	480.00	\$124,401.00	\$124,401.00
 Manhole Crew (0.17 EACH/DY, 6.00 DY)		1.00	EACH	480.00	480.00	\$64,954.08	\$64,954.08
 F-250 to F-450		48.00	HR			\$39.86	\$1,913.28
 Dump Truck - 10 Wheel		48.00	HR			\$88.40	\$4,243.20
 Water Truck 4000Gal		48.00	HR			\$75.36	\$3,617.28
 Cat 395 (Weight 190K)		48.00	HR			\$398.75	\$19,140.00
 Cat 950		48.00	HR			\$104.46	\$5,014.08
 Backhoe Light		48.00	HR			\$86.11	\$4,133.28
 Foreman		48.00	HR			\$77.15	\$3,703.20
 Operator: Large Equipment [3]		48.00	HR			\$60.68	\$8,737.92
 Pipelayer [2]		48.00	HR			\$51.05	\$4,900.80
 Laborer [2]		48.00	HR			\$44.30	\$4,252.80
 Truck Driver		48.00	HR			\$55.19	\$2,649.12
 Water Truck Driver		48.00	HR			\$55.19	\$2,649.12
 1/2 Sack Slurry		274.05	CY			\$103.51	\$28,366.92
 Trucking (1.00 DY, 8.50 Trucks, 4.25 EACH/HR)		34.00	EACH			\$270.00	\$9,180.00
 DUMP FEES		34.00	LOAD			\$100.00	\$3,400.00
 Shoring		10.00	DY			\$1,850.00	\$18,500.00
 Testing Structure		1.00	EACH	16.00	16.00	\$2,099.56	\$2,099.56
 Manhole Testing (2.00 EACH/DY, 0.50 DY)		1.00	EACH	16.00	16.00	\$2,099.56	\$2,099.56
 F-250 to F-450		4.00	HR			\$39.86	\$159.44
 42-78 Inch Plug [2]		4.00	HR			\$19.88	\$159.04
 185 Compressor		4.00	HR			\$51.69	\$206.76
 Backhoe Light		4.00	HR			\$86.11	\$344.44
 24-inch Trailer Mounted Blower		4.00	HR			\$51.25	\$205.00
 Confined Space Equipment		4.00	HR			\$15.63	\$62.52

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 21 - Install 60" Polymer Concrete Manhole And 30" Diameter Composite Frame/Cover (Tee-Style) continued)</i>							
 Flatbed Trailers 12 to 18 Fort Trailer		4.00	HR			\$12.75	\$51.00
 Foreman		4.00	HR			\$77.15	\$308.60
 Pipelayer		4.00	HR			\$51.05	\$204.20
 Laborer		4.00	HR			\$44.30	\$177.20
 Operator: Small Equipment		4.00	HR			\$55.34	\$221.36
 MH Adjustment		1.00	EACH	16.00	16.00	\$1,762.98	\$1,762.98
 RH-Manhole Raised Crew (2.00 EACH/DY, 0.50 DY)		1.00	EACH	16.00	16.00	\$1,460.60	\$1,460.60
 F-250 to F-450		4.00	HR			\$39.86	\$159.44
 Backhoe Light		4.00	HR			\$86.11	\$344.44
 Flatbed Trailers 12 to 18 Fort Trailer		4.00	HR			\$12.75	\$51.00
 Foreman		4.00	HR			\$77.15	\$308.60
 Laborer [2]		4.00	HR			\$44.30	\$354.40
 Operator: Large Equipment		4.00	HR			\$60.68	\$242.72
 CONCRETE - MAG AA (4000 PSI)		2.00	CY			\$151.19	\$302.38
 Buy Manholes		1.00	EACH	0.00	0.00	\$29,120.00	\$29,120.00
 Armorock 60" Manhole Tee Style		1.00	EACH			\$29,120.00	\$29,120.00
 Wet Set Impression Ring Or Tee Adapter		1.00	EACH	96.00	96.00	\$9,001.54	\$9,001.54
 CIP Manhole Crew (0.50 EACH/DY, 2.00 DY)		1.00	EACH	96.00	96.00	\$7,343.84	\$7,343.84
 F-250 to F-450		16.00	HR			\$39.86	\$637.76
 Backhoe Light		16.00	HR			\$86.11	\$1,377.76
 Foreman		16.00	HR			\$77.15	\$1,234.40
 Operator: Small Equipment		16.00	HR			\$55.34	\$885.44
 Pipelayer		16.00	HR			\$51.05	\$816.80
 Laborer		16.00	HR			\$44.30	\$708.80
 Concrete Finisher [2]		16.00	HR			\$52.59	\$1,682.88
 Forms / Rebar Cure		1.00	EACH			\$750.00	\$750.00
 CONCRETE - MAG A (3000 PSI)		6.30	CY			\$144.08	\$907.70
 FRP Treatment And Installation		1.00	EACH			\$34,000.00	\$34,000.00
 Manhole Inspections		1.00	EACH			\$250.00	\$250.00
 22 - Install 48" Polymer Concrete Manhole And 30" Diameter Composite Frame/Cover (Tee Style)		4.00	EACH	2,432.00	608.00	\$132,459.19	\$529,836.77
 Install 48" Manhole		4.00	EACH	2,432.00	608.00	\$117,209.19	\$468,836.77
 Excavate And Backfill Manhole		4.00	EACH	1,920.00	480.00	\$90,100.45	\$360,401.82
 Manhole Crew (0.17 EACH/DY, 24.00 DY)		4.00	EACH	1,920.00	480.00	\$60,447.36	\$241,789.44
 F-250 to F-450		192.00	HR			\$39.86	\$7,653.12
 Dump Truck - 10 Wheel		192.00	HR			\$88.40	\$16,972.80
 Water Truck 4000Gal		192.00	HR			\$75.36	\$14,469.12
 Excavator - 160 K LB		192.00	HR			\$304.86	\$58,533.12
 Cat 950		192.00	HR			\$104.46	\$20,056.32
 Backhoe Light		192.00	HR			\$86.11	\$16,533.12
 Foreman		192.00	HR			\$77.15	\$14,812.80
 Operator: Large Equipment [3]		192.00	HR			\$60.68	\$34,951.68
 Pipelayer [2]		192.00	HR			\$51.05	\$19,603.20
 Laborer [2]		192.00	HR			\$44.30	\$17,011.20
 Truck Driver		192.00	HR			\$55.19	\$10,596.48

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 22 - Install 48" Polymer Concrete Manhole And 30" Diameter Composite Frame/Cover (Tee Style) continued)</i>							
 Water Truck Driver		192.00	HR			\$55.19	\$10,596.48
 1/2 Sack Slurry		398.83	CY			\$103.51	\$41,282.38
 Trucking (1.00 DY, 14.75 Trucks, 7.38 EACH/HR)		59.00	EACH			\$270.00	\$15,930.00
 DUMP FEES		59.00	LOAD			\$100.00	\$5,900.00
 Shoring		30.00	DY			\$1,850.00	\$55,500.00
 Testing Structure		4.00	EACH	64.00	16.00	\$2,099.56	\$8,398.24
 Manhole Testing (2.00 EACH/DY, 2.00 DY)		4.00	EACH	64.00	16.00	\$2,099.56	\$8,398.24
 F-250 to F-450		16.00	HR			\$39.86	\$637.76
 42-78 Inch Plug [2]		16.00	HR			\$19.88	\$636.16
 185 Compressor		16.00	HR			\$51.69	\$827.04
 Backhoe Light		16.00	HR			\$86.11	\$1,377.76
 24-inch Trailer Mounted Blower		16.00	HR			\$51.25	\$820.00
 Confined Space Equipment		16.00	HR			\$15.63	\$250.08
 Flatbed Trailers 12 to 18 Fort Trailer		16.00	HR			\$12.75	\$204.00
 Foreman		16.00	HR			\$77.15	\$1,234.40
 Pipelayer		16.00	HR			\$51.05	\$816.80
 Laborer		16.00	HR			\$44.30	\$708.80
 Operator: Small Equipment		16.00	HR			\$55.34	\$885.44
 MH Adjustment		4.00	EACH	64.00	16.00	\$1,937.39	\$7,749.54
 RH-Manhole Raised Crew (2.00 EACH/DY, 2.00 DY)		4.00	EACH	64.00	16.00	\$1,460.60	\$5,842.40
 F-250 to F-450		16.00	HR			\$39.86	\$637.76
 Backhoe Light		16.00	HR			\$86.11	\$1,377.76
 Flatbed Trailers 12 to 18 Fort Trailer		16.00	HR			\$12.75	\$204.00
 Foreman		16.00	HR			\$77.15	\$1,234.40
 Laborer [2]		16.00	HR			\$44.30	\$1,417.60
 Operator: Large Equipment		16.00	HR			\$60.68	\$970.88
 CONCRETE - MAG AA (4000 PSI)		6.00	CY			\$151.19	\$907.14
 Concrete - Short Load Fee		4.00	EACH			\$250.00	\$1,000.00
 Buy Manholes		4.00	EACH	0.00	0.00	\$14,070.25	\$56,281.00
 Armorock 48" Manhole Tee Style		4.00	EACH			\$14,070.25	\$56,281.00
 Wet Set Impression Ring Or Tee Adapter		4.00	EACH	384.00	96.00	\$9,001.54	\$36,006.18
 CIP Manhole Crew (0.50 EACH/DY, 8.00 DY)		4.00	EACH	384.00	96.00	\$7,343.84	\$29,375.36
 F-250 to F-450		64.00	HR			\$39.86	\$2,551.04
 Backhoe Light		64.00	HR			\$86.11	\$5,511.04
 Foreman		64.00	HR			\$77.15	\$4,937.60
 Operator: Small Equipment		64.00	HR			\$55.34	\$3,541.76
 Pipelayer		64.00	HR			\$51.05	\$3,267.20
 Laborer		64.00	HR			\$44.30	\$2,835.20
 Concrete Finisher [2]		64.00	HR			\$52.59	\$6,731.52
 Forms / Rebar Cure		4.00	EACH			\$750.00	\$3,000.00
 CONCRETE - MAG A (3000 PSI)		25.20	CY			\$144.08	\$3,630.82
 FRP Treatment And Installation		4.00	EACH			\$15,000.00	\$60,000.00
 Manhole Inspections		4.00	EACH			\$250.00	\$1,000.00
 23 - Existing Junction Structure No. 1 (New 60" Polymer MH Stackout & Abandonment Of South MH)		1.00	EACH	432.00	432.00	\$95,414.07	\$95,414.07

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 23 - Existing Junction Structure No. 1 (New 60" Polymer MH Stackout & Abandonment Of South MH) continued)</i>							
 Install 60" Manhole		1.00	EACH	416.00	416.00	\$70,050.67	\$70,050.67
 Excavate And Backfill Manhole		1.00	EACH	272.00	272.00	\$39,950.51	\$39,950.51
 Manhole Crew (0.25 EACH/DY, 4.00 DY)		1.00	EACH	272.00	272.00	\$26,164.64	\$26,164.64
 F-250 to F-450		32.00	HR			\$39.86	\$1,275.52
 Dump Truck - 10 Wheel [0.5]		32.00	HR			\$88.40	\$1,414.40
 Water Truck 4000Gal		32.00	HR			\$75.36	\$2,411.52
 Cat 320 (Weight 30K)		32.00	HR			\$105.46	\$3,374.72
 Backhoe Light		32.00	HR			\$86.11	\$2,755.52
 Foreman		32.00	HR			\$77.15	\$2,468.80
 Operator: Large Equipment		32.00	HR			\$60.68	\$1,941.76
 Pipelayer [2]		32.00	HR			\$51.05	\$3,267.20
 Laborer [2]		32.00	HR			\$44.30	\$2,835.20
 Truck Driver [0.5]		32.00	HR			\$55.19	\$883.04
 Water Truck Driver		32.00	HR			\$55.19	\$1,766.08
 Operator: Small Equipment		32.00	HR			\$55.34	\$1,770.88
 1/2 Sack Slurry		67.20	CY			\$103.51	\$6,955.87
 Trucking (1.00 DY, 2.25 Trucks, 1.13 EACH/HR)		9.00	EACH			\$270.00	\$2,430.00
 DUMP FEES		9.00	LOAD			\$100.00	\$900.00
 Shoring		7.00	DY			\$500.00	\$3,500.00
 Testing Structure		1.00	EACH	16.00	16.00	\$2,099.56	\$2,099.56
 Manhole Testing (2.00 EACH/DY, 0.50 DY)		1.00	EACH	16.00	16.00	\$2,099.56	\$2,099.56
 F-250 to F-450		4.00	HR			\$39.86	\$159.44
 42-78 Inch Plug [2]		4.00	HR			\$19.88	\$159.04
 185 Compressor		4.00	HR			\$51.69	\$206.76
 Backhoe Light		4.00	HR			\$86.11	\$344.44
 24-inch Trailer Mounted Blower		4.00	HR			\$51.25	\$205.00
 Confined Space Equipment		4.00	HR			\$15.63	\$62.52
 Flatbed Trailers 12 to 18 Fort Trailer		4.00	HR			\$12.75	\$51.00
 Foreman		4.00	HR			\$77.15	\$308.60
 Pipelayer		4.00	HR			\$51.05	\$204.20
 Laborer		4.00	HR			\$44.30	\$177.20
 Operator: Small Equipment		4.00	HR			\$55.34	\$221.36
 MH Adjustment		1.00	EACH	16.00	16.00	\$1,762.98	\$1,762.98
 RH-Manhole Raised Crew (2.00 EACH/DY, 0.50 DY)		1.00	EACH	16.00	16.00	\$1,460.60	\$1,460.60
 F-250 to F-450		4.00	HR			\$39.86	\$159.44
 Backhoe Light		4.00	HR			\$86.11	\$344.44
 Flatbed Trailers 12 to 18 Fort Trailer		4.00	HR			\$12.75	\$51.00
 Foreman		4.00	HR			\$77.15	\$308.60
 Laborer [2]		4.00	HR			\$44.30	\$354.40
 Operator: Large Equipment		4.00	HR			\$60.68	\$242.72
 CONCRETE - MAG AA (4000 PSI)		2.00	CY			\$151.19	\$302.38
 Buy Manholes		1.00	EACH	0.00	0.00	\$11,440.00	\$11,440.00
 Armorock 60" Manhole Tee Style		1.00	EACH			\$11,440.00	\$11,440.00
 Wet Set Impression Ring Or Tee Adapter		1.00	EACH	112.00	112.00	\$14,797.62	\$14,797.62
 CIP Manhole Crew (0.50 EACH/DY, 2.00 DY)		1.00	EACH	112.00	112.00	\$13,192.48	\$13,192.48
F-250 to F-450		16.00	HR			\$39.86	\$637.76

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 23 - Existing Junction Structure No. 1 (New 60" Polymer MH Stackout & Abandonment Of South MH) continued)</i>							
 Backhoe Light		16.00	HR			\$86.11	\$1,377.76
 Excavator - 160 K LB		16.00	HR			\$304.86	\$4,877.76
 Foreman		16.00	HR			\$77.15	\$1,234.40
 Operator: Large Equipment		16.00	HR			\$60.68	\$970.88
 Operator: Small Equipment		16.00	HR			\$55.34	\$885.44
 Pipelayer		16.00	HR			\$51.05	\$816.80
 Laborer		16.00	HR			\$44.30	\$708.80
 Concrete Finisher [2]		16.00	HR			\$52.59	\$1,682.88
 Forms / Rebar Cure		1.00	EACH			\$750.00	\$750.00
 CONCRETE - MAG A (3000 PSI)		4.20	CY			\$144.08	\$605.14
 Concrete - Short Load Fee		1.00	EACH			\$250.00	\$250.00
 FRP Treatment And Installation		1.00	EACH			\$24,000.00	\$24,000.00
 Man Entry T-Lock Removal		1.00	EACH	16.00	16.00	\$1,363.40	\$1,363.40
 T-Lock Removal - No Grout Back (2.00 EACH/DY, 0.50 DY)		1.00	EACH	16.00	16.00	\$1,363.40	\$1,363.40
 F-250 to F-450		4.00	HR			\$39.86	\$159.44
 24-inch Trailer Mounted Blower		4.00	HR			\$51.25	\$205.00
 Confined Space Equipment		4.00	HR			\$15.63	\$62.52
 Dump Trailer		4.00	HR			\$16.50	\$66.00
 Foreman		4.00	HR			\$77.15	\$308.60
 Laborer [2]		4.00	HR			\$44.30	\$354.40
 Leadman		4.00	HR			\$51.86	\$207.44
 24 - Clean Existing 30" Sanitary Sewer		938.00	LF	52.00	0.06	\$10.28	\$9,642.46
 30" Cleaning		938.00	LF	52.00	0.06	\$10.28	\$9,642.46
 3 Pass Cleaning (469.00 LF/DY, 2.00 DY)		938.00	LF	32.00	0.03	\$5.72	\$5,368.96
 Combo Vactor Truck		16.00	HR			\$225.00	\$3,600.00
 Combo Truck Assistant		16.00	HR			\$48.46	\$775.36
 Combo Truck Operator		16.00	HR			\$62.10	\$993.60
 CCTV Inspection (4 Hour Minimum Or 1000 LF) (469.00 LF/DY, 2.00 DY)		938.00	LF	20.00	0.02	\$4.56	\$4,273.50
 Camera Truck		16.00	HR			\$165.00	\$2,640.00
 CCTV Technician		20.00	HR			\$81.68	\$1,633.50
 25 - Clean Existing 48" Sanitary Sewer		1,997.00	LF	156.00	0.08	\$14.49	\$28,927.38
 48" Cleaning		1,997.00	LF	156.00	0.08	\$14.49	\$28,927.38
 3 Pass Cleaning (332.83 LF/DY, 6.00 DY)		1,997.00	LF	96.00	0.05	\$8.07	\$16,106.88
 Combo Vactor Truck		48.00	HR			\$225.00	\$10,800.00
 Combo Truck Assistant		48.00	HR			\$48.46	\$2,326.08
 Combo Truck Operator		48.00	HR			\$62.10	\$2,980.80
 CCTV Inspection (4 Hour Minimum Or 1000 LF) (332.83 LF/DY, 6.00 DY)		1,997.00	LF	60.00	0.03	\$6.42	\$12,820.50
 Camera Truck		48.00	HR			\$165.00	\$7,920.00
 CCTV Technician		60.00	HR			\$81.68	\$4,900.50
 26 - Clean Existing 66" Sanitary Sewer		652.00	LF	63.89	0.10	\$18.82	\$12,272.14
 66" Cleaning		652.00	LF	63.89	0.10	\$18.17	\$11,847.14
 GC-Combo Hydro Truck Crew (265.33 LF/DY, 2.46 DY)		652.00	LF	39.32	0.06	\$10.12	\$6,596.54
 Combo Vactor Truck		19.66	HR			\$225.00	\$4,423.12

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 26 - Clean Existing 66" Sanitary Sewer continued)</i>							
	Combo Truck Assistant	19.66	HR			\$48.46	\$952.64
	Combo Truck Operator	19.66	HR			\$62.10	\$1,220.78
	CCTV Inspection (4 Hour Minimum Or 1000 LF) (265.33 LF/DY, 2.46 DY)	652.00	LF	24.57	0.04	\$8.05	\$5,250.61
	Camera Truck	19.66	HR			\$165.00	\$3,243.62
	CCTV Technician	24.57	HR			\$81.68	\$2,006.99
	Dump Fees	1.00	EACH			\$425.00	\$425.00
	27 - Pipe Man Entry Cleaning & Detached Liner Removal	2,669.00	LF	244.63	0.09	\$7.74	\$20,658.77
	Man-Entry T-lock Removal	2,669.00	LF	244.63	0.09	\$7.74	\$20,658.77
	T-Lock Removal - No Grout Back (349.13 LF/DY, 7.64 DY)	2,669.00	LF	244.63	0.09	\$7.74	\$20,658.77
	F-250 to F-450	61.16	HR			\$39.86	\$2,437.78
	24-inch Trailer Mounted Blower	61.16	HR			\$40.69	\$2,488.54
	Confined Space Equipment	61.16	HR			\$15.63	\$955.91
	Dump Trailer	61.16	HR			\$24.00	\$1,467.81
	Foreman	61.16	HR			\$77.15	\$4,718.39
	Laborer [2]	61.16	HR			\$44.30	\$5,418.65
	Leadman	61.16	HR			\$51.86	\$3,171.69
	28 - Install 30" CIPP Liner	938.00	LF	284.00	0.30	\$292.97	\$274,810.20
	CIPP Lining 30" W/ 1.5% Bond	938.00	LF			\$254.77	\$238,974.26
	RH-CIPP Lining Support Crew & Confined Space (93.80 LF/DY, 10.00 DY)	938.00	LF	160.00	0.17	\$13.76	\$12,904.80
	F-250 to F-450	80.00	HR			\$39.86	\$3,188.80
	Foreman	80.00	HR			\$77.15	\$6,172.00
	Laborer	80.00	HR			\$44.30	\$3,544.00
	Post Pano	938.00	LF			\$4.00	\$3,752.00
	Widen Bench	6.00	EACH	96.00	16.00	\$1,894.52	\$11,367.12
	RH-Manhole Reconstruct Crew (2.00 EACH/DY, 3.00 DY)	6.00	EACH	96.00	16.00	\$1,894.52	\$11,367.12
	24-inch Trailer Mounted Blower	24.00	HR			\$51.25	\$1,230.00
	Backhoe Light	24.00	HR			\$86.11	\$2,066.64
	Confined Space Equipment	24.00	HR			\$15.63	\$375.12
	F-250 to F-450	24.00	HR			\$39.86	\$956.64
	185 Compressor	24.00	HR			\$51.69	\$1,240.56
	Jack Hammer	24.00	HR			\$8.00	\$192.00
	Foreman	24.00	HR			\$77.15	\$1,851.60
	Laborer [2]	24.00	HR			\$44.30	\$2,126.40
	Operator: Small Equipment	24.00	HR			\$55.34	\$1,328.16
	Water Support For CIPP Lining Steam Cure (938.00 LF/DY, 1.00 DY)	938.00	LF	12.00	0.01	\$1.47	\$1,375.54
	Water Truck Driver	12.00	HR			\$64.39	\$772.66
	Water Truck 4000Gal	8.00	HR			\$75.36	\$602.88
	Post Pano	938.00	LF	16.00	0.02	\$6.86	\$6,436.48
	Hydro Crew (938.00 LF/DY, 1.00 DY)	938.00	LF	16.00	0.02	\$2.86	\$2,684.48
	Combo Vactor Truck	8.00	HR			\$225.00	\$1,800.00
	Combo Truck Assistant	8.00	HR			\$48.46	\$387.68
	Combo Truck Operator	8.00	HR			\$62.10	\$496.80

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 28 - Install 30" CIPP Liner continued)</i>							
	Post Pano	938.00	LF			\$4.00	\$3,752.00
	29 - Install 48" CIPP Liner	2,019.00	LF	352.00	0.17	\$427.40	\$862,917.53
	CIPP Lining 48" W/ 1.5% Bond	2,019.00	LF			\$404.99	\$817,674.81
	RH-CIPP Lining Support Crew (168.25 LF/DY, 12.00 DY)	2,019.00	LF	192.00	0.10	\$7.67	\$15,485.76
	F-250 to F-450	96.00	HR			\$39.86	\$3,826.56
	Foreman	96.00	HR			\$77.15	\$7,406.40
	Laborer	96.00	HR			\$44.30	\$4,252.80
	Post Pano	2,019.00	LF	64.00	0.03	\$9.32	\$18,813.92
	Hydro Crew (504.75 LF/DY, 4.00 DY)	2,019.00	LF	64.00	0.03	\$5.32	\$10,737.92
	Combo Vactor Truck	32.00	HR			\$225.00	\$7,200.00
	Combo Truck Assistant	32.00	HR			\$48.46	\$1,550.72
	Combo Truck Operator	32.00	HR			\$62.10	\$1,987.20
	Post Pano	2,019.00	LF			\$4.00	\$8,076.00
	Setup & Remove Temp Water Source	1.00	LS	96.00	96.00	\$10,943.04	\$10,943.04
	RH-Bypass Fusion 12-18in Crew (0.33 LS/DY, 3.00 DY)	1.00	LS	96.00	96.00	\$10,943.04	\$10,943.04
	F-250 to F-450	24.00	HR			\$39.86	\$956.64
	Backhoe Light	24.00	HR			\$86.11	\$2,066.64
	Flatbed Trailers 12 to 18 Fort Trailer	24.00	HR			\$12.75	\$306.00
	Fusion Machine 6-8in	24.00	HR			\$64.65	\$1,551.60
	Pipe Trailer	24.00	HR			\$23.65	\$567.60
	Foreman	24.00	HR			\$77.15	\$1,851.60
	Fuser Technician	24.00	HR			\$52.15	\$1,251.60
	Laborer	24.00	HR			\$44.30	\$1,063.20
	Operator: Small Equipment	24.00	HR			\$55.34	\$1,328.16
	30 - Install 66" CIPP Liner	652.00	LF	240.00	0.37	\$1,036.46	\$675,768.80
	CIPP Lining 66" W/ 1.5% Bond	652.00	LF			\$995.72	\$649,209.44
	RH-CIPP Lining Support Crew (81.50 LF/DY, 8.00 DY)	652.00	LF	128.00	0.20	\$15.83	\$10,323.84
	F-250 to F-450	64.00	HR			\$39.86	\$2,551.04
	Foreman	64.00	HR			\$77.15	\$4,937.60
	Laborer	64.00	HR			\$44.30	\$2,835.20
	Post Pano	652.00	LF	16.00	0.02	\$8.12	\$5,292.48
	Hydro Crew (652.00 LF/DY, 1.00 DY)	652.00	LF	16.00	0.02	\$4.12	\$2,684.48
	Combo Vactor Truck	8.00	HR			\$225.00	\$1,800.00
	Combo Truck Assistant	8.00	HR			\$48.46	\$387.68
	Combo Truck Operator	8.00	HR			\$62.10	\$496.80
	Post Pano	652.00	LF			\$4.00	\$2,608.00
	Setup & Remove Temp Water Source	1.00	LS	96.00	96.00	\$10,943.04	\$10,943.04
	RH-Bypass Fusion 12-18in Crew (0.33 LS/DY, 3.00 DY)	1.00	LS	96.00	96.00	\$10,943.04	\$10,943.04
	F-250 to F-450	24.00	HR			\$39.86	\$956.64
	Backhoe Light	24.00	HR			\$86.11	\$2,066.64
	Flatbed Trailers 12 to 18 Fort Trailer	24.00	HR			\$12.75	\$306.00
	Fusion Machine 6-8in	24.00	HR			\$64.65	\$1,551.60
	Pipe Trailer	24.00	HR			\$23.65	\$567.60

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 30 - Install 66" CIPP Liner continued)</i>							
	Foreman	24.00	HR			\$77.15	\$1,851.60
	Fuser Technician	24.00	HR			\$52.15	\$1,251.60
	Laborer	24.00	HR			\$44.30	\$1,063.20
	Operator: Small Equipment	24.00	HR			\$55.34	\$1,328.16
D	31 - 48" East Bypass Pump Setup & Teardown	1.00	EACH	1,599.00	1,599.00	\$274,833.54	\$274,833.54
D	Mobilization/Demobilization Bypass Equipment	1.00	EACH	192.00	192.00	\$24,495.36	\$24,495.36
D	Mob Pipe	700.00	LF	12.00	0.02	\$2.19	\$1,530.96
	GC-Mobilization Crew (1,400.00 LF/DY, 0.50 DY)	700.00	LF	12.00	0.02	\$2.19	\$1,530.96
	Transport Truck Semi	4.00	HR			\$83.31	\$333.24
	Transport Trailer - 48 FT	4.00	HR			\$17.75	\$71.00
	Cat TL1255 (12K Telehandler Forklift)	4.00	HR			\$126.85	\$507.40
	Laborer	4.00	HR			\$44.30	\$177.20
	Operator: Small Equipment	4.00	HR			\$55.34	\$221.36
	Truck Driver	4.00	HR			\$55.19	\$220.76
D	Mob Pumps	3.00	EACH	72.00	24.00	\$3,061.92	\$9,185.76
	GC-Mobilization Crew (1.00 EACH/DY, 3.00 DY)	3.00	EACH	72.00	24.00	\$3,061.92	\$9,185.76
	Transport Truck Semi	24.00	HR			\$83.31	\$1,999.44
	Transport Trailer - 48 FT	24.00	HR			\$17.75	\$426.00
	Cat TL1255 (12K Telehandler Forklift)	24.00	HR			\$126.85	\$3,044.40
	Laborer	24.00	HR			\$44.30	\$1,063.20
	Operator: Small Equipment	24.00	HR			\$55.34	\$1,328.16
	Truck Driver	24.00	HR			\$55.19	\$1,324.56
D	Mob Appurtenances	1.00	EACH	12.00	12.00	\$1,530.96	\$1,530.96
	GC-Mobilization Crew (2.00 EACH/DY, 0.50 DY)	1.00	EACH	12.00	12.00	\$1,530.96	\$1,530.96
	Transport Truck Semi	4.00	HR			\$83.31	\$333.24
	Transport Trailer - 48 FT	4.00	HR			\$17.75	\$71.00
	Cat TL1255 (12K Telehandler Forklift)	4.00	HR			\$126.85	\$507.40
	Laborer	4.00	HR			\$44.30	\$177.20
	Operator: Small Equipment	4.00	HR			\$55.34	\$221.36
	Truck Driver	4.00	HR			\$55.19	\$220.76
D	Demob Pipe	700.00	LF	12.00	0.02	\$2.19	\$1,530.96
	GC-Mobilization Crew (1,400.00 LF/DY, 0.50 DY)	700.00	LF	12.00	0.02	\$2.19	\$1,530.96
	Transport Truck Semi	4.00	HR			\$83.31	\$333.24
	Transport Trailer - 48 FT	4.00	HR			\$17.75	\$71.00
	Cat TL1255 (12K Telehandler Forklift)	4.00	HR			\$126.85	\$507.40
	Laborer	4.00	HR			\$44.30	\$177.20
	Operator: Small Equipment	4.00	HR			\$55.34	\$221.36
	Truck Driver	4.00	HR			\$55.19	\$220.76
D	Demob Pumps	3.00	EACH	72.00	24.00	\$3,061.92	\$9,185.76
	GC-Mobilization Crew (1.00 EACH/DY, 3.00 DY)	3.00	EACH	72.00	24.00	\$3,061.92	\$9,185.76
	Transport Truck Semi	24.00	HR			\$83.31	\$1,999.44
	Transport Trailer - 48 FT	24.00	HR			\$17.75	\$426.00
	Cat TL1255 (12K Telehandler Forklift)	24.00	HR			\$126.85	\$3,044.40
	Laborer	24.00	HR			\$44.30	\$1,063.20
	Operator: Small Equipment	24.00	HR			\$55.34	\$1,328.16

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 31 - 48" East Bypass Pump Setup & Teardown continued)</i>							
 Truck Driver		24.00	HR			\$55.19	\$1,324.56
 Demob Appurtenances		1.00	EACH	12.00	12.00	\$1,530.96	\$1,530.96
 Mobe Appurtenances		1.00	UNIT	12.00	12.00	\$1,530.96	\$1,530.96
 GC-Mobilization Crew (2.00 UNIT/DY, 0.50 DY)		1.00	UNIT	12.00	12.00	\$1,530.96	\$1,530.96
 Transport Truck Semi		4.00	HR			\$83.31	\$333.24
 Transport Trailer - 48 FT		4.00	HR			\$17.75	\$71.00
 Cat TL1255 (12K Telehandler Forklift)		4.00	HR			\$126.85	\$507.40
 Laborer		4.00	HR			\$44.30	\$177.20
 Operator: Small Equipment		4.00	HR			\$55.34	\$221.36
 Truck Driver		4.00	HR			\$55.19	\$220.76
 Bypass Pump Station Setup & Removal		1.00	EACH	1,407.00	1,407.00	\$250,338.18	\$250,338.18
 Cut Coupons		1.00	EACH			\$2,800.00	\$2,800.00
 Bypass Pump Setup & Removal		3.00	EACH	904.00	301.33	\$58,041.34	\$174,124.01
 Bypass Pump Setup		3.00	EACH	240.00	80.00	\$11,125.97	\$33,377.92
 Crane To Setup Pump		2.00	DY			\$5,600.00	\$11,200.00
 RH-Bypass Sewer Setup Crew (0.50 EACH/DY, 6.00 DY)		3.00	EACH	240.00	80.00	\$7,392.64	\$22,177.92
 F-250 to F-450		48.00	HR			\$39.86	\$1,913.28
 Cat TL1055 (10K Telehandler Forklift)		48.00	HR			\$112.65	\$5,407.20
 Data Logger		48.00	HR			\$12.50	\$600.00
 Flatbed Trailers 12 to 18 Fort Trailer		48.00	HR			\$12.75	\$612.00
 Foreman		48.00	HR			\$77.15	\$3,703.20
 Fuser Technician		48.00	HR			\$52.15	\$2,503.20
 Laborer		48.00	HR			\$44.30	\$2,126.40
 Operator: Small Equipment [2]		48.00	HR			\$55.34	\$5,312.64
 Excavation Suction Pit & Subcut For Pump Station		1.00	EACH	192.00	192.00	\$25,341.28	\$25,341.28
 Excavate Suction Pits (0.33 EACH/DY, 3.00 DY)		1.00	EACH	192.00	192.00	\$22,751.28	\$22,751.28
 Cat 352 (Weight 110K)		24.00	HR			\$190.54	\$4,572.96
 Cat 950		24.00	HR			\$104.46	\$2,507.04
 Dump Truck - 10 Wheel		24.00	HR			\$88.40	\$2,121.60
 F-250 to F-450		24.00	HR			\$39.86	\$956.64
 Water Truck 4000Gal		24.00	HR			\$75.36	\$1,808.64
 Foreman		24.00	HR			\$77.15	\$1,851.60
 Laborer [2]		24.00	HR			\$44.30	\$2,126.40
 Operator: Large Equipment [2]		24.00	HR			\$60.68	\$2,912.64
 Truck Driver		24.00	HR			\$55.19	\$1,324.56
 Water Truck Driver		24.00	HR			\$55.19	\$1,324.56
 Leadman		24.00	HR			\$51.86	\$1,244.64
 Trucking (4.75 DY, 0.37 Trucks, 0.18 EACH/HR)		7.00	EACH			\$270.00	\$1,890.00
 DUMP FEES		7.00	LOAD			\$100.00	\$700.00
 Pour Concrete In Suction Pit		1.00	EACH	40.00	40.00	\$5,718.20	\$5,718.20
 SL-Manhole Coated 7ft Diameter Appurtenance Crew (1.00 EACH/DY, 1.00 DY)		1.00	EACH	40.00	40.00	\$3,227.60	\$3,227.60
 F-250 to F-450		8.00	HR			\$39.86	\$318.88

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 31 - 48" East Bypass Pump Setup & Teardown continued)</i>							
 Backhoe Light		8.00	HR			\$86.11	\$688.88
 Foreman		8.00	HR			\$77.15	\$617.20
 Laborer [2]		8.00	HR			\$44.30	\$708.80
 Operator: Large Equipment		8.00	HR			\$60.68	\$485.44
 Pipelayer		8.00	HR			\$51.05	\$408.40
 Ready Mix Concrete - MAG C 2000		16.80	CY			\$148.25	\$2,490.60
 Repair T-Lock Coupons (Sonco Construction, Inc.)		1.00	EACH			\$11,000.00	\$11,000.00
 Excavation And Backfill Suction Pits		1.00	EACH	240.00	240.00	\$67,883.41	\$67,883.41
 Excavate Suction Pits (0.33 EACH/DY, 3.00 DY)		1.00	EACH	144.00	144.00	\$20,443.44	\$20,443.44
 Cat 352 (Weight 110K)		24.00	HR			\$190.54	\$4,572.96
 Cat 950		24.00	HR			\$104.46	\$2,507.04
 Dump Truck - 10 Wheel		24.00	HR			\$88.40	\$2,121.60
 F-250 to F-450		24.00	HR			\$39.86	\$956.64
 Water Truck 4000Gal		24.00	HR			\$75.36	\$1,808.64
 Foreman		24.00	HR			\$77.15	\$1,851.60
 Laborer		24.00	HR			\$44.30	\$1,063.20
 Operator: Large Equipment [2]		24.00	HR			\$60.68	\$2,912.64
 Truck Driver		24.00	HR			\$55.19	\$1,324.56
 Water Truck Driver		24.00	HR			\$55.19	\$1,324.56
 1/2 Sack Slurry		231.00	CY			\$103.51	\$23,910.81
 Backfill Pits Remove CMP (0.50 EACH/DY, 2.00 DY)		1.00	EACH	64.00	64.00	\$7,309.28	\$7,309.28
 Cat 352 (Weight 110K)		16.00	HR			\$190.54	\$3,048.64
 F-250 to F-450		16.00	HR			\$39.86	\$637.76
 Foreman		16.00	HR			\$77.15	\$1,234.40
 Laborer [2]		16.00	HR			\$44.30	\$1,417.60
 Operator: Large Equipment		16.00	HR			\$60.68	\$970.88
 Buy CMP 10"		1.00	EACH			\$4,560.92	\$4,560.92
 GC-Combo Hydro Truck Crew (0.50 EACH/DY, 2.00 DY)		1.00	EACH	32.00	32.00	\$5,368.96	\$5,368.96
 Combo Vactor Truck		16.00	HR			\$225.00	\$3,600.00
 Combo Truck Assistant		16.00	HR			\$48.46	\$775.36
 Combo Truck Operator		16.00	HR			\$62.10	\$993.60
 Trucking (4.75 DY, 0.89 Trucks, 0.45 LOAD/HR)		17.00	LOAD			\$270.00	\$4,590.00
 DUMP FEES		17.00	LOAD			\$100.00	\$1,700.00
 Remove Pump Setup		3.00	EACH	192.00	64.00	\$10,267.73	\$30,803.20
 RH-Bypass Sewer Setup Crew (0.50 EACH/DY, 6.00 DY)		3.00	EACH	192.00	64.00	\$6,534.40	\$19,603.20
 F-250 to F-450		48.00	HR			\$39.86	\$1,913.28
 Foreman		48.00	HR			\$77.15	\$3,703.20
 Fuser Technician		48.00	HR			\$52.15	\$2,503.20
 Laborer		48.00	HR			\$44.30	\$2,126.40
 Cat TL1255 (12K Telehandler Forklift)		48.00	HR			\$126.85	\$6,088.80
 Data Logger [0]		48.00	HR			\$12.50	\$0.00
 Flatbed Trailers 12 to 18 Fort Trailer		48.00	HR			\$12.75	\$612.00

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 31 - 48" East Bypass Pump Setup & Teardown continued)</i>							
 Operator: Small Equipment		48.00	HR			\$55.34	\$2,656.32
 Crane To Setup Pump		2.00	DY			\$5,600.00	\$11,200.00
 Fuse Pipe For 48" Bypass		700.00	LF	503.00	0.72	\$102.73	\$71,914.17
 18-inch Fusion		700.00	LF	35.00	0.05	\$6.24	\$4,367.65
 RH-Bypass Fusion 12-18in Crew (800.00 LF/DY, 0.88 DY)		700.00	LF	35.00	0.05	\$6.24	\$4,367.65
 F-250 to F-450		7.00	HR			\$39.86	\$279.02
 Cat TL1055 (10K Telehandler Forklift)		7.00	HR			\$112.65	\$788.55
 Pipe Trailer		7.00	HR			\$23.65	\$165.55
 Backhoe Light		7.00	HR			\$86.11	\$602.77
 Flatbed Trailers 12 to 18 Fort Trailer		7.00	HR			\$12.75	\$89.25
 Fusion Machine 6-8in		7.00	HR			\$64.65	\$452.55
 Foreman		7.00	HR			\$77.15	\$540.05
 Fuser Technician		7.00	HR			\$52.15	\$365.05
 Laborer		7.00	HR			\$44.30	\$310.10
 Operator: Small Equipment [2]		7.00	HR			\$55.34	\$774.76
 18" Misc. Fusion		8.00	EACH	40.00	5.00	\$629.29	\$5,034.32
 RH-Bypass Fusion 12-18in Crew (8.00 LF/DY, 1.00 DY)		8.00	LF	40.00	5.00	\$629.29	\$5,034.32
 F-250 to F-450		8.00	HR			\$39.86	\$318.88
 Cat TL1055 (10K Telehandler Forklift)		8.00	HR			\$112.65	\$901.20
 Pipe Trailer		8.00	HR			\$23.65	\$189.20
 Backhoe Light		8.00	HR			\$86.11	\$688.88
 Flatbed Trailers 12 to 18 Fort Trailer		8.00	HR			\$12.75	\$102.00
 Fusion Machine 6-8in		8.00	HR			\$64.65	\$517.20
 Foreman		8.00	HR			\$77.15	\$617.20
 Fuser Technician		8.00	HR			\$52.15	\$417.20
 Laborer		8.00	HR			\$44.30	\$354.40
 Operator: Small Equipment		8.00	HR			\$55.34	\$442.72
 Operator: Large Equipment		8.00	HR			\$60.68	\$485.44
 Testing		700.00	LF	12.00	0.02	\$2.03	\$1,419.48
 Pressure Test Crew (1,400.00 LF/DY, 0.50 DY)		700.00	LF	12.00	0.02	\$2.03	\$1,419.48
 Backhoe Light		4.00	HR			\$86.11	\$344.44
 F-250 to F-450		4.00	HR			\$39.86	\$159.44
 Hydro Static Test Trailer		4.00	HR			\$45.36	\$181.44
 Foreman		4.00	HR			\$77.15	\$308.60
 Operator: Small Equipment		4.00	HR			\$55.34	\$221.36
 Pipelayer		4.00	HR			\$51.05	\$204.20
 Remove Bypass Pump Station		1.00	EACH	160.00	160.00	\$28,260.48	\$28,260.48
 Crane To Remove Pumps		2.00	DY			\$5,600.00	\$11,200.00
 RH-Bypass Sewer Takedown Crew (0.25 EACH/DY, 4.00 DY)		1.00	EACH	160.00	160.00	\$17,060.48	\$17,060.48
 Backhoe Light		32.00	HR			\$86.11	\$2,755.52
 Cat TL1055 (10K Telehandler Forklift)		32.00	HR			\$112.65	\$3,604.80
 F-250 to F-450		32.00	HR			\$39.86	\$1,275.52
 Flatbed Trailers 12 to 18 Fort Trailer		32.00	HR			\$12.75	\$408.00
 Foreman		32.00	HR			\$77.15	\$2,468.80

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 31 - 48" East Bypass Pump Setup & Teardown continued)</i>							
	Laborer [2]	32.00	HR			\$44.30	\$2,835.20
	Operator: Large Equipment	32.00	HR			\$60.68	\$1,941.76
	Operator: Small Equipment	32.00	HR			\$55.34	\$1,770.88
	Bypass Pipe Removal	700.00	LF	256.00	0.37	\$46.90	\$32,832.24
	Flush Pipe & Pigging Crew	1,400.00	LF	136.00	0.10	\$10.56	\$14,784.80
	Chlorine And Dechlorine	1.00	LS			\$1,200.00	\$1,200.00
	Flush & Chlorinate Discharge Piping (1,400.00 LF/DY, 1.00 DY)	1,400.00	LF	40.00	0.03	\$3.03	\$4,237.76
	Backhoe Light	8.00	HR			\$86.11	\$688.88
	F-250 to F-450	8.00	HR			\$39.86	\$318.88
	Hydro Static Test Trailer	8.00	HR			\$45.36	\$362.88
	Water Truck 4000Gal	8.00	HR			\$75.36	\$602.88
	Foreman	8.00	HR			\$77.15	\$617.20
	Operator: Small Equipment	8.00	HR			\$55.34	\$442.72
	Pipelay	8.00	HR			\$51.05	\$408.40
	Laborer	8.00	HR			\$44.30	\$354.40
	Water Truck Driver	8.00	HR			\$55.19	\$441.52
	RH-Bypass Sewer Takedown Crew (700.00 LF/DY, 2.00 DY)	1,400.00	LF	96.00	0.07	\$6.68	\$9,347.04
	Backhoe Light	16.00	HR			\$86.11	\$1,377.76
	Cat TL1055 (10K Telehandler Forklift)	16.00	HR			\$112.65	\$1,802.40
	F-250 to F-450	16.00	HR			\$39.86	\$637.76
	Flatbed Trailers 12 to 18 Fort Trailer	16.00	HR			\$12.75	\$204.00
	Foreman	16.00	HR			\$77.15	\$1,234.40
	Laborer [2]	16.00	HR			\$44.30	\$1,417.60
	Operator: Large Equipment	16.00	HR			\$60.68	\$970.88
	Operator: Small Equipment	16.00	HR			\$55.34	\$885.44
	Pipelay	16.00	HR			\$51.05	\$816.80
	Remove And Replace Discharge Manhole	1.00	EACH	120.00	120.00	\$18,047.44	\$18,047.44
	RH- Bypass Sewer Takedown Crew (0.33 EACH/DY, 3.00 DY)	1.00	EACH	120.00	120.00	\$17,497.44	\$17,497.44
	Backhoe Light	24.00	HR			\$86.11	\$2,066.64
	F-250 to F-450	24.00	HR			\$39.86	\$956.64
	Flatbed Trailers 12 to 18 Fort Trailer	24.00	HR			\$12.75	\$306.00
	Foreman	24.00	HR			\$77.15	\$1,851.60
	Laborer [2]	24.00	HR			\$44.30	\$2,126.40
	Operator: Small Equipment	24.00	HR			\$55.34	\$1,328.16
	Cat TL1255 (12K Telehandler Forklift)	24.00	HR			\$126.85	\$3,044.40
	Cat 352 (Weight 110K)	24.00	HR			\$190.54	\$4,572.96
	Leadman	24.00	HR			\$51.86	\$1,244.64
	Buy Manhole Section	1.00	EACH			\$550.00	\$550.00
	Bypass Engineering Stamp	1.00	EACH			\$1,500.00	\$1,500.00
	32 - 48" East Bypass Pump Operation	289.00	DY	13,872.00	48.00	\$10,884.80	\$3,145,708.22
	Rental Equipment And Fuel	289.00	DY	0.00	0.00	\$7,683.60	\$2,220,561.42
	Pump - 12in Pump [3]	2,312.00	HR			\$59.75	\$414,426.00
	Box Trailer Pump Watch	2,312.00	HR			\$12.75	\$29,478.00
	18-inch HDPE (LF/MO) [700]	13.44	MO			\$1.25	\$11,761.63

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
(Item 32 - 48" East Bypass Pump Operation continued)							
 Fuel [170]		2,312.00	HR			\$4.00	\$1,572,160.00
 Spill Kits		1.00	EACH			\$850.00	\$850.00
 Fuel Cell 500 Gallons		13.44	MO			\$500.00	\$6,720.93
 18-inch 45 OR 90 Degree Elbow [5]		13.44	MO			\$86.00	\$5,780.00
 18-inch Flange Adapters With Backer Ring [8]		289.00	DY			\$1.22	\$2,820.64
 18-inch Gate Valve [4]		13.44	MO			\$750.00	\$40,325.58
 Large Manifold		13.44	MO			\$2,650.00	\$35,620.93
 24-inch Suction Pipe [3]		13.44	MO			\$1,250.00	\$50,406.98
 24-inch Suction 45-90 Degree DR17 [3]		13.44	MO			\$48.90	\$1,971.92
 24-inch Flange Adapters With Backer Ring [3]		13.44	MO			\$43.50	\$1,754.16
 Spill Containment [4]		13.44	MO			\$195.00	\$10,484.65
 42-78 Inch Plug [0]		2,325.44	HR			\$19.88	\$0.00
 Purchase Plugs		2.00	EACH			\$18,000.00	\$36,000.00
 Pump Watch And Equipment		289.00	DY	13,872.00	48.00	\$3,201.20	\$925,146.80
 RH-Bypass Sewer Pump Watch Crew (1.00 DY/DY, 289.00 DY)		289.00	DY	13,872.00	48.00	\$3,171.20	\$916,476.80
 F-250 to F-450		2,312.00	HR			\$39.86	\$92,156.32
 Light Tower [2]		2,312.00	HR			\$12.25	\$56,644.00
 Operator: Small Equipment [6]		2,312.00	HR			\$55.34	\$767,676.48
 Consumables		289.00	DY			\$30.00	\$8,670.00
 33 - Furnish & Install 84" Polymer Concrete Sewer Manhole On CIP Base		1.00	EACH	968.00	968.00	\$278,133.06	\$278,133.06
 Install New 60" In Line MH With Cast-In-Place Base		1.00	EACH	968.00	968.00	\$278,133.06	\$278,133.06
 Manhole Crew (0.10 EACH/DY, 10.00 DY)		1.00	EACH	640.00	640.00	\$91,807.20	\$91,807.20
 F-250 to F-450		80.00	HR			\$39.86	\$3,188.80
 Cat 950		80.00	HR			\$104.46	\$8,356.80
 Excavator - 160 K LB		80.00	HR			\$304.86	\$24,388.80
 Backhoe Light		80.00	HR			\$86.11	\$6,888.80
 Dump Truck - 10 Wheel		80.00	HR			\$88.40	\$7,072.00
 Water Truck 4000Gal		80.00	HR			\$75.36	\$6,028.80
 Foreman		80.00	HR			\$77.15	\$6,172.00
 Operator: Large Equipment [2]		80.00	HR			\$60.68	\$9,708.80
 Pipelayer		80.00	HR			\$51.05	\$4,084.00
 Laborer [2]		80.00	HR			\$44.30	\$7,088.00
 Truck Driver		80.00	HR			\$55.19	\$4,415.20
 Water Truck Driver		80.00	HR			\$55.19	\$4,415.20
 Aggregate - ABC MAG Spec		19.95	TON			\$20.00	\$399.00
 1/2 Sack Slurry		176.40	CY			\$103.51	\$18,259.16
 DUMP FEES		21.00	LOAD			\$75.00	\$1,575.00
 Trucking (6.25 DY, 0.84 Trucks, 0.42 EACH/HR)		21.00	EACH			\$270.00	\$5,670.00
 Concrete Cast In Place Bench		1.00	EACH	280.00	280.00	\$36,910.76	\$36,910.76
 CIP Manhole Crew (0.20 EACH/DY, 5.00 DY)		1.00	EACH	280.00	280.00	\$32,981.20	\$32,981.20
 F-250 to F-450		40.00	HR			\$39.86	\$1,594.40
 Backhoe Light		40.00	HR			\$86.11	\$3,444.40
 Excavator - 160 K LB		40.00	HR			\$304.86	\$12,194.40
 Foreman		40.00	HR			\$77.15	\$3,086.00
 Operator: Large Equipment		40.00	HR			\$60.68	\$2,427.20

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 33 - Furnish & Install 84" Polymer Concrete Sewer Manhole On CIP Base continued)</i>							
 Operator: Small Equipment		40.00	HR			\$55.34	\$2,213.60
 Pipelayer		40.00	HR			\$51.05	\$2,042.00
 Laborer		40.00	HR			\$44.30	\$1,772.00
 Concrete Finisher [2]		40.00	HR			\$52.59	\$4,207.20
 Forms / Rebar Cure		1.00	EACH			\$2,568.00	\$2,568.00
 CONCRETE - MAG A (3000 PSI)		9.45	CY			\$144.08	\$1,361.56
 Hydro Excavate Around Pipe		1.00	EACH	32.00	32.00	\$5,368.96	\$5,368.96
 GC-Combo Hydro Truck Crew (0.50 EACH/DY, 2.00 DY)		1.00	EACH	32.00	32.00	\$5,368.96	\$5,368.96
 Combo Vactor Truck		16.00	HR			\$225.00	\$3,600.00
 Combo Truck Operator		16.00	HR			\$62.10	\$993.60
 Combo Truck Assistant		16.00	HR			\$48.46	\$775.36
 FRP Bench Treatment		1.00	EACH			\$34,000.00	\$34,000.00
 Manhole - Polymer Manhole -84" Stackout		1.00	UNIT			\$54,580.00	\$54,580.00
 Shoring		20.00	DY			\$1,250.00	\$25,000.00
 MH Adjustment		1.00	EACH	16.00	16.00	\$1,762.98	\$1,762.98
 RH-Manhole Raised Crew (2.00 EACH/DY, 0.50 DY)		1.00	EACH	16.00	16.00	\$1,460.60	\$1,460.60
 F-250 to F-450		4.00	HR			\$39.86	\$159.44
 Backhoe Light		4.00	HR			\$86.11	\$344.44
 Flatbed Trailers 12 to 18 Fort Trailer		4.00	HR			\$12.75	\$51.00
 Foreman		4.00	HR			\$77.15	\$308.60
 Laborer [2]		4.00	HR			\$44.30	\$354.40
 Operator: Large Equipment		4.00	HR			\$60.68	\$242.72
 CONCRETE - MAG AA (4000 PSI)		2.00	CY			\$151.19	\$302.38
 Cut Coupons		1.00	EACH			\$2,800.00	\$2,800.00
 34 - 48" West Bypass Pump Setup & Teardown		1.00	EACH	1,661.10	1,661.10	\$238,901.81	\$238,901.81
 Mobilization/Demobilizaion Bypass Equipment		1.00	EACH	336.00	336.00	\$42,866.88	\$42,866.88
 Mob Pipe		2,550.00	LF	48.00	0.02	\$2.40	\$6,123.84
 GC-Mobilization Crew (1,275.00 LF/DY, 2.00 DY)		2,550.00	LF	48.00	0.02	\$2.40	\$6,123.84
 Transport Truck Semi		16.00	HR			\$83.31	\$1,332.96
 Transport Trailer - 48 FT		16.00	HR			\$17.75	\$284.00
 Cat TL1255 (12K Telehandler Forklift)		16.00	HR			\$126.85	\$2,029.60
 Laborer		16.00	HR			\$44.30	\$708.80
 Operator: Small Equipment		16.00	HR			\$55.34	\$885.44
 Truck Driver		16.00	HR			\$55.19	\$883.04
 Mob Pumps		3.00	EACH	72.00	24.00	\$3,061.92	\$9,185.76
 GC-Mobilization Crew (1.00 EACH/DY, 3.00 DY)		3.00	EACH	72.00	24.00	\$3,061.92	\$9,185.76
 Transport Truck Semi		24.00	HR			\$83.31	\$1,999.44
 Transport Trailer - 48 FT		24.00	HR			\$17.75	\$426.00
 Cat TL1255 (12K Telehandler Forklift)		24.00	HR			\$126.85	\$3,044.40
 Laborer		24.00	HR			\$44.30	\$1,063.20
 Operator: Small Equipment		24.00	HR			\$55.34	\$1,328.16
 Truck Driver		24.00	HR			\$55.19	\$1,324.56
 Mob Appurtenances		1.00	EACH	48.00	48.00	\$6,123.84	\$6,123.84
 GC-Mobilization Crew (0.50 EACH/DY, 2.00 DY)		1.00	EACH	48.00	48.00	\$6,123.84	\$6,123.84

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 34 - 48" West Bypass Pump Setup & Teardown continued)</i>							
 Transport Truck Semi		16.00	HR			\$83.31	\$1,332.96
 Transport Trailer - 48 FT		16.00	HR			\$17.75	\$284.00
 Cat TL1255 (12K Telehandler Forklift)		16.00	HR			\$126.85	\$2,029.60
 Laborer		16.00	HR			\$44.30	\$708.80
 Operator: Small Equipment		16.00	HR			\$55.34	\$885.44
 Truck Driver		16.00	HR			\$55.19	\$883.04
 Demob Pipe		2,550.00	LF	48.00	0.02	\$2.40	\$6,123.84
 GC-Mobilization Crew (1,275.00 LF/DY, 2.00 DY)		2,550.00	LF	48.00	0.02	\$2.40	\$6,123.84
 Transport Truck Semi		16.00	HR			\$83.31	\$1,332.96
 Transport Trailer - 48 FT		16.00	HR			\$17.75	\$284.00
 Cat TL1255 (12K Telehandler Forklift)		16.00	HR			\$126.85	\$2,029.60
 Laborer		16.00	HR			\$44.30	\$708.80
 Operator: Small Equipment		16.00	HR			\$55.34	\$885.44
 Truck Driver		16.00	HR			\$55.19	\$883.04
 Demob Pumps		3.00	EACH	72.00	24.00	\$3,061.92	\$9,185.76
 GC-Mobilization Crew (1.00 EACH/DY, 3.00 DY)		3.00	EACH	72.00	24.00	\$3,061.92	\$9,185.76
 Transport Truck Semi		24.00	HR			\$83.31	\$1,999.44
 Transport Trailer - 48 FT		24.00	HR			\$17.75	\$426.00
 Cat TL1255 (12K Telehandler Forklift)		24.00	HR			\$126.85	\$3,044.40
 Laborer		24.00	HR			\$44.30	\$1,063.20
 Operator: Small Equipment		24.00	HR			\$55.34	\$1,328.16
 Truck Driver		24.00	HR			\$55.19	\$1,324.56
 Demob Appurtenances		1.00	EACH	48.00	48.00	\$6,123.84	\$6,123.84
 Mobe Appurtenances		2.00	UNIT	48.00	24.00	\$3,061.92	\$6,123.84
 GC-Mobilization Crew (1.00 UNIT/DY, 2.00 DY)		2.00	UNIT	48.00	24.00	\$3,061.92	\$6,123.84
 Transport Truck Semi		16.00	HR			\$83.31	\$1,332.96
 Transport Trailer - 48 FT		16.00	HR			\$17.75	\$284.00
 Cat TL1255 (12K Telehandler Forklift)		16.00	HR			\$126.85	\$2,029.60
 Laborer		16.00	HR			\$44.30	\$708.80
 Operator: Small Equipment		16.00	HR			\$55.34	\$885.44
 Truck Driver		16.00	HR			\$55.19	\$883.04
 Bypass Pump Station Setup		1.00	EACH	1,085.10	1,085.10	\$152,105.25	\$152,105.25
 Cut Coupons		1.00	EACH			\$2,800.00	\$2,800.00
 Bypass Pump Setup		3.00	EACH	240.00	80.00	\$11,125.97	\$33,377.92
 Crane To Setup Pump		2.00	DY			\$5,600.00	\$11,200.00
 RH-Bypass Sewer Setup Crew (0.50 EACH/DY, 6.00 DY)		3.00	EACH	240.00	80.00	\$7,392.64	\$22,177.92
 F-250 to F-450		48.00	HR			\$39.86	\$1,913.28
 Cat TL1055 (10K Telehandler Forklift)		48.00	HR			\$112.65	\$5,407.20
 Data Logger		48.00	HR			\$12.50	\$600.00
 Flatbed Trailers 12 to 18 Fort Trailer		48.00	HR			\$12.75	\$612.00
 Foreman		48.00	HR			\$77.15	\$3,703.20
 Fuser Technician		48.00	HR			\$52.15	\$2,503.20
 Laborer		48.00	HR			\$44.30	\$2,126.40
 Operator: Small Equipment [2]		48.00	HR			\$55.34	\$5,312.64

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 34 - 48" West Bypass Pump Setup & Teardown continued)</i>							
 Fuse Pipe For 48" Bypass		2,550.00	LF	653.10	0.26	\$33.38	\$85,124.13
 18-inch Fusion		2,550.00	LF	102.00	0.04	\$5.69	\$14,499.56
 RH-Bypass Fusion 12-18in Crew (800.00 LF/DY, 3.19 DY)		2,550.00	LF	102.00	0.04	\$5.69	\$14,499.56
 F-250 to F-450		25.50	HR			\$39.86	\$1,016.43
 Backhoe Light		25.50	HR			\$86.11	\$2,195.81
 Cat TL1055 (10K Telehandler Forklift)		25.50	HR			\$112.65	\$2,872.58
 Flatbed Trailers 12 to 18 Fort Trailer		25.50	HR			\$12.75	\$325.13
 Fusion Machine 6-8in		25.50	HR			\$64.65	\$1,648.58
 Pipe Trailer		25.50	HR			\$23.65	\$603.08
 Foreman		25.50	HR			\$77.15	\$1,967.33
 Fuser Technician		25.50	HR			\$52.15	\$1,329.83
 Laborer		25.50	HR			\$44.30	\$1,129.65
 Operator: Small Equipment		25.50	HR			\$55.34	\$1,411.17
 18" Misc. Fusion		8.00	EACH	32.00	4.00	\$568.61	\$4,548.88
 RH-Bypass Fusion 12-18in Crew (8.00 LF/DY, 1.00 DY)		8.00	LF	32.00	4.00	\$568.61	\$4,548.88
 F-250 to F-450		8.00	HR			\$39.86	\$318.88
 Backhoe Light		8.00	HR			\$86.11	\$688.88
 Cat TL1055 (10K Telehandler Forklift)		8.00	HR			\$112.65	\$901.20
 Flatbed Trailers 12 to 18 Fort Trailer		8.00	HR			\$12.75	\$102.00
 Fusion Machine 6-8in		8.00	HR			\$64.65	\$517.20
 Pipe Trailer		8.00	HR			\$23.65	\$189.20
 Foreman		8.00	HR			\$77.15	\$617.20
 Fuser Technician		8.00	HR			\$52.15	\$417.20
 Laborer		8.00	HR			\$44.30	\$354.40
 Operator: Small Equipment		8.00	HR			\$55.34	\$442.72
 Testing		2,550.00	LF	22.70	0.01	\$1.05	\$2,684.59
 Pressure Test Crew (2,696.63 LF/DY, 0.95 DY)		2,550.00	LF	22.70	0.01	\$1.05	\$2,684.59
 Backhoe Light		7.57	HR			\$86.11	\$651.42
 F-250 to F-450		7.57	HR			\$39.86	\$301.54
 Hydro Static Test Trailer		7.57	HR			\$45.36	\$343.15
 Foreman		7.57	HR			\$77.15	\$583.64
 Operator: Small Equipment		7.57	HR			\$55.34	\$418.65
 Pipelayer		7.57	HR			\$51.05	\$386.19
 Remove Bypass Pump Station		1.00	EACH	160.00	160.00	\$28,260.48	\$28,260.48
 Crane To Remove Pumps		2.00	DY			\$5,600.00	\$11,200.00
 RH-Bypass Sewer Takedown Crew (0.25 EACH/DY, 4.00 DY)		1.00	EACH	160.00	160.00	\$17,060.48	\$17,060.48
 Backhoe Light		32.00	HR			\$86.11	\$2,755.52
 Cat TL1055 (10K Telehandler Forklift)		32.00	HR			\$112.65	\$3,604.80
 F-250 to F-450		32.00	HR			\$39.86	\$1,275.52
 Flatbed Trailers 12 to 18 Fort Trailer		32.00	HR			\$12.75	\$408.00
 Foreman		32.00	HR			\$77.15	\$2,468.80
 Laborer [2]		32.00	HR			\$44.30	\$2,835.20
 Operator: Large Equipment		32.00	HR			\$60.68	\$1,941.76
 Operator: Small Equipment		32.00	HR			\$55.34	\$1,770.88

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 34 - 48" West Bypass Pump Setup & Teardown continued)</i>							
 Bypass Pipe Removal		2,550.00	LF	336.40	0.13	\$13.78	\$35,130.62
 Flush Pipe & Pigging Crew		4,800.00	LF	336.40	0.07	\$7.32	\$35,130.62
 Chlorine And Dechlorine		1.00	LS			\$1,200.00	\$1,200.00
 Flush And Chlorinate Pipe (1,399.42 LF/DY, 3.43 DY)		4,800.00	LF	137.20	0.03	\$3.03	\$14,535.52
 Backhoe Light		27.44	HR			\$86.11	\$2,362.86
 F-250 to F-450		27.44	HR			\$39.86	\$1,093.76
 Hydro Static Test Trailer		27.44	HR			\$45.36	\$1,244.68
 Water Truck 4000Gal		27.44	HR			\$75.36	\$2,067.88
 Foreman		27.44	HR			\$77.15	\$2,117.00
 Operator: Small Equipment		27.44	HR			\$55.34	\$1,518.53
 Pipelayer		27.44	HR			\$51.05	\$1,400.81
 Laborer		27.44	HR			\$44.30	\$1,215.59
 Water Truck Driver		27.44	HR			\$55.19	\$1,514.41
 RH-Bypass Sewer Takedown Crew (1,156.63 LF/DY, 4.15 DY)		4,800.00	LF	199.20	0.04	\$4.04	\$19,395.11
 Backhoe Light		33.20	HR			\$86.11	\$2,858.85
 Cat TL1055 (10K Telehandler Forklift)		33.20	HR			\$112.65	\$3,739.98
 F-250 to F-450		33.20	HR			\$39.86	\$1,323.35
 Flatbed Trailers 12 to 18 Fort Trailer		33.20	HR			\$12.75	\$423.30
 Foreman		33.20	HR			\$77.15	\$2,561.38
 Laborer [2]		33.20	HR			\$44.30	\$2,941.52
 Operator: Large Equipment		33.20	HR			\$60.68	\$2,014.58
 Operator: Small Equipment		33.20	HR			\$55.34	\$1,837.29
 Pipelayer		33.20	HR			\$51.05	\$1,694.86
 Remove Pump Setup		3.00	EACH	192.00	64.00	\$10,267.73	\$30,803.20
 RH-Bypass Sewer Setup Crew (0.50 EACH/DY, 6.00 DY)		3.00	EACH	192.00	64.00	\$6,534.40	\$19,603.20
 F-250 to F-450		48.00	HR			\$39.86	\$1,913.28
 Foreman		48.00	HR			\$77.15	\$3,703.20
 Fuser Technician		48.00	HR			\$52.15	\$2,503.20
 Laborer		48.00	HR			\$44.30	\$2,126.40
 Cat TL1255 (12K Telehandler Forklift)		48.00	HR			\$126.85	\$6,088.80
 Data Logger [0]		48.00	HR			\$12.50	\$0.00
 Flatbed Trailers 12 to 18 Fort Trailer		48.00	HR			\$12.75	\$612.00
 Operator: Small Equipment		48.00	HR			\$55.34	\$2,656.32
 Crane To Setup Pump		2.00	DY			\$5,600.00	\$11,200.00
 Bypass Engineering Stamp		1.00	EACH			\$1,500.00	\$1,500.00
 Excavation/Subcut For Pump Station		1.00	EACH	128.00	128.00	\$18,833.68	\$18,833.68
 Subcut Crew (0.50 EACH/DY, 2.00 DY)		1.00	EACH	128.00	128.00	\$16,983.68	\$16,983.68
 F-250 to F-450		16.00	HR			\$39.86	\$637.76
 Excavator - 160 K LB		16.00	HR			\$304.86	\$4,877.76
 Dump Truck - 10 Wheel		16.00	HR			\$88.40	\$1,414.40
 Water Truck 4000Gal		16.00	HR			\$75.36	\$1,205.76
 Cat 950		16.00	HR			\$104.46	\$1,671.36
 Foreman		16.00	HR			\$77.15	\$1,234.40
 Operator: Large Equipment [2]		16.00	HR			\$60.68	\$1,941.76

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 34 - 48" West Bypass Pump Setup & Teardown continued)</i>							
	Pipelayer	16.00	HR			\$51.05	\$816.80
	Laborer [2]	16.00	HR			\$44.30	\$1,417.60
	Truck Driver	16.00	HR			\$55.19	\$883.04
	Water Truck Driver	16.00	HR			\$55.19	\$883.04
	Trucking (1.00 DY, 1.25 Trucks, 0.63 EACH/HR)	5.00	EACH			\$270.00	\$1,350.00
	DUMP FEES	5.00	LOAD			\$100.00	\$500.00
	Backfill & Restore Subcut/Pump Station	1.00	EACH	112.00	112.00	\$13,546.00	\$13,546.00
	Subcut Crew (0.50 EACH/DY, 2.00 DY)	1.00	EACH	112.00	112.00	\$11,824.00	\$11,824.00
	F-250 to F-450	16.00	HR			\$39.86	\$637.76
	Dump Truck - 10 Wheel	16.00	HR			\$88.40	\$1,414.40
	Water Truck 4000Gal	16.00	HR			\$75.36	\$1,205.76
	Cat 950	16.00	HR			\$104.46	\$1,671.36
	Cat CB24 (Roller)	16.00	HR			\$32.02	\$512.32
	Foreman	16.00	HR			\$77.15	\$1,234.40
	Operator: Large Equipment	16.00	HR			\$60.68	\$970.88
	Pipelayer	16.00	HR			\$51.05	\$816.80
	Laborer	16.00	HR			\$44.30	\$708.80
	Truck Driver	16.00	HR			\$55.19	\$883.04
	Water Truck Driver	16.00	HR			\$55.19	\$883.04
	Operator: Small Equipment	16.00	HR			\$55.34	\$885.44
	Aggregate - ABC MAG Spec	86.10	TON			\$20.00	\$1,722.00
	Paving Subcontractor	134.00	SY			\$75.00	\$10,050.00
	35 - 48" West Bypass Pump Operation	122.00	DY	5,856.00	48.00	\$17,136.73	\$2,090,680.70
	Rental Equipment And Fuel	122.00	DY	0.00	0.00	\$13,935.53	\$1,700,134.30
	12-Inch Standard Pump [3]	1,096.00	HR			\$52.82	\$173,672.16
	Box Trailer Pump Watch	1,096.00	HR			\$12.75	\$13,974.00
	18-inch HDPE (LF/MO) [2,550]	6.37	MO			\$1.25	\$20,311.05
	Fuel [313]	1,096.00	HR			\$4.00	\$1,372,192.00
	Spill Kits	1.00	EACH			\$850.00	\$850.00
	2-inch ARV And Containment	6.37	MO			\$195.00	\$1,242.56
	Fuel Cell 500 Gallons	6.37	MO			\$500.00	\$3,186.05
	18-inch 45 OR 90 Degree Elbow	6.37	MO			\$86.00	\$548.00
	18-inch Flange Adapters With Backer Ring	137.00	DY			\$1.22	\$167.14
	18-inch Gate Valve	6.37	MO			\$750.00	\$4,779.07
	Large Manifold	6.37	MO			\$2,650.00	\$16,886.05
	24-inch Flange Adapters With Backer Ring	6.37	MO			\$43.50	\$277.19
	24-inch Suction 45-90 Degree DR17	6.37	MO			\$48.90	\$311.60
	24-inch Suction Pipe	6.37	MO			\$1,250.00	\$7,965.12
	Spill Containment	6.37	MO			\$195.00	\$1,242.56
	42-78 Inch Plug [0]	976.00	HR			\$19.88	\$0.00
	Purchase Plugs	2.00	EACH			\$18,000.00	\$36,000.00
	8-inch HDPE (LF/MO) [1,000]	5.67	MO			\$1.00	\$5,674.42
	6-Inch Centrifugal [2]	976.00	HR			\$12.78	\$24,946.56
	6-Inch High Pressure Hose (EA/MO) [2]	976.00	HR			\$1.18	\$2,303.36
	6-Inch Valve [2]	976.00	HR			\$1.72	\$3,357.44
	Small Manifold	976.00	HR			\$7.10	\$6,929.60

Description		Quantity	UM	Man Hours	Man Hours per Unit	Unit Direct Cost	Total Direct Cost
<i>(Item 35 - 48" West Bypass Pump Operation continued)</i>							
 4-12 Inch Plug [2]		976.00	HR			\$1.70	\$3,318.40
 Pump Watch And Equipment		122.00	DY	5,856.00	48.00	\$3,201.20	\$390,546.40
 RH-Bypass Sewer Pump Watch Crew (1.00 DY/DY, 122.00 DY)		122.00	DY	5,856.00	48.00	\$3,171.20	\$386,886.40
 F-250 to F-450		976.00	HR			\$39.86	\$38,903.36
 Light Tower [2]		976.00	HR			\$12.25	\$23,912.00
 Operator: Small Equipment [6]		976.00	HR			\$55.34	\$324,071.04
 Consumables		122.00	DY			\$30.00	\$3,660.00
 36 - General Conditions		15.00	MO	8,640.00	576.00	\$90,483.09	\$1,357,246.29
 Job Office Expense		15.00	MO			\$500.00	\$7,500.00
 Office Power		15.00	MO			\$500.00	\$7,500.00
 ST&S		53,962.76	MH			\$3.00	\$161,888.29
 Software/ Hardware / Devices		15.00	MO			\$250.00	\$3,750.00
 Water And Ice		15.00	MO			\$1,000.00	\$15,000.00
 Site Security		15.00	MO			\$3,000.00	\$45,000.00
 Project Management		15.00	MO	2,640.00	176.00	\$23,474.88	\$352,123.20
 Project Manager (0.05 MO/DY, 330.00 DY)		15.00	MO	2,640.00	176.00	\$23,474.88	\$352,123.20
 F-150		2,640.00	HR			\$28.36	\$74,870.40
 Project Manager		2,640.00	HR			\$105.02	\$277,252.80
 Project Engineer		15.00	MO	2,640.00	176.00	\$19,747.20	\$296,208.00
 Project Engineer (0.05 MO/DY, 330.00 DY)		15.00	MO	2,640.00	176.00	\$19,747.20	\$296,208.00
 F-150		2,640.00	HR			\$28.36	\$74,870.40
 Project Engineer		2,640.00	HR			\$83.84	\$221,337.60
 Superintendent (Full-Time)		15.00	MO	2,640.00	176.00	\$23,166.88	\$347,503.20
 Project Superintendent (0.05 MO/DY, 330.00 DY)		15.00	MO	2,640.00	176.00	\$23,166.88	\$347,503.20
 F-150		2,640.00	HR			\$28.36	\$74,870.40
 Project Superintendent		2,640.00	HR			\$103.27	\$272,632.80
 Bypass Superintendent (Half-Time)		9.00	MO	720.00	80.00	\$10,530.40	\$94,773.60
 Project Superintendent (0.10 MO/DY, 90.00 DY)		9.00	MO	720.00	80.00	\$10,530.40	\$94,773.60
 F-150		720.00	HR			\$28.36	\$20,419.20
 Project Superintendent		720.00	HR			\$103.27	\$74,354.40
 Dumpster Service		15.00	MO			\$500.00	\$7,500.00
 Portable Restroom		15.00	MO			\$1,000.00	\$15,000.00
 Armorock QA/QC Travel Expenses		1.00	LS			\$3,500.00	\$3,500.00

Exhibit D4: List of GMP Documents
LOOP 101 AND 202 FREEWAY
66-INCH REDUNDANT SEWER PROJECT
COC PROJECT No.: WW2402.401
GURANTEDD MAXIMUM PRICE

DOCUMENTS:

- **TECHNICAL SPECIFICATIONS:** CITY OF CHANDLER, ARIZONA – LOOP 101 AND 202 FREEWAY 66-INCH REDUNDANT SEWER PROJECT, TECHNICAL SPECIFICATIONS, MAY 2025
- **PROJECT DRAWINGS:** CITY OF CHANDLER, ARIZONA – LOOP 101 AND 202 FREEWAY 66-INCH REDUNDANT SEWER PROJECT, CITY PROJECT NO. WW2402-401, JUNE 2025

EXHIBIT E

SUBCONTRACTOR DOCUMENTS WITH CM@RISK

Any subcontractor assumptions, clarifications, exclusions, terms & conditions, signature blocks, etc. included are strictly between the CM@RISK and their subcontractors, and do not apply to the Agreement between the CM@RISK and the City.

SUBCONTRACTOR'S LIST FORM

If CM@Risk intends to subcontract any portion of this Agreement, the CM@Risk must submit the name, address, and contractor's license number (if applicable) of each subcontractor, including the work component of such subcontracting. Include the form with the bid submittal documents. CM@Risk may make multiple copies of this form as needed.

Company Name:	
Contact Name:	
Contact Email:	
Contact Phone:	
Work Component:	
Percentage of Total Work Performed:	

Company Name:	
Contact Name:	
Contact Email:	
Contact Phone:	
Work Component:	
Percentage of Total Work Performed:	

Company Name:	
Contact Name:	
Contact Email:	
Contact Phone:	
Work Component:	
Percentage of Total Work Performed:	

Company Name:	
Contact Name:	
Contact Email:	
Contact Phone:	
Work Component:	
Percentage of Total Work Performed:	

Company Name:	
Contact Name:	
Contact Email:	
Contact Phone:	
Work Component:	
Percentage of Total Work Performed:	

Company Name:	
Contact Name:	
Contact Email:	
Contact Phone:	
Work Component:	
Percentage of Total Work Performed:	

EXHIBIT F

GIS / GPS DATA DELIVERY REQUIREMENTS

GIS / GPS Data Delivery Requirements

Contractor shall provide survey grade GPS / GIS data, meeting the following requirements for all facilities to be owned and/or operated by the City of Chandler.

a) ESRI File Geodatabase format:

All GPS point data, along with corresponding GIS Attribute data, must be submitted in ESRI File Geodatabase format. The City of Chandler will provide a copy of the File Geodatabase in ESRI ArcGIS format. Please submit your request through the Project Manager.

b) Data dictionary and Attribute data:

Attribute data should be provided for each of the GPS'd Utility Features listed in Section 'd' below. All Attribute data should conform to the ESRI File Geodatabase format, provided by the City. A copy of the data dictionary is also attached here for reference.

c) Coordinate System:

Horizontal Datum: Arizona State Plane Coordinates, Central Zone NAD83 (HARN)
Vertical Datum: NAVD88

d) Point Data for GPS:

1) Water System Features:

- Water main location (top of pipe), size and material (one (1) coordinate provided every 100 feet minimum) and at fittings.
- Water fitting location (top of feature), size, material and type including: Bend (22.5, 45, or 90), Tee, Tapping Sleeve, Cross, Coupling, End Cap, Line Stop, Reducer, Saddle, Other.
- Water valve location (center of valve box cover), size and type.
- Fire hydrant location (top of hydrant), manufacturer and year.
- Water service line location (at connection to main, any bends, and termination at meter box or meter vault or at the edge of easement or ROW), size and material.
- Water blow-off and air release valve location (center of cover), size, type and manufacturer.
- Water manhole or vault location (center of cover), size and type.

2) Waste Water System Features:

- Sewer manhole and cleanout location (center of cover), size, material, and cover type.
- Sewer gravity main location (invert of pipe), size, material and flow direction (from) at all manholes, cleanouts and structures.
- Sewer service line location, size and material (at connection to main, and termination at cleanout, or stub out at edge of the easement or ROW).
- Sewer force main location (top of pipe), size, and material (one (1) coordinate provided every 100 feet minimum) and at fittings.

- Sewer force main (and gravity) fitting location (top of feature), size, material and type including: Bend (22.5, 45, or 90), Tee, Tapping Sleeve, Cross, Coupling, End Cap, Linestop, Reducer, Saddle, Wye, Other
- Sewer force main valve location (center of valve box cover), size and type.
- Sewer force main air release valve location (center of cover), size, type and manufacturer.
- Sewer force main manhole or vault location (center of cover), size and material.
- Sewer structure (center of structure), type (diversion, junction box).

3) Reclaimed Water System Features:

- Reclaimed main location (top of pipe), size, and material (one (1) coordinate provided every 100 feet minimum) and at fittings.
- Reclaimed fitting location (top of feature), size, material and type including: Bend (22.5, 45, or 90), Tee, Tapping Sleeve, Cross, Coupling, End Cap, Linestop, Reducer, Saddle, Other.
- Reclaimed valve location (center of valve box cover), size and type.
- Reclaimed service line location (at connection to main, any bends, and termination at meter box or meter vault or at the edge of easement or ROW), size and material.
- Reclaimed blow-off and air release valve location (center of cover), size, type and manufacturer.
- Reclaimed manhole or vault location (center of cover), size and type.

4) Storm Water System Features:

- Storm manhole and cleanout location (center of cover), size, material, and cover type.
- Storm gravity main location (invert of pipe), size, material, and flow direction (from) at all manholes, cleanouts and structures.
- Storm structure location (center of structure), type and category including: Drywell, Catch Basin, Scupper, Bubbler Box, and Collection Vault.

5) Streetlight Feature:

- Streetlights

6) Standard GPS Metadata on all points collected: Date, Time, Horizontal Accuracy, Vertical Accuracy, Latitude, Longitude, Altitude, Surveyor, and Comments.

7) Electronic Data Submittal: Each submittal must consist of

- ESRI – ArcGIS Geodatabase with cumulative data and attributes
- Construction plans with collected utilities clearly redlined and changes marked

The frequency of data submittals will be every two weeks after water, sewer, reclaim or storm assets go into the ground. Please email submittals to: GIS@chandleraz.gov

DATA DICTIONARY: WATER

WATER_BACKFLOW				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	25	GIS_BACKFLOW_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	25	GIS_BACKFLOW_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	25	GIS_BACKFLOW_SPATIALSOURCE
SIZE	SIZE	Double		
TYPE	TYPE	Text	25	GIS_BACKFLOW_TYPE
MANUFACTURER	MANUFACTURER	Text	25	GIS_BACKFLOW_MANUFACTURER
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	15	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

WATER_FITTING				
Field Name	Alias Name	Data Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	20	WS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	20	WS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	20	WS_SPATIALSOURCE_STD
MATERIAL	MATERIAL	Text	20	WS_FITTING_MATERIAL
DIAMETER1	DIAMETER1	Text	8	RWS_FITTING_DIAMETER
DIAMETER2	DIAMETER2	Text	8	RWS_FITTING_DIAMETER
TYPE	TYPE	Text	30	WS_FITTING_TYPE
BEND	BEND	Text	20	WS_FITTING_BEND
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

WATER_HYDRANT				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	Feature Owner	Text	25	WS_FEATUREOWNER
LIFECYCLESTATUS	Lifecycle Status	Text	25	WS_LIFECYCLESTATUS
SPATIALSOURCE	Spatial Source	Text	25	WS_SPATIALSOURCE_STD
BARRELDIAMETER	Barrel Diameter	Double		
LARGENOZZLEDIAMETER	Large Nozzle Diameter	Double		
SMALLNOZZLEDIAMETER	Small Nozzle Diameter	Double		
OUTLETCONFIGURATION	Outlet Configuration	Double		
SEATDIAMETER	Seat Diameter	Double		
MANUFACTURER	Manufacturer	Text	25	WS_HYD_MANUFACTURER
YEARMANUFACTURED	Manufacture Year	Text	25	
HASLOCK	Has Lock ?	Text	25	GIS_BOOLEAN_YES_NO
COLOR	Color	Text	25	WS_HYD_COLOR
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	25	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	50	

WATER_MANHOLE				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	20	WS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	20	WS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	20	WS_SPATIALSOURCE_STD
ACCESSDIAMETER	ACCESSDIAMETER	Text	8	
ACCESSTYPE	ACCESSTYPE	Text	20	
GROUNDTYPE	GROUNDTYPE	Text	20	WS_SYSVAL_GROUNDTYPE
COVERMATERIAL	COVERMATERIAL	Text	10	
COVERTYPE	COVERTYPE	Text	8	
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

WATER_METER				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	Feature Owner	Text	20	GBA_WMSL_FEATUREOWNER
LIFECYCLESTATUS	Lifecycle Status	Text	25	GBA_WMSL_LIFECYCLESTATUS
SPATIALSOURCE	Spatial Source	Text	25	GBA_STATIC_SPATIALSOURCE
DIAMETER	DIAMETER	Double		WS_METER_DIAM
MANUFACTURER	Manufacturer	Text	25	WS_METER_MANUFACTURER
METER_TYPE	Meter Type	Text	25	WS_METER_TYPE
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	15	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

WATER_SYSTEM_VALVE				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATURE OWNER	Text	20	WS_SYSVAL_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLE STATUS	Text	20	WS_SYSVAL_LIFECYCLESTATUS
SPATIALSOURCE	SPATIAL SOURCE	Text	20	WS_SYSVAL_SPATIALSOURCE
TYPE	TYPE	Text	25	WS_SYSVAL_TYPE
VALVETYPE	VALVE TYPE	Text	10	WS_SYSVAL_VALVETYPE
FUNCTION	FUNCTION	Text	25	WS_SYSVAL_FUNCTION
GROUNDTYPE	GROUNDTYPE	Text	25	WS_SYSVAL_GROUNDTYPE
DIAMETER	Diameter	Text	8	WS_SYSVAL_DIAM
GPS_DATE	GPS DATE	Date		
GPS_TIME	GPS TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS COMMENT	Text	30	

Water Domains:

Domain name	WS_LIFECYCLESTATUS
ABANDONED	ABANDONED
ACTIVE	ACTIVE
FUTURE	FUTURE
INACTIVE	INACTIVE
REMOVED	REMOVED
UNKNOWN	UNKNOWN

Domain name	WS_SYSVAL_SPATIALSOURCE
APPROVED PLANS	APPROVED PLANS
ASBUILT	ASBUILT
DESIGN	DESIGN
GPS	GPS
HYDRMAINT	HYDRMAINT
ORTHOPHOTO	ORTHOPHOTO
TM7	TM7
UNKNOWN	UNKNOWN

Domain name	WS_HYD_MANUFACTURER
AMERICANDARLING	AMERICANDARLING
AVK	AVK
CLOW	CLOW
DRESSER	DRESSER
GREENBERG	GREENBERG
KENNEDY	KENNEDY
MUELLER	MUELLER
PACIFIC	PACIFIC
UNKNOWN	UNKNOWN
WATEROUS	WATEROUS

Domain name	WS_FEATUREOWNER
COC	Chandler
COG	Gilbert
COM	Mesa
COP	Phoenix
COT	Tempe
COUNTY	Maricopa County
GOULD	Gould
GRIC	GRIC
INTEL	Intel
MICROCHIP	Microchip
MOTOROLA	Motorola
PRIVATE	Private
SRP	SRP
SRVWUA	SRVWUA
UNKNOWN	UNKNOWN

Domain name	WS_SYSVAL_LIFECYCLESTATUS
FUTURE	FUTURE
UNKNOWN	UNKNOWN
INACTIVE	INACTIVE
ACTIVE	ACTIVE
REMOVED	REMOVED
ABANDONED	ABANDONED

Domain name	WS_FITTING_BEND
HORIZONTAL	HORIZONTAL
VERTICAL	VERTICAL

Domain name	WS_SYSVAL_DIAM
2	2
3	3
4	4
6	6
8	8
10	10
12	12
16	16
20	20
24	24
30	30
36	36
42	42
48	48
54	54

Domain name	WS_HYD_COLOR
0	YELLOW
1	BLACK/YELLOW
2	RED
3	UNKNOWN

Domain name	WS_FITTING_TYPE
11B	11B
22B	22B
45B	45B
90B	90B
COUPLING	COUPLING
CROSS	CROSS
ENDCAP	ENDCAP
LINESTOP	LINESTOP
OTHER	OTHER
REDUCER	REDUCER
SADDLE	SADDLE
TAPPING SLEEVE	TAPPING SLEEVE
TEE	TEE
UNKNOWN	UNKNOWN
WYE	WYE

Domain name	WS_SYSVAL_GROUNDTYPE
CONCRETE	CONCRETE
ASPHALT	ASPHALT
LANDSCAPE	LANDSCAPE
UNKNOWN	UNKNOWN

Domain name	WS_SPATIALSOURCE_STD
ASBUILT	ASBUILT
DESIGN	DESIGN
FIELD	FIELD
GPS	GPS
ORTHOPHOTO	ORTHOPHOTO
UNKNOWN	UNKNOWN

Domain name	WS_METER_DIAM
0.63	.63
0.75	.75
1	1
1.5	1.5
2	2
2.5	2.5
3	3
4	4
6	6
8	8
10	10

Domain name	WS_METER_MANUFACTURER
BADGER METERS	BADTER METERS
NEPTUNE METERS	NEPTUNE METERS
PRECISION METERS	PRECISION METERS
BADGER-NEPTUNE	BADGER-NEPTUNE
SENSUS	SENSUS

Domain name	WS_METER_TYPE
BYPASS	BYPASS
COMBO	COMBO
CMPND	CMPND
DISC	DISC
MAG	MAG
FDTEC	FDTEC
MLTJET	MLTJET
TURBIN	TURBIN
Ultrasonic	Ultrasonic

S

Domain name	WS_SYSVAL_FEATUREOWNER
COC	COC
PRIVATE	PRIVATE
COM	COM
MOTOROLA	MOTOROLA
INTEL	INTEL
UNKNOWN	UNKNOWN
TOG	TOG

Domain name	WS_FITTING_MATERIAL
DI	DI
PVC	PVC
BRASS	BRASS
STL	STL
COPPER	COPPER
AC	AC
CC	CC
CI	CI

Domain name	WS_SYSVAL_TYPE
STANDARD	STANDARD
BLOWOFF	BLOWOFF
ARV	ARV
AIR VACUUM	AIR VACUUM
PERMASEAL	PERMASEAL

Domain name	GBA_WMSI_LIFECYCLESTATUS
Operational	Operational
Empty	Empty
Out of Service	Out of Service
Abandoned	Abandoned

Domain name	WS_SYSVAL_FUNCTION
INLINE	INLINE
HYDRANT	HYDRANT
STUBOUT	STUBOUT
UNKNOWN	UNKNOWN
SERVICE	SERVICE
BYPASS	BYPASS
WPF	WPF
FDC	FDC
VAULT	VAULT
BACKFLOW	BACKFLOW
PUMPOUT	PUMPOUT
PUMPSTATION	PUMPSTATION
FIRELINE	FIRELINE
ARV	ARV
ZONE_SPLIT_VALVES	ZONE_SPLIT_VALVES

Domain name	GIS_BACKFLOW_FEATUREOWNER
COC	COC
PRIVATE	PRIVATE
CONTRACTOR	CONTRACTOR
INTEL	INTEL
OCA	OCA
OMG	OMG
TOG	TOG
UNKNOWN	UNKNOWN

Domain name	WS_SYSVAL_VALVETYPE
GATE	GATE
BUTTERFLY	BUTTERFLY
BALL	BALL
PLUG	PLUG
PIV	PIV
UNKNOWN	UNKNOWN
INSERTA	INSERTA

Domain name	GIS_BACKFLOW_MANUFACTURER
FEBCO	FEBCO
WATTS	WATTS
WILKINS	WILKINS
AMES	AMES
CONBRACO	SERVICE
HERSEY	HERSEY
RAIN BIRD	RAIN BIRD
APOLLO VALVES	APOLLO VALVES
METRON	METRON
OTHER	OTHER

Domain name	GIS_BACKFLOW_TYPE
Air Gap	Air Gap
Unknown	Unknown
Double Check Detector	Double Check Detector
Pressure Vacuum Breaker	Pressure Vacuum Breaker
Double Check	Double Check
Reduced Pressure Detector	Reduced Pressure Detector
Atmospheric Vacuum Breaker	Atmospheric Vacuum Breaker

DATA DICTIONARY: SEWER

SEWER CLEANOUT				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	20	WW_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	20	WW_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	20	WW_SPATIALSOURCE
RIM_ELEV	RIM_ELEV	Double		
INV_ELEV	INV_ELEV	Double		
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

SEWER CONTROL VALVE				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	20	WW_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	20	WW_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	20	WW_SPATIALSOURCE
TYPE	TYPE	Text	25	WW_CONTROL_VALVE_TYPE
DIAMETER	DIAMETER	Text	8	
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

SEWER_FITTING				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	20	WW_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	20	WW_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	20	WW_SPATIALSOURCE
MATERIAL	MATERIAL	Text	20	WW_FITTING_MATERIAL
TYPE	TYPE	Text	30	WW_FITTING_TYPE
BEND	BEND	Text	20	WW_FITTING_BEND
SIZE	SIZE	Text	20	
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

SEWER_MANHOLE				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	25	WW_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	25	WW_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	20	WW_SPATIALSOURCE
TYPE	TYPE	Text	20	WW_MH_TYPE
COVER_MATERIAL	COVER_MATERIAL	Text	25	WW_MH_COVER_MATERIAL
COVER_HOLE	COVER_HOLE	Text	25	WW_MH_COVER_HOLE
COVER_SIZE	COVER_SIZE	Text	20	WW_MH_COVER_SIZE
RIM_ELEV	RIM_ELEV	Double		
INV_ELEV	INV_ELEV	Double		
LINING	LINING	Text	255	WW_MH_LINER
WALL_MATERIAL	WALL_MATERIAL	Text	255	WW_MH_WALL_MATERIAL
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

Sewer Domains:

Domain name	WW_FEATUREOWNER
COC	COC
PRIVATE	PRIVATE
TOG	TOG
UNKNOWN	UNKNOWN

Domain name	WW_SPATIALSOURCE
GPS	GPS
ASBUILT	ASBUILT
UNKNOWN	UNKNOWN

Domain name	WW_MH_COVER_HOLE
UNKNOWN	UNKNOWN
SEALED	SEALED

Domain name	WW_MH_LINER
CAST-IN-PLACE	CAST-IN-PLACE
CEMENTITIOUS	CEMENTITIOUS
COMPOSITE	COMPOSITE
CURED-IN-PLACE	CURED-IN-PLACE
EPOXY	EPOXY
FR PLASTIC INSERT	FR PLASTIC INSERT
FR POLYMER INSERT	FR POLYMER INSERT
INSERT	INSERT
UNKNOWN	UNKNOWN

Domain name	WW_LIFECYCLESTATUS
ABANDONED	ABANDONED
ACTIVE	ACTIVE
INACTIVE	INACTIVE
PULLED	PULLED
UNKNOWN	UNKNOWN

Domain name	WW_MH_COVER_MATERIAL
ALUMINUM	ALUMINUM
CASTIRON	CASTIRON
HDPE COMPOSITE - CAP	HDPE COMPOSITE - CAP
HDPE COMPOSITE - EJ	HDPE COMPOSITE - EJ
HDPE COMPOSITE - HD20	HDPE COMPOSITE - HD20
STEEL	STEEL
UNKNOWN	UNKNOWN

Domain name	WW_MH_COVER_SIZE
0	Unknown
4	4
6	6
8	8
22	22
24	24
26	26
28	28
30	30
32	32
38	38
48	48
60	60

Domain name	WW_MH_WALL_MATERIAL
NONE	NONE
COMPOSITE	COMPOSITE
UNKNOWN	UNKNOWN
CONCRETE	CONCRETE
POURED	POURED
TLOCK	TLOCK
PRECAST	PRECAST
BRICK	BRICK

Domain name	WW_FITTING_MATERIAL
PVC	PVC
DIP	DIP
HDPE	HDPE
RCP	RCP
ACP	ACP
ABS	ABS
RUBBER	RUBBER
CONCRETE	CONCRETE
CLAY	CLAY
STEEL ENCASED VCP	STEEL ENCASED VCP
TLOCK LINE	TLOCK LINE
LINED RCP	LINED RCP
PVC LINED RCP	PVC LINED RCP
PVC LINED DIP	PVC LINED RCP

Domain name	WW_FITTING_BEND
HORIZONTAL	HORIZONTAL
VERTICAL	VERTICAL
NONE	NONE

Domain name	WW_MH_TYPE
0	UNKNOWN
1	SANITARY SEWER
2	ARV

Domain name	WW_FITTING_TYPE
TEE	TEE
WYE	WYE
90 BEND	90 BEND
45 BEND	45 BEND
22 BEND	22 BEND
11 BEND	11 BEND
ENCAP	ENCAP
PLUG	PLUG
REDUCER	REDUCER
ARV	ARV
FLOW METER	FLOW METER
NO FLOW POINT	NO FLOW POINT
OWNERSHIP CHANGE	OWNERSHIP CHANGE
OTHER	OTHER
UNKNOWN	UNKNOWN
COUPLING	COUPLING

Domain name	WW_CONTROL_VALVE_TYPE
AIRRELEASE	AIRRELEASE
AIRVACUUMRELEASE	AIRVACUUMRELEASE
PRESSURERELIEF	PRESSURERELIEF
VACUUMRELEASE	VACUUMRELEASE
UNKNOWN	UNKNOWN
N/A	N/A

DATA DICTIONARY: RECLAIM

RECLAIM_WATER_BACKFLOW				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	25	GIS_BACKFLOW_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	25	GIS_BACKFLOW_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	25	GIS_BACKFLOW_SPATIALSOURCE
SIZE	SIZE	Double		
TYPE	TYPE	Text	25	GIS_BACKFLOW_TYPE
MANUFACTURER	MANUFACTURER	Text	25	GIS_BACKFLOW_MANUFACTURER
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	15	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

RECLAIM_WATER_FITTING				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	20	RWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	20	RWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	20	RWS_SPATIALSOURCE
MATERIAL	MATERIAL	Text	20	RWS_MAIN_OR_SERVICE_MATERIAL
DIAMETER1	DIAMETER1	Text	8	RWS_FITTING_DIAMETER
DIAMETER2	DIAMETER2	Text	8	RWS_FITTING_DIAMETER
TYPE	TYPE	Text	30	RWS_FITTING_TYPE
BEND	BEND	Text	20	RWS_FITTING_BEND
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double	8	
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double	8	
GPS_COMMENT	GPS_COMMENT	Text	30	

RECLAIM_WATER_MANHOLE				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	20	RWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	20	RWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	20	RWS_SPATIALSOURCE
ACCESSDIAMETER	ACCESSDIAMETER	Text	8	
ACCESSTYPE	ACCESSTYPE	Text	20	
GROUNDTYPE	GROUNDTYPE	Text	20	RWS_GROUNDTYPE
COVERMATERIAL	COVERMATERIAL	Text	10	RWS_COVER_MATERIAL
COVERTYPE	COVERTYPE	Text	8	RWS_COVER_TYPE
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

RECLAIM_WATER_METER				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	Feature Owner	Text	20	GBA_WMSL_FEATUREOWNER
LIFECYCLESTATUS	Lifecycle Status	Text	25	GBA_WMSL_LIFECYCLESTATUS
SPATIALSOURCE	Spatial Source	Text	25	GBA_STATIC_SPATIALSOURCE
DIAMETER	DIAMETER	Double		RWS_METER_DIAM
MANUFACTURER	Manufacturer	Text	25	RWS_METER_MANUFACTURER
METERTYPE	Meter Type	Text	25	RWS_METERTYPE
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	15	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	30	

RECLAIM WATER SYSTEM VALVE				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATURE OWNER	Text	20	RWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLE STATUS	Text	20	RWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIAL SOURCE	Text	20	RWS_SPATIALSOURCE
TYPE	TYPE	Text	25	RWS_SYSVAL_TYPE
VALVETYPE	VALVE TYPE	Text	10	RWS_SYSVAL_VALVETYPE
FUNCTION	FUNCTION	Text	25	RWS_SYSVAL_FUNCTION
GROUNDTYPE	GROUNDTYPE	Text	25	RWS_SYSVAL_GROUNDTYPE
DIAMETER	DIAMETER	Double		RWS_SYSVAL_DIAM
GPS_DATE	GPS DATE	Date	8	
GPS_TIME	GPS TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS COMMENT	Text	30	

Reclaim Water Domains:

Domain name	RWS_SYSVAL_VALVETYPE
BALL	BALL
BUTTERFLY	BUTTERFLY
GATE	GATE
INSERTA	INSERTA
PIV	PIV
PLUG	PLUG
UNKNOWN	UNKNOWN

Domain name	RWS_SYSVAL_FUNCTION
ARV	ARV
INLINE	INLINE
PUMPOUT	PUMPOUT
PUMPSTATION	PUMPSTATION
SERVICE	SERVICE
STUBOUT	STUBOUT
UNKNOWN	UNKNOWN
VAULT	VAULT

Domain name	RWS_SYSVAL_TYPE
ARV	ARV
AIR VACUUM	AIR VACUUM
BLOWOFF	BLOWOFF
PERMASEAL	PERMASEAL
STANDARD	STANDARD

Domain name	RWS_FITTING_BEND
HORIZONTAL	HORIZONTAL
VERTICAL	VERTICAL

Domain name	RWS_MAIN_OR_SERVICE_MATERIAL
AC	AC
CC	CC
CI	CI
CU	COPPER
DI	DI
PVC	PVC
UNKNOWN	UNKNOWN
AC	AC
CC	CC

Domain name	RWS_FITTING_TYPE
11B	11B
22B	22B
45B	45B
90B	90B
COUPLING	COUPLING
CROSS	CROSS
ENDCAP	ENDCAP
LINESTOP	LINESTOP
MECHANICAL JOINT	MECHANICAL JOINT
OTHER	OTHER
RECHARGE WELL	RECHARGE WELL
REDUCER	REDUCER
SADDLE	SADDLE
TAPPING SLEEVE	TAPPING SLEEVE
TEE	TEE
UNKNOWN	UNKNOWN
WYE	WYE

Domain name	RWS_SPATIALSOURCE
ASBUILT	ASBUILT
FIELD	FIELD
GPS	GPS
ORTHOPHOTO	ORTHOPHOTO
UNKNOWN	UNKNOWN

Domain name	RWS_LIFECYCLESTATUS
ABANDONED	ABANDONED
ACTIVE	ACTIVE
INACTIVE	INACTIVE
REMOVED	REMOVED
UNKNOWN	UNKNOWN

Domain name	RWS_SYSVAL_DIAM
1.5	1.5
2	2
3	3
4	4
6	6
8	8
10	10
12	12
16	16
20	20
24	24
30	30
36	36
42	42
48	48
9999	UNKNOWN

Domain name	RWS_GROUNDTYPE
ASPHALT	ASPHALT
CONCRETE	CONCRETE
DIRT	DIRT
UNKNOWN	UNKNOWN

Domain name	RWS_SYSVAL_GROUNDTYPE
ASPHALT	ASPHALT
CONCRETE	CONCRETE
LANDSCAPE	LANDSCAPE
UNKNOWN	UNKNOWN

Domain name	RWS_FITTING_DIAMETER
0.63	0.63
0.75	0.75
1	1
1.25	1.25
1.5	1.5
2	2
2.5	2.5
3	3
4	4
6	6
8	8
10	10
12	12
15	15
16	16
18	18
20	20
24	24
30	30

Domain name	RWS_COVER_MATERIAL
CAST IRON	CAST IRON
ALUMINUM	ALUMINUM
UNKNOWN	UNKNOWN

Domain name	RWS_COVER_TYPE
SEALED	SEALED
VENTED	VENTED
BOLTED	BOLTED
UNKNOWN	UNKNOWN

Domain name	RWS_METER_MANUFACTURER
BADGER METERS	BADGER METERS
NEPTUNE METERS	NEPTUNE METERS
PRECISION METERS	PRECISION METERS
BADGER-NEPTUNE	BADGER-NEPTUNE
SENSUS	SENSUS

Domain name	RWS_METERTYPE
BYPASS	BYPASS
COMBO	COMBO
CMPND	CMPND
DISC	DISC
MAG	MAG
FDTEC	FDTEC
MLTJET	MLTJET
TURBIN	TURBIN

Domain name	RWS_METER_DIAM
0.63	.63
0.75	.75
1	1
1.5	1.5
2	2
2.5	2.5
3	3
4	4
6	6
8	8
10	10

Domain name	RWS_FEATUREOWNER
COC	CHANDLER
COM	MESA
INTEL	INTEL
OCA	OCA
OMG	OMG
PRIVATE	PRIVATE
SRP	SRP
TOG	GILBERT
UNKNOWN	UNKNOWN

DATA DICTIONARY: STORM

STORM_BUBBLER_BOX				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	25	SWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	25	SWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	25	SWS_SPATIALSOURCE
GRATE_OPENINGS_NUM	GRATE_OPENINGS_NUM	Short		SWS_GRATE_OPENINGS
GRATE_LENGTH	GRATE_LENGTH	Double	8	SWS_GRATE_LENGTH
GRATE_WIDTH	GRATE_WIDTH	Double	8	SWS_GRATE_WIDTH
DRYWELL_ASSOC	DRYWELL_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	50	

STORM_CATCHBASIN				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	25	SWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	25	SWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	25	SWS_SPATIALSOURCE
STRUCTURE_TYPE	STRUCTURE_TYPE	Short		SWS_STRUCTURE_TYPE
CATCHBASIN_TYPE	CATCHBASIN_TYPE	Text	25	SWS_CB_TYPE
CITY_EMBLEM	CITY_EMBLEM	Text	5	GIS_BOOLEAN_YES_NO
ACCESS_OPENING_TYPE	ACCESS_OPENING_TYPE	Text	25	SWS_CB_ACCESS_OPENING_TYPE
GRATE_LENGTH	GRATE_LENGTH	Double		SWS_GRATE_LENGTH
GRATE_WIDTH	GRATE_WIDTH	Double		SWS_GRATE_WIDTH
LID	LID	Text	5	GIS_BOOLEAN_YES_NO
LID_TYPE	LID_TYPE	Text	25	SWS_CB_LID_TYPE
DRYWELL_CONN	DRYWELL_CONN	Text	5	GIS_BOOLEAN_YES_NO
OUTLET_CONN	OUTLET_CONN	Text	5	GIS_BOOLEAN_YES_NO
BUBBLERBOX_CONN	BUBBLERBOX_CONN	Text	5	GIS_BOOLEAN_YES_NO
BUBBLERBOX_DRYWELL_ASSOC	BUBBLERBOX_DRYWELL_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
LAKE_DRAIN	LAKE_DRAIN	Text	5	GIS_BOOLEAN_YES_NO
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	50	

STORM_COLLECTION_VAULT				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	20	SWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	20	SWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	20	SWS_SPATIALSOURCE
INLET_NUM	INLET_NUM	Short		SWS_CV_NUMB_INLETS
INLET_DIAM	INLET_DIAM	Short		SWS_CV_PIPE_SIZE
INLET_PIPE_TYPE	INLET_PIPE_TYPE	Text	30	SWS_CV_PIPE_TYPE
NUISANCE_LINE_ASSOC	NUISANCE_LINE_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
DRYWELL_ASSOC	DRYWELL_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
LID_GRATE	LID_GRATE	Text	5	SWS_LID_GRATE
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	50	

STORM_DRAIN_MANHOLE				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	25	SWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	25	SWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	25	SWS_SPATIALSOURCE
STRUCTURE_TYPE	STRUCTURE_TYPE	Short	2	SWS_STRUCTURE_TYPE
CHANDLER_NAME	CHANDLER_NAME_ON_LID	Text	5	GIS_BOOLEAN_YES_NO
DIAMETER	DIAMETER	Short	2	SWS_SDMH_SIZE
VENT_HOLE	VENT_HOLE	Text	30	GIS_BOOLEAN_YES_NO
ACCESS_OPENING_TYPE	ACCESS_OPENING_TYPE	Text	25	SWS_SDMH_ACCESS_OPENING_TYPE
LID_TYPE	LID_TYPE	Text	30	SWS_SDMH_TYPE
LID_TEXT	LID_TEXT	Text	30	SWS_SDMH_LID_TEXT
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	50	

STORM_DRYWELL				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	25	SWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	25	SWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	25	SWS_SPATIALSOURCE
FIELD_LOC	FIELD_LOC	Text	100	SWS_DW_LOCATION
INSTALLER	INSTALLER	Text	30	SWS_DW_INSTALLER
CHAMBER_TYPE	CHAMBER_TYPE	Text	30	SWS_DW_CHAMBER_TYPE
LID_SIZE	LID_SIZE	Short		SWS_DW_LID_SIZE
LID_TYPE	LID_TYPE	Text	25	SWS_DW_LID_TYPE
NUISANCE_LINE_ASSOC	NUISANCE_LINE_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
CONCRETE_PAD	CONCRETE_PAD	Text	5	GIS_BOOLEAN_YES_NO
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	50	

STORM_INLET				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	25	SWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	25	SWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	25	SWS_SPATIALSOURCE
CATCHBASIN_TYPE	CATCHBASIN_TYPE	Text	25	SWS_CB_TYPE
FIELD_LOC	FIELD_LOC	Text	100	SWS_FIELD_LOC
GRATE_ASSOC	GRATE_ASSOC	Text	25	GIS_BOOLEAN_YES_NO
GRATE_LENGTH	GRATE_LENGTH	Double		SWS_GRATE_LENGTH
GRATE_WIDTH	GRATE_WIDTH	Double		SWS_GRATE_WIDTH
INLET_PIPE_DIAM	INLET_PIPE_DIAM	Short		SWS_INLET_PIPE_DIAM
INLET_NUMBER	NUMBER_OF_INLETS	Short		SWS_INLET_NUMBER
TRASHRACK_ASSOC	TRASHRACK_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
TRASH_LENGTH	TRASH_LENGTH	Double		
TRASH_WIDTH	TRASH_WIDTH	Double		
NUISANCE_LINE_ASSOC	NUISANCE_LINE_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
BUBBLER_BOX_DRYWELL_ASSOC	BUBBLER_BOX_DRYWELL_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
HEADWALLASSOC	HEADWALLASSOC	Text	5	GIS_BOOLEAN_YES_NO
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	50	

STORM_OUTLET				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	25	SWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	25	SWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	25	SWS_SPATIALSOURCE
GRATE_ASSOC	GRATE_ASSOC	Text	25	GIS_BOOLEAN_YES_NO
GRATE_LENGTH	GRATE_LENGTH	Double		SWS_GRATE_LENGTH
GRATE_WIDTH	GRATE_WIDTH	Double		SWS_GRATE_WIDTH
INLET_PIPE_DIAM	INLET_PIPE_DIAM	Short		SWS_INLET_PIPE_DIAM
INLET_NUMBER	NUMBER_OF_INLETS	Short		SWS_INLET_NUMBER
TRASHRACK_ASSOC	TRASHRACK_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
TRASH_LENGTH	TRASH_LENGTH	Double		
TRASH_WIDTH	TRASH_WIDTH	Double		
NUISANCE_LINE_ASSOC	NUISANCE_LINE_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
BUBBLER_BOX_DRYWELL_ASSOC	BUBBLER_BOX_DRYWELL_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
HEADWALLASSOC	HEADWALLASSOC	Text	5	GIS_BOOLEAN_YES_NO
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	50	

STORM_SCUPPER				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	FEATUREOWNER	Text	25	SWS_FEATUREOWNER
LIFECYCLESTATUS	LIFECYCLESTATUS	Text	25	SWS_LIFECYCLESTATUS
SPATIALSOURCE	SPATIALSOURCE	Text	25	SWS_SPATIALSOURCE
CITY_EMBLEM	CITY_EMBLEM	Text	5	GIS_BOOLEAN_YES_NO
NUMBER_OPENINGS	NUMBER_OPENINGS	Short		
WIDTH	WIDTH	Double		
SPILLWAY_ASSOC	SPILLWAY_ASSOC	Text	25	GIS_BOOLEAN_YES_NO
HEADWALL_ASSOC	HEADWALL_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
NUISANCE_LINE_ASSOC	NUISANCE_LINE_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
BUBBLERBOX_ASSOC	BUBBLERBOX_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
BUBBLER_BOX_DRYWELL_ASSOC	BUBBLER_BOX_DRYWELL_ASSOC	Text	5	GIS_BOOLEAN_YES_NO
LAKE_DRAIN	LAKE_DRAIN	Text	5	GIS_BOOLEAN_YES_NO
RIPRAP	RIPRAP	Text	25	GIS_BOOLEAN_YES_NO
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	50	

Storm Domains:

Domain name	SWS_DW_LOCATION
AIRPORT	AIRPORT
CITYRETENTION	CITY RETENTION
PARK	PARK
PARKING_LOT	PARKING LOT
PRIVATERETENTION	PRIVATE RETENTION
STREET	STREET
WELL_SITE	WELL SITE

Domain name	SWS_GRATE_LENGTH
12	12
24	24
28	28
30	30
36	36
38	38
40	40
50	50
60	60
76	76

Domain name	SWS_CB_LID_TYPE
STEEL	STEEL
CONCRETE	CONCRETE

Domain name	SWS_DW_CHAMBER_TYPE
DOUBLE	DOUBLE
SINGLE	SINGLE
TRIPLE	TRIPLE

Domain name	SWS_LIFECYCLESTATUS
ACTIVE	ACTIVE
REMOVED	REMOVED
ABANDONED	ABANDONED
INACTIVE	INACTIVE
FUTURE	FUTURE

Domain name	SWS_GRATE_OPENINGS
1	1
2	2
3	3
4	4
5	5
6	6

Domain name	SWS_SPATIALSOURCE
APPROVED_PLANS	APPROVED_PLANS
ASBUILT	ASBUILT
DESIGN	DESIGN
FIELD	FIELD
GPS	GPS
ORTHOPHOTO	ORTHOPHOTO
UNKNOWN	UNKNOWN

Domain name	SWS_SDMH_SIZE
22	22
24	24
26	26
28	28
30	30
32	32
38	38

Domain name	SWS_DW_LID_SIZE
22	22
24	24
25	25
26	26
32	32
38	38

Domain name	SWS_STRUCTURE_TYPE
1	CATCH BASIN
2	SCUPPER
3	DRYWELL
4	MANHOLE
5	OUTLET
6	INLET
7	BUBBLERBOX
8	SPILLWAY
9	SEPARATOR

Domain name	SWS_SDMH_LID_TEXT
CHANDLER_SANITATION_SEWER	CHANDLER SANITATION SEWER
CHANDLER_STORM/SEWER	CHANDLER STORM/SEWER
NONE	NONE
OTHER	OTHER
STORM SEWER	STORM SEWER

Domain name	SWS_DW_INSTALLER
McGUCKINDRILLING	McGUCKIN DRILLING
TORRENTRESOURCES	TORRENT RESOURCES
WACODRILLING	WACO DRILLING
OTHER	OTHER

Domain name	SWS_CB_ACCESS_OPENING_TYPE
NONE	NONE
RECTANGULAR	RECTANGULAR
ROUND	ROUND
SQUARE	SQUARE

Domain name	SWS_FEATUREOWNER
ADOT	ADOT
CITY_OF_CHANDLER	CITY_OF_CHANDLER
HOA	HOA
OTHER	OTHER

Domain name	SWS_GRATE_WIDTH
6	6
12	12
16	16
18	18
24	24
28	28
30	30
36	36
38	38
44	44
72	72

Domain name	SWS_SDMH_ACCESS_OPENING_TYPE
NONE	NONE
RECTANGULAR	RECTANGULAR
ROUND	ROUND
SQUARE	SQUARE

Domain name	SWS_FIELD_LOC
PARK	PARK
WELLSI	WELL_SITE
PRKLOT	PARKING_LOT
STREET	STREET
AIRPOR	AIRPORT
CTYRTN	CITYRETENTION
PVRET	PRIVATERETENTION

Domain name	SWS_SDMH_TYPE
PRESSURIZED	PRESSURIZED
NON-PRESSURIZED	NON-PRESSURIZED

Domain name	SWS_DW_LID_TYPE
STEEL	STEEL
GRANITE	GRANITE
CONCRETE_SEALED	CONCRETE_SEALED
GRATE	GRATE

Domain name	SWS_INLET_PIPE_DIAM
12	12
13	13
14	14
15	15
16	16
18	18
24	24
26	26
30	30
36	36
42	42
46	46
60	60
72	72

Domain name	SWS_INLET_NUMBER
1	1
2	2
3	3
4	4

Domain name	SWS_CB_TYPE
TYPE_A	TYPE_A
TYPE_B	TYPE_B
TYPE_C	TYPE_C
TYPE_D	TYPE_D
TYPE_E	TYPE_E
TYPE_F	TYPE_F
TYPE_G	TYPE_G
TYPE_H	TYPE_H
TYPE_Q	TYPE_Q
NONE	NONE

Domain name	SWS_LID_GRATE
LID	LID
GRATE	GRATE

Domain name	SWS_CV_PIPE_TYPE
CONCRETE	CONCRETE
OTHER	OTHER
METAL	METAL

Domain name	SWS_CV_PIPE_SIZE
12	12
14	14
16	16
18	18
20	20
24	24
30	30
36	36

DATA DICTIONARY: MISCELLANEOUS POINTS

MISCELLANEOUS POINT				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
SIZE	SIZE	Double		
MATERIAL	MATERIAL	Text	30	
UTILITY_TYPE	UTILITY_TYPE	Text	30	PW_UTILITY_TYPE
NOTES	NOTES	Text	150	
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	50	

Domains:

Domain name	PW_UTILITY_TYPE
WATER	WATER
WASTEWATER	WASTEWATER
RECLAIM	RECLAIM
STORM	STORM
OTHER	OTHER

DATA DICTIONARY: STREETLIGHTS

STREETLIGHTS				
Field Name	Alias Name	Type	Length	Domain
OBJECTID	OBJECTID	OID		
SHAPE	SHAPE	Geometry		
FEATUREOWNER	POLE_OWNER	Text	10	ST_STLITINV_FEATUREOWNER
SPATIALSOURCE	SPATIALSOURCE	Text	25	GBA_STATIC_SPATIALSOURCE
POLENUMBER	POLENUMBER	Text	10	
POLE_HEIGHT	POLE_HEIGHT	Short		
POLE_TYPE	POLE_TYPE	Text	30	TE_STLITINV_POLE_TYPE
POLE_COAT	POLE_COAT	Text	30	TE_STLITINV_POLE_COAT
FOUNDATION	FOUNDATION	Text	30	GIS_BOOLEAN_YES_NO
GPS_DATE	GPS_DATE	Date		
GPS_TIME	GPS_TIME	Text	10	
LATITUDE	LATITUDE	Double		
LONGITUDE	LONGITUDE	Double		
ALTITUDE	ALTITUDE	Double		
HORIZONTAL_ACCURACY	HORIZONTAL_ACCURACY (m)	Double		
VERTICAL_ACCURACY	VERTICAL_ACCURACY (m)	Double		
GPS_COMMENT	GPS_COMMENT	Text	255	

Streetlights Domains:

Domain name	ST_STLITINV_FEATUREOWNER
COC	CITY OF CHANDLER
PRIVATE	PRIVATE
SRP	SRP
AP	APS

Domain name	GBA_STATIC_SPATIALSOURCE
GPS	GPS
ORTHOPHOTO	ORTHOPHOTO
APPROVED_PLANS	APPROVED_PLANS
GEOCODED	GEOCODED
UNKNOWN	UNKNOWN

Domain name	TE_STLITINV_POLE_TYPE
SL 1	SL 1
SL 6	SL 6
SL 8	SL 8
SL 10	SL 10
SL 16	SL 16
SL 17	SL 17
CURVED	CURVED
CUSTOM	CUSTOM
STERNBERG	STERNBERG
SIGNAL POLE	SIGNAL POLE
POWER POLE	POWER POLE

Domain name	TE_STLITINV_POLE_COAT
GALVANIZED	GALVANIZED
PAINTED	PAINTED
POWDER COATED	POWDER COATED

EXHIBIT G

SUBCONTRACTOR DOCUMENTS WITH CM@RISK

Any subcontractor assumptions, clarifications, exclusions, terms & conditions, signature blocks, etc. included are strictly between the CM@Risk and their subcontractors, and do not apply to the Agreement between the CM@Risk and the City.

DRM ABC QUOTE
SELECTED AS
LOWEST BIDDER

DESERT READY MIX

READY MIX CONCRETE SALES AND SERVICE

4011 PRESIDIO STREET

MESA, AZ 85215

OFFICE: 480-428-2280 FAX: 480-985-4248

Begin Date: February 19, 2025
Expiration Date: December 31, 2025

Quote # : 25967
MAP/GRID

Customer: **B & F CONTRACTING INC**
Customer Code: BFCO001
Address: 11011 N 23RD AVE
PHOENIX, AZ 85029
Contact: NANCY A/P
Phone: 623-582-1170 O

Fax:
Project: **CITY OF CHANDLER CMAR 66-INCH REDUNDANT**
Job Address: PRICE ROAD & LOOP 202
Terms Description: **2% 10TH NET 30**

Item Code	Item Description	Est. Qty	Price
S500501	PHCLSM.5 (1/2 SK SLURRY COP)	5,400	\$108.00
SRP104	1/2SK MAG CLSM		\$108.00
TC	TEMPERATURE CONTROL		\$1.00
A203001	MAG A 3000 PSI W/ASH	60	\$140.00
A204001	MAG AA 4000 PSI W/ASH	22	\$148.00
G402061	2000 PSI GROUT	210	\$165.00
AG010003 DELIVERED	MAG SPEC ABC (NON-LIME)	15,000	\$20.00
AG020003 DELIVERED	CONCRETE SAND	200	\$25.00
AG040002 DELIVERED	3/8 REJECT BEDDING CHIPS ***IF AVAILABLE***	1,000	\$18.00
AG040001 DELIVERED	#8 (3/8 CONCRETE ROCK)		\$23.25

Ready Mix Additional Fees

Fee ID *	UOM	Price
E01	cy	3.25
FSC	LOAD	TBD

Ready Mix Minimum Load Fee

Less than or equal to	UOM	Price
2	Load	0
3.5	Load	0
5.5	Load	0

- Normal operating hours are Mon-Fri, Sat, Sun or Holidays are subject to surcharges.
- Color added at no expressed guarantee.
- Drivers do not accept cash or make change.
- Desert Ready Mix reserves the right to increase prices due to RAW Material cost increases such as Cement, Fly Ash, Aggregates or unexpected Fuel increases.
- All specialized aggregate sales are based on availability.
- Deliveries less than 6 Cubic yards are subject to our minimum load charges.
- This quote does not guarantee product or service availability
- All Aggregates and Concrete loads have an additional fuel surcharge.

Quoted By: JUSTIN DAWSON

* E – Environmental | S – Saturday Fee | F – Fuel Surcharge

Travis Monk

From: Insearch Corp Estimating <estimating@insearchcorp.com>
Sent: Wednesday, July 16, 2025 8:04 AM
To: Travis Monk
Cc: Danny Reese; Saakshi Amrale
Subject: RE: City of Chandler CMAR 66" Redundant Sewer - Updated Quote Request

Travis,
Unfortunately, we have no idea what the suppliers are going to do in 2026. My guess is add another \$1.00 to the ABC and make the reject \$20.00 a ton. Hopefully, that is enough, and we may get lucky and there will be no increase.

Thanks,

Thank you,

Heather Woodhurst
Senior Estimator
602-622-6072



From: Travis Monk <tmonk@bfcontracting.com>
Sent: Tuesday, July 15, 2025 5:41 PM
To: Insearch Corp Estimating <estimating@insearchcorp.com>
Cc: Danny Reese <dreese@insearchcorp.com>; Saakshi Amrale <saakshi.amrale@bfcontracting.com>
Subject: RE: City of Chandler CMAR 66" Redundant Sewer - Updated Quote Request

Good Evening,

Can I get an updated quote for MAG Spec ABC delivered to Price Road and the 202, Chandler, AZ? It would be greatly appreciated if you could provide the updated quote by Friday. Project is anticipated to start January of 2026. Please keep that in mind when pricing and please detail any anticipated pricing increases for next year.

Thanks,



Built on Integrity!

Travis Monk
Project Manager
Mobile: 602-326-9282 | tmonk@bfcontracting.com

B&F Contracting, Inc. | 11011 N 23rd Ave, Phoenix, AZ 85029
Office: 623-582-1170 | Fax: 623-582-3761 | www.bfcontracting.com
ROC08974 A | ROC111282 B-4

From: Insearch Corp Estimating <estimating@insearchcorp.com>
Sent: Monday, March 24, 2025 6:55 AM
To: Travis Monk <tmonk@bfcontracting.com>
Cc: Danny Reese <dreese@insearchcorp.com>
Subject: RE: City of Chandler CMAR 66" Redundant Sewer - Updated Quote Request

Travis,
I emailed a price for the chips, and the previous quote for ABC \$20.40 is good thru June. Then we need to add \$1.00 per the supplier. \$21.40 is good thru 12/31/2025

Thank you,

Heather Woodhurst
Senior Estimator
602-622-6072



From: Travis Monk <tmonk@bfcontracting.com>
Sent: Friday, March 21, 2025 9:49 AM
To: Insearch Corp Estimating <estimating@insearchcorp.com>
Subject: City of Chandler CMAR 66" Redundant Sewer - Updated Quote Request

Good Morning,

For this Chandler 66" Redundant Sewer CMAR project, can I get an updated quote for the below materials delivered to Price Road and the 202, Chandler, AZ? Thanks and let me know if you have any questions.

- Aggregates:
 - MAG Spec ABC: 15,000 Tons
 - Sand: 200 Tons
 - Sand is for 48" Sewer Pipe Bedding and Backfill to Springline of Pipe (I couldn't find any specific specs in MAG)
 - 3/8" Minus Pea Gravel: 1000 Tons

Travis Monk

From: Castro, Jesse <castroje@vmcmail.com>
Sent: Thursday, July 17, 2025 4:21 PM
To: Travis Monk
Cc: Saakshi Amrale
Subject: Re: RFQ - City of Chandler CMAR 66-inch Redundant Sewer

Good afternoon,

Updated pricing for this project would be-

MAG Spec ABC 25.96/Ton Delivered

Trackout Rock 33.7/Ton Delivered

This pricing would be through end of 2025

Jesse Castro
Vulcan Materials Company
AZ Sales Representative
Mobile - 480-708-9498
Email - castroje@vmcmail.com

On Tue, Jul 15, 2025 at 5:44 PM Travis Monk <tmonk@bfcontracting.com> wrote:

Jesse,

Can you provide an updated price for MAG Spec ABC delivered to Price Road and the 202, Chandler, AZ?
It will be about 15,000 tons.

Can you also include 100 tons of track-out rock?

Thanks,

7/18/2025

Project Proposal

Re: Loop 101 and 202 Freeway 66" Redundant Sewer – CIPP Lining
Chandler, AZ

Attn: Travis Monk
B&F Contracting, Inc.

INSITUFORM TECHNOLOGIES, LLC herein proposes to furnish a Proposal for the labor, materials, equipment, and services set forth below to reconstruct the referenced project. The following prices should be considered confidential.

PROPOSAL PRICING

Bid Item	Description	Quantity	Unit	Unit Cost	Amount
1	Mobilization (Three total)	1	LS	\$ 38,109.00	\$ 38,109.00
2	CIPP 30"	938	LF	\$ 251.00	\$ 235,438.00
3	CIPP 48"	2,019	LF	\$ 399.00	\$ 805,581.00
4	CIPP 66"	652	LF	\$ 981.00	\$ 639,612.00
	TOTAL				\$ 1,718,740.00
	<i>Lining and cleaning/CCTV only (See full list of Inclusions & Exclusions below)</i>				

ASSUMPTIONS AND QUALIFICATIONS

Insituform Technologies, LLC (Insituform) have based this proposal on a nominal wall thickness for the Insitutube as shown in the specifications or derived from general, industry accepted specifications and design criteria. This is based on the best available information at the time of this proposal. Existing pipe deterioration in excess of the conditions assumed, ground water loads in excess of those assumed, or other loads or conditions may increase the recommended thickness for all or portions of the work. Final recommendations may be submitted to you following the completion of the preliminary TV phase of the project. Stated prices are subject to adjustment if design changes are agreed upon.

Insituform is a NON-union shop, is considered a specialty contractor, and shall not be subject to any union requirements or project labor agreements.

Water shall be provided at no cost to Insituform for all construction phases of this project. Insituform will follow all required deposit, backflow prevention, and metering procedures.

Access requirements for existing pipeline to be rehabilitated shall be provided by Owner and/or Prime Contractor – includes entry and exit access approved from Insituform Project Manager and/or Field Engineer.

Specific service connections, if encountered, will not be reconnected only when written directions are received from the Owner. The Owner will indemnify and hold Insituform harmless from all claims arising from backups and other effects of such actions or inactions from services not opened at the Owner's request. In the event that

CONTRACTOR LICENSE & REGISTRATION NUMBERS: **AZ** – 277787 **UT** – 6981198-5501 **NM** – 374011
CA – 758411 **OR** – 133115 **WA** – INSITTLL883CW **NV** – 0048110 **HI** – 21894 **AK** – 25830

Insituform® Proposal

Insituform is unable to locate or reconnect a service lateral internally, the Prime Contractor will externally reconnect the service and repair any damages to the CIPP at no cost to Insituform.

PROPOSAL INCLUSIONS

Insituform proposal pricing includes only the items listed below; other items, scope of work, and/or project requirements are not part of the Insituform proposal pricing:

1. Three mobilizations and demobilizations. - based **on a mutually agreeable schedule between Insituform and the Owner and/or Prime Contractor**. The Owner and/or Prime Contractor needs to give Insituform at least one-week advance notice of any changes to the mutually agreed upon schedule. If through no fault of Insituform the mutually agreed upon schedule changes with less than one-week notice, then Insituform may charge the Owner and/or Prime Contractor an additional mobilization charge and any potential lost materials including but not limited to tube already "wetout".
2. Installation of Cured-In-Place Pipe (CIPP) lining per ASTM F1216 Cured-In-Place Pipe Lining including inversion, curing, and finishing. Insituform will utilize a polyester resin system.
3. CCTV, post-installation.
4. Confined space safe entry practices.
5. As specified standard construction warranty.
6. Certificate of insurance with a standard coverage.

PROPOSAL EXCLUSIONS

Insituform proposal pricing excludes all items and scope of work listed below. These items, if needed or found to be applicable, would be provided by Insituform at your additional cost; or would be furnished by others, at your direction, at no cost to Insituform:

1. Bypass Pumping/Dewatering
2. Traffic Control
3. Cleaning and CCTV, pre-installation.
4. 1-year warranty CCTV inspection.
5. Heavy cleaning, mechanical cleaning.
6. T-Lock PVC Liner Repair.
7. Special disposal of cure water.
8. Styrene barrier on CIPP liner
9. Removal/installation of fittings or valves
10. Internal lateral reinstatement.
11. Lateral Sealing – Top Hats by others
12. Manhole installation, rehabilitation, and/or replacement
13. Point repairs, if required, prior to lining
14. Excavation of any kind
15. Filling of annular space between casing and host pipe
16. Water for curing of CIPP installation
17. Trenchless point repairs or sectional liners
18. Removal of manhole cones for access to sewer pipe
19. Manhole bench widenings or throat adjustments
20. Restorations, finished surface repairs (e.g. landscaping, sidewalk, road repairs), or any finished surface repairs required by the owner.
21. Archaeological Monitoring
22. Owner or General Contractor to provide access to water from fire hydrants within a convenient distance from each cleaning and inversion site location
23. *If any hazardous or toxic materials are encountered during the project, the Owner will be responsible for the removal and disposal of the materials.*
24. Any special permits or licenses
25. State and local sales and/or use taxes are excluded, including gross receipts. If the project is exempt, please submit the appropriate documentation.
26. Additional premiums for special insurance coverage(s) required by you or other parties particular to this project.
27. Performance and payment bond not included but is available upon request. If required, please add 2.5% to the total project cost.
28. Stamped Designs.

PROPOSAL TERMS AND CONDITIONS

- a) When CIPP is required to negotiate bends or defects in the host pipe, there is a tendency to experience wrinkling in the installed CIPP around the inside radius of the bend or in the area where the host pipe defect is located. This is normal and should be expected. It will not affect the structural integrity of the finished product. Insituform Technologies, LLC will not be liable for repairs or penalties due to wrinkles in the CIPP at the locations of bends or defects in the host pipe.
- b) Limits of Liability. In consideration of **INSITUFORM TECHNOLOGIES, LLC** agreement to maintain no less than \$5,000,000 of comprehensive general liability insurance in the form required by the Contract, **INSITUFORM TECHNOLOGIES, LLC** liability to the Owner for any matter covered by such insurance will be limited to the extent of such insurance and the Owner will indemnify and hold **INSITUFORM TECHNOLOGIES, LLC** harmless from any third party claims covered by such insurance to the extent such claims exceed the limits of such insurance. Neither party shall be liable to the other for consequential damages relating to the contract. In case of conflict between this provision and any other provision in the Contract as ultimately executed, this provision shall govern and prevail.
- c) LIMITED WARRANTY. IN LIEU OF ALL OTHER EXPRESSED, IMPLIED AND/OR STATUTORY WARRANTIES, INCLUDING WARRANTIES OF MERCHANTABILITY AND FITNESS FOR A PARTICULAR PURPOSE, INSITUFORM AGREES TO CORRECT ANY DEFECTS IN THE MATERIALS OR SERVICES PROVIDED BY INSITUFORM WHICH ARE BROUGHT TO THE ATTENTION OF INSITUFORM WITHIN ONE YEAR FOLLOWING COMPLETION OF INSITUFORM'S WORK, PROVIDED OWNER AFFORDS INSITUFORM SUITABLE ACCESS AND WORKING CONDITIONS TO ACCOMPLISH SUCH CORRECTION.
- d) MUTUAL RELEASE OF CONSEQUENTIAL DAMAGES. Neither party shall be liable to the other for consequential damages relating to or arising out of the Contract.
- e) PROPOSAL SUBJECT TO NEGOTIATION OF OTHER STANDARD TERMS OF AGREEMENT. This proposal is subject to agreement of the parties on other terms and conditions as are customary in contracts of this nature.
- f) If actual quantities differ significantly from those stated in the proposal, then work will be renegotiated.
- g) Payments are due at net within thirty days of invoice. Final payment is due within thirty days of completion of project.
- h) Monthly progress partial payments may be requested for the value of work in progress or completed, including materials secured and on site.
- i) Prices stated are in effect for thirty days from the date of this proposal. The acceptance period may be extended at the sole option of **INSITUFORM TECHNOLOGIES, LLC**.
- j) Conflicts. In case of conflict between the provision of the aforesaid paragraphs and any other provision in the Contract as ultimately executed the provisions as set forth above shall govern and prevail.

OFFERED BY:

ACCEPTED BY:

INSITUFORM TECHNOLOGIES, LLC



BRAD CONDER
COMMERCIAL MANAGER
(801) 472-8472
bconder@azuria.com

ARIZONA OFFICE:

645 SOUTH 24TH STREET
TEMPE, AZ 85282
(480) 446-0620

CONTRACTOR NAME OR ORGANIZATION

SIGNATURE

AUTHORIZED REPRESENTATIVE

TITLE

DATE



2400 W. Medtronic Way, Ste. 1
Tempe, AZ 85281
480.766.8214 tel/
www.sakcon.com

PROPOSAL

FROM: Victor Villalobos **COMPANY:** SAK Construction, LLC **PAGES:** 4

DATE: 07/18/25

RE: CMAR – Chandler, AZ – Loop 101 & 202 Freeway 66" Redundant Sewer – CIPP Scope Only

The following is our unit price quotation for the CIPP portion of the above referenced project:

ITEM NO.	ITEM DESCRIPTION	QTY	U/M	UNIT PRICE	TOTAL PRICE
BASE BID					
1	Mob/Demob	1	LS	\$ 20,000.00	\$ 20,000.00
2	30" CIPP Install	938	LF	\$ 1,165.00	\$ 1,092,770.00
3	Mob/Demob	1	LS	\$ 20,000.00	\$ 20,000.00
4	48" CIPP Install	1,997	LF	\$ 1,185.00	\$ 2,366,445.00
5	Mob/Demob	1	LS	\$ 30,000.00	\$ 30,000.00
6	66" CIPP Install	642	LF	\$ 2,285.00	\$ 1,466,970.00
				TOTAL BASE BID:	\$ 4,996,185.00

***** SAK CONSTRUCTION IS A UNION CONTRACTOR *****

PROPOSAL INCLUSIONS:

1. One mobilization / demobilization to the project site for the CIPP lining crew for each of the diameters to be lined. Any additional mobilizations will be negotiated as extra work.
2. Install approximately 3,577 LF of cured-in-place pipe (CIPP) in the designated 30", 48", and 66" diameter pipelines. The CIPP will utilize a polyester resin system and be installed per ASTM F1216 and the supplied project specifications.
3. No laterals are anticipated in this scope of work.
4. Perform CCTV inspection of the CIPP-lined mainline pipe immediately subsequent to CIPP installation and provide required documentation of the inspection to the General Contractor. Only mainline pipes that have been lined with CIPP by SAK will be inspected.
5. Provide required testing of samples of the installed CIPP at an independent laboratory to confirm conformance with minimum specified physical properties. Sampling methods and frequency will conform to specified parameters.
6. Provide specified 2-year construction warranty on CIPP materials and workmanship.
7. Provide certificate of insurance and endorsements evidencing typical coverage and limits for General Liability (\$1 million / \$2 million), Automobile Liability (\$1 million), Workers' Compensation & Employer's Liability (\$1 million), and if required, Excess Liability (\$5 million). If insurance coverage above and beyond this is required, the General Contractor will be responsible for securing any additional insurance coverage or reimbursing SAK for the costs associated with obtaining the additional insurance.

8. When payment of prevailing wages is applicable, provide the required documentation showing workers on this project have been paid in accordance with the relevant prevailing wage determination for the appropriate laborer classification.

PROPOSAL EXCLUSIONS:

The following items are not included in this proposal, and when required, will be provided by the General Contractor at no cost to SAK. If SAK is required to provide any of these items, the costs will be charged to the General Contractor as extra work.

1. Provide any necessary traffic, parking, &/or pedestrian control. Traffic, parking, &/or pedestrian control systems shall be coordinated to accommodate SAK's operations and placed in a timely manner such that SAK's operations are not impeded or delayed. The General Contractor shall also be responsible for all required traffic control plans.
2. Perform the Clean & TV inspection of the pipe and provide documentation of the inspection to SAK at least 6 weeks before Lining is scheduled. Perform any recleaning necessary in the interim between the initial cleaning and the CIPP installation.
3. Provide any point repairs or other repairs necessary or required to repair or prepare the existing pipeline for CIPP lining.
4. Provide any necessary plugging, damming, diversion, shutdown, and/or bypass pumping of existing pipeline flows, including providing any necessary dewatering of the pipelines, structures, access points, and liner termination points once the plugs, dams, diversion, shutdown, &/or bypass systems are in place and operating. Bypass or diversions systems shall be coordinated to accommodate SAK's operations and placed in a timely manner such that SAK's equipment and personnel are not obstructed by suction & discharge locations, or other components of the diversion or bypass system, and SAK's operations are not impeded or delayed. This shall also include providing any necessary odor control &/or monitoring work.
5. Provide legal and physical access to the pipeline, manholes, structures, and other access points sufficient to allow SAK's operations to be conducted, and to allow large trucks and equipment to access the structures and manholes directly when in streets, or as near as possible for easement locations. When applicable, this shall include locating, exposing, and making accessible any manholes, structures, or other access points, associated with SAK's work, including any necessary alterations to fencing, trees and vegetation, and providing any final restoration once SAK's operations are complete. When necessary, this shall also include providing expanded access by either staging construction of new structures or temporarily altering existing structures where the existing or new structure configuration will impede SAK's operations.
6. Provide any final repair, testing &/or inspection of CIPP-lined pipes when additional work has been performed in or on those pipes by others subsequent to the completion of SAK's work.
7. Provide access to a high-volume water supply at the project site either from the nearest fire hydrants, or other high-volume on-demand sources, as designated by and coordinated with SAK. Access to the water source must be within a mutually agreed upon distance from the installation sites.
8. Provide access to appropriate dump/discharge sites at the project site for disposal of liquids and material collected from the pipelines during pipeline cleaning and lining operations and other waste materials from the project.
9. All top hats or lateral liners. No laterals are anticipated or included in the current pricing.
10. Provide access to a secure yard at or near site for staging and storage of equipment and materials, when available.
11. Provide all required notifications of agencies and other parties affected by the work. This shall include providing any necessary public relations work.
12. Obtain and pay for all special licenses, permits, variances, or other fees required to perform the work. The only exception to this is that SAK will obtain and pay for its own city &/or county business license(s), when required.

13. If the contract is subject to Prime Contracting Transaction Privilege Tax (TPT), the pricing provided above does not include TPT on the total subcontract gross receipts or on the permanent materials to be integrated into the work under this proposal. The General Contractor shall provide the appropriate resale certificate, or other required documentation, verifying that any subcontract resulting from this proposal is exempt from the payment of TPT on the subcontract gross receipts and permanent integrated materials. If appropriate resale or exemption documentation is not provided, TPT will be calculated and added to the subcontract pricing above at the applicable tax rate.
14. Subcontract bonds are not included. If subcontract bonds will be required, they can be provided for an additional fee of 1.5% of the bonded amount.

OTHER CONDITIONS:

1. Work covered by this proposal will be performed under a mutually agreed upon subcontract agreement which incorporates the terms and conditions contained in this proposal either by direct attachment, by insertion, or by reference. In case of conflict, the terms and conditions of this proposal shall prevail. The subcontract agreement shall not require either party to be liable to the other for consequential damages arising out of or relating to the subcontract or contract.
2. If SAK's operations are delayed by the General Contractor or others, then standby costs &/or additional mobilization costs will be charged to the General Contractor accordingly.
3. This proposal is based on the assumption that, even though the pipelines associated with this project will be out of service, they are connected to active systems downstream, such that water discharged in the course of SAK's operations will drain downstream to an active sewer system and not be trapped or surcharged within the system.
4. Normal working hours for installing CIPP are 24-hours a day due to the nature of the product, which requires that once an install is started, it cannot be suspended until it is completed. To the extent that SAK will incur overtime expenses due to this schedule, those costs have been anticipated and are covered in this proposal. However, SAK will not be liable for any penalties, liquidated damages, overtime inspection costs, or other fees that may result from these required working hours.
5. Progress and final payments are due within 30 days of invoice, or when applicable regulations require, whichever is soonest. Release of any withheld retention shall be no later than 30 days from the completion and acceptance of the work covered under this proposal, excluding any subsequent work required in the warranty period, and regardless of whether the General Contractor's retention has been released by the Owner. If retention is not withheld from the General Contractor's payments, then retention shall not be withheld from SAK's payments. Overdue amounts will be subject to interest at a rate of 1.5% monthly, 18% annually.

6. This proposal covers only the scope of work specifically included above. It does not cover performing any cleaning or inspection of pipe or structures not specifically included above; warranty inspections or testing; verification of active/inactive status of laterals; alteration, repair, installation, or rehabilitation of manholes or other structures; abandonment of any pipe or structures; bypassing, plugging, shutdown, damming, diversion, or dewatering of existing pipe flows or structures; odor control &/or odor monitoring work or plans; pedestrian control, parking control, or traffic control; sampling or testing of the installed CIPP liner not specifically included above; sampling or testing of any substances or materials on the project not specifically included above; alteration, testing, sealing, repair, replacement, or rehabilitation of any lateral pipes or lateral connections, aside from any internal reinstatement specifically included above; grouting or repair of obstructions, voids, or defects in the host pipe prior to or after lining; UV cured fiberglass point repairs (UVPR) or cured-in-place sectional liners (CIPSL); top hat type service connection seals, unless specifically included above; mandrel testing or other similar testing of the pipe; or any work requiring excavation. This proposal also does not cover providing any field yards, meeting facilities, or office facilities for the project; phones, cameras, computer software, or computer equipment for the use of the General Contractor or Owner; photos or other documentation for record of pre- or post-construction site conditions; tree or vegetation clearing, trimming, removal, protection or restoration; site grading; snow &/or ice removal; handling, transportation, or disposal of hazardous materials; surveying or staking; engineered plans, designs, or drawings, with the only exception being if the CIPP specification specifically requires engineered CIPP thickness designs, then those designs will be provided; preparation or implementation of any pollution control or environmental protection programs or plans; environmental stewardship; street sweeping; security; fire control truck and operator; fencing of any kind; erosion or sedimentation control; community relations or public relations work; project signs or advance notification signs; and any necessary archaeological, biological, or ground settlement / movement monitoring and mitigation. If any of this type of work is required to be performed by SAK, it will be considered to be extra work and charged accordingly.
7. For the purposes of this proposal, the General Contractor shall be defined to be the entity contracting with SAK to perform the work covered under this proposal.
8. This proposal is valid for 90 days from the date of submission as indicated above.



Bragg
PO Box 727
Long Beach, CA 90801
(562) 984-2400 Ph.
(562) 984-2467 Fax

Quote Number:	P-12550	Quoted out of:	Phoenix
Date of Quotation:	7/18/2025	Bragg Contact:	Cerny, Paul
		Phone Number:	(602) 284-2546
		E-Mail:	paul.cerny@braggcrane.com
Customer:	Bragg Crane & Rigging - PH	Quoted To:	Benjamin Esquivel
Address:	2816 N 27th St Phoenix, AZ 85009	Phone Number:	(602) 233-0205
		E-Mail:	
Jobsite Name:	Price Road & 202	Start Date:	
Jobsite Address:		End Date:	
		Quotation Status:	Quoted to Customer

Work to be performed: Set Precast

Lift Information:

Heaviest Pick	160000	Radius	60	Height	0
Up		In		Set Back Fr. Bldg.	

Equipment Required:

<u>Description</u>	<u>Est. Qty</u>	<u>Rate</u>	<u>Est. Amt</u>
Crane Mobilization Per Trip	1.00 Each	\$12,750.00	\$12,750.00
550 Ton Hydraulic Truck Crane Per Day- 8 Hour Minimum	1.00 Day	\$7,480.00	\$7,480.00
Crane Demobilization Per Trip	1.00 Each	\$12,750.00	\$12,750.00
Surcharge	1.00	10.00%	\$3,298.00
Quote Estimate:			\$36,278.00

Comments: This quote is contingent on a clear working and level working area. Any hours worked over 8-hours \$50.00 per man, per hour will be added for overtime. A firm and level set up area for the crane must be provided. There will be a 10% fuel surcharge on the invoice total

*****Issuance of a purchase order acknowledges acceptance of this quotation**



Bragg
PO Box 727
Long Beach, CA 90801
(562) 984-2400 Ph.
(562) 984-2467 Fax

Quote Number:	P-12549	Quoted out of:	Phoenix
Date of Quotation:	7/18/2025	Bragg Contact:	Cerny, Paul
		Phone Number:	(602) 284-2546
		E-Mail:	paul.cerny@braggcrane.com
Customer:	Bragg Crane & Rigging - PH	Quoted To:	Benjamin Esquivel
Address:	2816 N 27th St Phoenix, AZ 85009	Phone Number:	(602) 233-0205
		E-Mail:	
Jobsite Name:	Price Road & 202	Start Date:	
Jobsite Address:		End Date:	
		Quotation Status:	Quoted to Customer

Work to be performed: Set Precast

Lift Information:

Heaviest Pick	60000	Radius	30	Height	0
Up		In		Set Back Fr. Bldg.	

Equipment Required:

<u>Description</u>	<u>Est. Qty</u>	<u>Rate</u>	<u>Est. Amt</u>
Crane Mobilization Per Trip- Sewer	1.00 Each	\$1,890.00	\$1,890.00
190 Ton Hydraulic Truck Crane Per Day- 8 Hour Minimum- Sewer	1.00 Day	\$5,478.00	\$5,478.00
Crane Demobilization Per Trip- Sewer	1.00 Each	\$1,890.00	\$1,890.00
Crane Mobilization Per trip-pumps	1.00 Each	\$650.00	\$650.00
90 Ton Hydraulic Truck Crane Per Day- 8 Hours-Pumps	1.00 Day	\$3,850.00	\$3,850.00
Crane Demobilization Per Trips- Pumps	1.00 Each	\$650.00	\$650.00
Surcharge	1.00	10.00%	\$1,440.80
Quote Estimate:			\$15,848.80

Comments: This quote is contingent on a clear working and level working area. Any hours worked over 8-hours \$50.00 per man, per hour will be added for overtime. A firm and level set up area for the crane must be provided. There will be a 10% fuel surcharge on the invoice total

*****Issuance of a purchase order acknowledges acceptance of this quotation**

DAVIT CRANE QUOTE



Engineered Solutions. Smart Infrastructure.

JAMES, COOKE & HOBSON, INC.

3501 E Broadway Rd, Phoenix, AZ.85040

Phone: 602-586-1439

Fax: 602-276-5402

E-mail egm@jchinc.com

MARCH 21, 2025

To: B&F CONTRACTING

Quotation #: 032125-1EGM

Attn: TRAVIS MONK

Ph:

Job Name: **CHANDLER CMAR 66**

INCH REDUNDANT SEWER

Location: CHANDLER, AZ

Quotation by: ED MARTIN

Unless otherwise stated: Prices are firm for 30 days from bid date, payment terms are NET 30 DAYS from shipment. Interest shall accrue at 1.5% per month on past due amounts per month. Any taxes are additional. Items included are only listed below. Please review carefully

QTY	DESCRIPTION	TOTAL
2	5PT20G-E2 Commander Series 2000 lbs. / 900 kg maximum capacity portable davit crane with a 4WP2-K power winch. Crane includes: - Handle for easy 360° rotation with Nylatron flange bearing - Adjust boom angle with ratchet style screw jack finished with Galvanized Protective Coating - Adjust boom reach with boom extension - Quick connect winch mounting plate with single clevis pin connection - Welded constructed structural steel pipe and tubing - Hot Dipped Galvanized finish - Stainless Steel assembly hardware Winch Includes: - 1.3 HP TENV, 115-1-60 motor - Automatic internal load brake - 6-foot NEMA 4 pendant control and an 8-foot power cord with grounded plug - Line Speed range from 8 – 13 fpm - Frame constructed of mild steel - Gear reducer and drum constructed of cast aluminum - Pressure plate with corrosion-resistant trivalent zinc finish - Red Enamel finish - Slot for swaged ball end on wire rope assembly to allow for Quick Disconnect option. This feature allows you to keep the hook end of the wire rope assembly attached to the load at all times, disconnect the wire rope from the winch and attach the swaged ball end to a Wire Rope Keeper (RK19-25S) or Cable Reel (RW50). - Quick disconnect option allows crane to be moved to another base location - Stainless Steel fasteners to mount winch to crane	
2	5BE20-15G Commander Series 2000 Base Extension - 15" / 38cm long - Available for use with the 5PT20 series davit crane and base configuration - Hot Dipped Galvanized finish Use with flush or wall mount base to maintain crane height. Use with pedestal base if additional height is required. Bases available in various finishes.	
5	5BF20G Commander Series 2000 (5PT20) Flush Mount Base - Base cover with lanyard - Hot Dipped Galvanized finish	

CONT.

5 AN62A-6S

Expansion bolt anchor kit
Includes four bolts
ANCHOR DIAMETER: 5/8"
ANCHOR LENGTH: 6"
304 STAINLESS STEEL

5 WS25-45NS

1/4" diameter 7x19 aircraft cable assembly
- 304 Stainless Steel
- 45 feet long
- Swaged ball one end
- 316 SST Swivel hook with safety latch other end
- Ships loose with order

THE TOTAL NET PRICE F.O.B. FACTORY WITH FREIGHT ALLOWED TO THE JOB SITE IS...\$20,517.00

NOTES:

- The above price is firm for 30 days. After such time it may be subject to review. Shipment has been estimated at Approx. 4-6 weeks after receipt of purchase order and/or approved submittal drawings.
- Price includes: Freight to the job site off loaded by-others. Submittal drawings and start-up supervision.
- Prices do not include: Sales tax
- Thank you for this opportunity to be of service. If you have any questions regarding this quotation or any other matter, Please do not hesitate to call.

Fibergrate Composite Structures Inc

1234 Johnson Road, Suite 366 ~ Allen, TX 75013 USA

T: 800-527-4043 ~ F: 972-250-1530



Quote No.91219	Quote Type: Fab Per Approval	(original)
Bid Type: Budget		

Customer:	B&F Contracting, Inc	Date: July 18, 2025
Address:	11011 North 23rd Avenue	Phone: 623-582-1170
	Phoenix, Arizona 85029	Fax:
	United States	
Attention:	Travis Monk	Cell:
	tmonk@bfcontracting.com	
	623-582-1170	

Opportunity: City of Chandler Pre-cast
Location: Chandler, AZ 85248

Fibergrate appreciates the opportunity to bid on your FRP requirement, and offers the following quotation for your consideration. Fibergrate is an ISO-9001:2015 certified manufacturer and is one of the original pioneers in the FRP fabrication industry, having invented fiberglass molded grating in 1965. With more than 50 years of experience in FRP fabrications, Fibergrate has the experience and capabilities necessary to design, manufacture, and fabricate the materials for your project to ensure that the assembly at the jobsite will proceed smoothly. Fibergrate continues its commitment as a leader in the FRP market. All of the FRP products and solutions offered in this quotation are heavy metal safe. If you have any questions about heavy metals in FRP products or how they can affect your business, our communities and our environment, please go to our website (www.fibergrate.com) or contact your local Fibergrate Representative.

All material take-offs were derived from drawing M-2.0, M-2.1, M-2.2, M-3.0, M-3.1, M-3.2, M-3.3, M-3.4, M-3.5, M3.6, S1.2, S-3.0, S-301 revision 0 dated February 2025. Specification 06611 was provided with request for quote. Quoted FRP materials area based off e-mail received on 02/20/2025. All quoted FRP materials will be per the design by the engineering department and are based of a 500 pound concentrated loading.

MATERIALS of CONSTRUCTION:

Vi-Corr® 1 1/2" deep 1 1/2" square mesh molded grating with meniscus surface, color dark gray.
Dynaform® VEFR structural shapes, color beige.
316 Stainless Steel Hardware.

SCOPE of WORK:

Fibergrate® will provide the following:

Drawing M-2.0 revision 0 dated February 2025:

Junction Structure B:

One (1) area of 1 1/2" deep 1 1/2" square mesh molded grating with a non-slip meniscus surface. This area to be approximately 902 square feet. Grating supports to be FRP structural beams, all FRP to FRP structural connections to be made using 316 stainless steel hardware.. Beams to rest on concrete except for areas where spanning over open channels. Channel opening range from 6'-0" to 6'-8". Grating to attach to the FRP supports using Fibergrate's standard 316 stainless steel grating hold down clip assemblies. All grating pieces to be cut down to 24" wide or less to fit through a man-hole.

Drawing M-3.0 revision 0 dated February 2025:

Junction Structure C:

One (1) area of 1 1/2" deep 1 1/2" square mesh molded grating with a non-slip meniscus surface. This area to be approximately 200 square feet. Grating supports to be FRP structural beams, all FRP to FRP structural connections to be made using 316 stainless steel hardware.. Beams to rest on concrete except for areas where spanning over open channels. Channel opening to be approximately 5'-10". Grating to attach to the FRP supports using Fibergrate's standard 316 stainless steel grating hold down clip assemblies. All grating pieces to be cut down to 24" wide or less to fit through a man-hole. This area includes a 2'-0" (square) section of removable grating.

Drawing M-3.4 revision 0 dated February 2025:

Junction Structure E:

One (1) area of 1 1/2" deep 1 1/2" square mesh molded grating with a non-slip meniscus surface. This area to be approximately 200 square feet. Grating supports to be FRP structural beams, all FRP to FRP structural connections to be made using 316 stainless steel hardware.. Beams to rest on concrete except for areas where spanning over open channels. Channel opening to be approximately 4'-10". Grating to attach to the FRP supports using Fibergrate's standard 316 stainless steel grating hold down clip assemblies. All grating pieces to be cut down to 24" wide or less to fit through a man-hole. This area includes a 2'-0" (square) section of removable grating.

Special Notes:

- 1.) This quote has been prepared using non-domestic bolts, nuts, and washers.
- 2.) If US domestic hardware is required contact Fibergrate Composite Structures for a revised quote using US domestic hardware.

Special Notes:

NOTES:

Fibergrate reserves the right to a final review of it's quotation prior to accepting a purchase order.

This quotation allows for three (3) sets of Approval & Erection drawings, additional drawings are available at an additional cost of \$5.00 per sheet.

Fibergrate's standard fabrication and manufacturing tolerances will apply as follows:

- 1.) Structural shapes & plate = +/- 1/8"; Cut grating = + 1/8" / - 5/16".
- 2.) Cut grating may have open or unequal stub bars.
- 3.) No specifications for design/products were provided with request for quote.
- 4.) No design calculations are included with the quotation.
- 5.) Design calculations are available with projects requiring approval drawings at an additional charge.
- 6.) Fibergrate Composite Structures has a manufacturing facility in Stephenville, TX.
- 7.) Grating will be attached to the structure using provided hold down clip assemblies.
- 8.) This quotation is based upon customer providing AutoCad drawings to Fibergrate before approval drawings can be started.
- 9.) All grating notches/penetrations shown on the contract drawing are included in this quotation.
- 10.) All grating notches/penetrations not shown can be provided at additional cost.
- 11.) Hold-down clips/anchors are available but not included in this quotation unless specifically referenced.
- 12.) FRP platforms and other structural fabrications will require assembly.
- 13.) Visit www.fibergrate.com for Company history, literature, catalogs, specifications and product information.

EXCLUSIONS:(to be provided by others)

Anchor bolts (including concrete screws used in conjunction with grating hold down clips), installation, field measurements and any other items not specifically included in the above described "Scope of Work" are to be provided by others.

PRICING:

Lump Sum Sales Price:

(USD)

Freight Included in Total Quoted Price for delivery to zip code 85248.

Multiple shipments will result in additional charges to be determined at time of request. Executed change orders will be required prior to release of materials for shipment.

Terms of Delivery: FOB Point of Origin

QUOTE VALIDITY: This quotation is firm for acceptance within thirty (30) days from the Bid Date.

ESTIMATED DELIVERY:

(Lead times to be determined upon receipt of a purchase order.)

Fibergrate Territory Manager:

Ben Loth

E-Mail: ben.loth@fibergrate.com

Tel: 480-851-8541

Fibergrate Field Sales Manager:

West USA House, Greg Harris

E-Mail: greg.harris@fibergrate.com

Tel: 480-695-2052

This quotation was prepared by:

KW

Terms of Payment: net 30

Terms of Delivery:

Fibergrate Composite Structures terms and conditions govern this quote. [x] See attached.

**FOSTER ENGINEERED
PRODUCTS SELECTED AS
LOWEST BIDDER FOR ALL
PROJECT HATCHES**

an Amesbury **Truth** company

The Bilco Company

P.O. Box 1203
New Haven, CT. 06505
(203) 934-6363, Email cs@bilco.com

QUOTE

Date	Quote	Pg
07/17/25	480327	1

THIS QUOTE WILL EXPIRE ON 08/31/25

Cust # 542574

FOSTER ENGINEERED PRODUCTS
1819 E MORTEN AVE
STE 200
PHOENIX AZ 85020
RICHARD
Phone 602-265-0127

Fax 602-266-0455

City of Chandler CMAR 66-

Email jen@fosterengineered.com

Qty	Product	Description	Weight Pounds	Unit Price	Extension US Dollars
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<Main Products>

1	JD-SS-H20	BILCO FLOOR/VAULT/SIDEWALK DOOR 4' 0.00 X 7' 0.00 (1220mm X 2135mm) HINGED ON 7' 0.00 (2135mm) SIDE Description 1/4" (6.35mm) DIAMOND PATTERN DBL COVE Description DRAINAGE FRAME W/ DRAIN COUPLING Live Load H-20 WHEEL LOAD (OFF-STREET LOCATIONS) Lock STAINLESS STEEL SLAM LOCK Finish MILL FINISH W/ELECTROCHEMICAL PASSIVATION OF WELDS Hdwe 316 STAINLESS STEEL HARDWARE BILCO PART # XJD095001 HINGED ON THE 7'0" SIDES. MANUFACTURED WITH TYPE 316 STAINLESS STEEL, 1/4" DIAMOND PLATE COVERS, 1/4" SMOOTH PLATE FRAME, TYPE 316 STAINLESS STEEL HARDWARE, MILL FINISH WITH ELECTROCHEMICAL PASSIVATION OF WELDS REINFORCED FOR H-20 LOADING. MODIFIED WITH RECESSED PADLOCK HASP.	1528	15037.65	15037.65 NET
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1	PARTS SW	FALL PROTECTION FALL PROTECTION FOR JD-SS-H20 4'X7' UNIT.	70	814.05	814.05 NET
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3	J-SS H20	BILCO FLOOR/VAULT/SIDEWALK DOOR 2' 0.00 X 7' 0.00 (609mm X 2135mm) HINGED ON 7' 0.00 (2135mm) SIDE Description 1/4" (6.35mm) DIAMOND PATTERN COVER Description DRAINAGE FRAME W/ DRAIN COUPLING Live Load H-20 WHEEL LOAD (OFF-STREET LOCATIONS) Lock STAINLESS STEEL SLAM LOCK Finish MILL FINISH W/ELECTROCHEMICAL PASSIVATION OF WELDS Hdwe 316 STAINLESS STEEL HARDWARE BILCO PART # XJ0095001 HINGED ON THE 7'0" SIDE. MANUFACTURED WITH TYPE 316 STAINLESS STEEL, 1/4" DIAMOND PLATE COVER, 1/4" SMOOTH PLATE FRAME, TYPE 316 STAINLESS STEEL HARDWARE, MILL FINISH WITH ELECTROCHEMICAL PASSIVATION OF WELDS REINFORCED FOR H-20 LOADING. MODIFIED WITH RECESSED PADLOCK HASP.	860	8039.25	24117.75 NET
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The Bilco Company

P.O. Box 1203
New Haven, CT. 06505
(203) 934-6363, Email cs@bilco.com

QUOTE

Date	Quote	Pg
07/17/25	480327	2

THIS QUOTE WILL EXPIRE ON 08/31/25

Cust # 542574

FOSTER ENGINEERED PRODUCTS
1819 E MORTEN AVE
STE 200
PHOENIX AZ 85020
RICHARD
Phone 602-265-0127

Fax 602-266-0455

City of Chandler CMAR 66-

Email jen@fosterengineered.com

Qty	Product	Description	Weight Pounds	Unit Price	Extension US Dollars
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3	PARTS SW	FALL PROTECTION	37	421.20	1263.60
					NET

FALL PROTECTION FOR J-SS H20 2'X7' UNIT.

** ABOVE PRICE IS NET FOR ALL TYPES OF CUSTOMERS. MAXIMUM DISCOUNT APPLIED.

FOB: FACTORY

FABRICATION TIME:

DUE TO MARKET CONDITIONS RELATED TO MATERIAL AVAILABILITY, LEAD TIME TO BE DETERMINED WHEN PURCHASE ORDER IS RELEASED FOR FABRICATION.

**ADD TO ABOVE PRICE FOR FREIGHT TO PHOENIX, AZ 85029: \$1,502.81
CHR LTL

<Terms>

ALL SALES BY THE BILCO COMPANY ARE GOVERNED BY TERMS AND CONDITIONS ATTACHED TO THIS QUOTATION.
PLEASE MAKE P.O. OUT TO THE BILCO COMPANY AS A SUPPLIER WITH NO RETAINAGE

<Taxes>

Above Pricing Does NOT Include Sales Tax. Sales Tax WILL BE CHARGED Based On The SHIPPING DESTINATION.
IF TAX EXEMPT, PLEASE FORWARD EXEMPTION CERTIFICATE WITH YOUR ORDER

<Exceptions>

Fall Protection Grating System consists of aluminum panel(s) with stain less steel hardware (hinges, hold open latch, and all fasteners). Padlock lug provided for security (padlock by others).
Fall protection system meets OSHA 29 CFR 1926.502(c)

Stainless Steel material is subject to prior sale. Please consult factory at time of order for material availability.

Important Note: The above doors are suitable for use in off-street locations where not subjected to high density, fast moving traffic

Support required under frame and shelf -designed and installed by others in the field.



The Bilco Company

P.O. Box 1203
New Haven, CT. 06505
(203) 934-6363, Email cs@bilco.com

QUOTE

Date	Quote	Pg
07/17/25	480327	3

THIS QUOTE WILL EXPIRE ON 08/31/25

Cust # 542574

FOSTER ENGINEERED PRODUCTS
1819 E MORTEN AVE
STE 200
PHOENIX AZ 85020
RICHARD
Phone 602-265-0127

Fax 602-266-0455

City of Chandler CMAR 66-

Email jen@fosterengineered.com

Qty	Product	Description	Weight Pounds	Unit Price	Extension US Dollars
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If Units Not Released To Fabrication By 08/31/25 Pricing Is Subject To Review

<Notes>

Freight is subject to additional charges for appointment/notification, delivery to construction sites, limited access sites, residential deliveries, lift gates and excessive pallet length (greater than 8').

Price is FOB factory and does not include tax or freight.

Any change or addition to above items as quoted will result in a price change.

If an order is placed for the items quoted above, lead time will be reviewed and may change based on plant capacity & schedule and/or our material suppliers lead times at the time of the order.

Price may be subject to change pending review of specifications and/or details.

Note: Pricing is based on all units being shipped at the same time to the same location.

If we are favored with an order, it would be necessary for us to furnish drawings for architectural approval prior to fabrication.

If Units Not Released To Fabrication By 08/31/25 Pricing Is Subject To Review

PLEASE ADVISE IF THE BUY AMERICAN ACT, THE BUY AMERICA ACT, OR THE AMERICAN RECOVERY AND REINVESTMENT ACT APPLIES TO THIS PROJECT

In the event that there are product or project details that you would like to discuss further, you may also contact your local BILCO representative (noted below) Each of our representatives has been factory trained and is fully qualified to answer any questions that you may have.

FOSTER ENGINEERED PRODUCTS
1819 E MORTEN AVE
STE 200
PHOENIX AZ 85020
Phone 602-265-0127
Fax 602-266-0455



The Bilco Company
P.O. Box 1203
New Haven, CT. 06505
(203) 934-6363, Email cs@bilco.com

QUOTE

Date	Quote	Pg
07/17/25	480361	1

THIS QUOTE WILL EXPIRE ON 08/31/25

Cust # 542574

FOSTER ENGINEERED PRODUCTS
1819 E MORTEN AVE
STE 200
PHOENIX AZ 85020
JEN FOSTER
Phone 602-265-0127

Fax 602-266-0455

JUNCTION STRUCTURE C

Email jen@fosterengineered.com

Qty	Product	Description	Weight Pounds	Unit Price	Extension US Dollars
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<Main Products>

1	J-SS H20	BILCO FLOOR/VAULT/SIDEWALK DOOR 3' 0.00 X 6' 8.00 (914mm X 2030mm) HINGED ON 6' 8.00 (2030mm) SIDE Description 1/4" (6.35mm) DIAMOND PATTERN COVER Description DRAINAGE FRAME W/ DRAIN COUPLING Live Load H-20 WHEEL LOAD (OFF-STREET LOCATIONS) Lock STAINLESS STEEL SLAM LOCK Finish MILL FINISH W/ELECTROCHEMICAL PASSIVATION OF WELDS Hdwe 316 STAINLESS STEEL HARDWARE BILCO PART # XJ0095001 HINGED ON THE 6'8" SIDE. MANUFACTURED WITH TYPE 316 STAINLESS STEEL, 1/4" DIAMOND PLATE COVER, 1/4" SMOOTH PLATE FRAME, TYPE 316 STAINLESS STEEL HARDWARE, MILL FINISH WITH ELECTROCHEMICAL PASSIVATION OF WELDS. REINFORCED FOR H-20 LOADING. MODIFIED WITH RECESSED PADLOCK HASP. 8" CHANNEL UNDER FRAME.	1347	13571.55	13571.55 NET
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1	PARTS SW	FALL PROTECTION FALL PROTECTION FOR J-SS H20 3'X6'8" UNIT.	49	486.00	486.00 NET
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2	J-SS H20	BILCO FLOOR/VAULT/SIDEWALK DOOR 2' 0.00 X 7' 0.00 (609mm X 2135mm) HINGED ON 7' 0.00 (2135mm) SIDE Description 1/4" (6.35mm) DIAMOND PATTERN COVER Description DRAINAGE FRAME W/ DRAIN COUPLING Live Load H-20 WHEEL LOAD (OFF-STREET LOCATIONS) Lock STAINLESS STEEL SLAM LOCK Finish MILL FINISH W/ELECTROCHEMICAL PASSIVATION OF WELDS Hdwe 316 STAINLESS STEEL HARDWARE BILCO PART # XJ0095001 HINGED ON THE 7'0" SIDE. MANUFACTURED WITH TYPE 316 STAINLESS STEEL, 1/4" DIAMOND PLATE COVER, 1/4" SMOOTH PLATE FRAME, TYPE 316 STAINLESS STEEL HARDWARE, MILL FINISH WITH ELECTROCHEMICAL PASSIVATION OF WELDS. REINFORCED FOR H-20 LOADING. MODIFIED WITH RECESSED PADLOCK HASP.	860	8063.55	16127.10 NET
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an AmesburyTruth™ company

The Bilco Company

P.O. Box 1203
New Haven, CT. 06505
(203) 934-6363, Email cs@bilco.com

QUOTE

Date	Quote	Pg
07/17/25	480361	2

THIS QUOTE WILL EXPIRE ON 08/31/25

Cust # 542574

FOSTER ENGINEERED PRODUCTS
1819 E MORTEN AVE
STE 200
PHOENIX AZ 85020
JEN FOSTER
Phone 602-265-0127

Fax 602-266-0455

JUNCTION STRUCTURE C

Email jen@fosterengineered.com

Qty	Product	Description	Weight Pounds	Unit Price	Extension US Dollars
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2	PARTS SW	FALL PROTECTION	37	421.20	842.40
					NET

FALL PROTECTION FOR J-SS H20 2'X7' UNIT

**ABOVE PRICE IS NET FOR ALL TYPES OF CUSTOMERS. MAXIMUM DISCOUNT APPLIED.

FOB: FACTORY

FABRICATION TIME:

DUE TO MARKET CONDITIONS RELATED TO MATERIAL AVAILABILITY, LEAD TIME TO BE DETERMINED WHEN PURCHASE ORDER IS RELEASED FOR FABRICATION.

**ADD TO ABOVE PRICE FOR FREIGHT TO EL PASO, TX 79930: \$1,309.18
CHR LTL

<Terms>

ALL SALES BY THE BILCO COMPANY ARE GOVERNED BY TERMS AND CONDITIONS ATTACHED TO THIS QUOTATION.
PLEASE MAKE P.O. OUT TO THE BILCO COMPANY AS A SUPPLIER WITH NO RETAINAGE

<Taxes>

Above Pricing Does NOT Include Sales Tax. Sales Tax WILL BE CHARGED Based On The SHIPPING DESTINATION.
IF TAX EXEMPT, PLEASE FORWARD EXEMPTION CERTIFICATE WITH YOUR ORDER

<Exceptions>

Fall Protection Grating System consists of aluminum panel(s) with stain less steel hardware (hinges, hold open latch, and all fasteners). Padlock lug provided for security (padlock by others).
Fall protection system meets OSHA 29 CFR 1926.502(c)

Stainless Steel material is subject to prior sale. Please consult factory at time of order for material availability.

Important Note: The above doors are suitable for use in off-street locations where not subjected to high density, fast moving traffic

Support required under frame and shelf -designed and installed by



QUOTE

The Bilco Company

P.O. Box 1203
New Haven, CT. 06505
(203) 934-6363, Email cs@bilco.com

Date	Quote	Pg
07/17/25	480361	3

THIS QUOTE WILL EXPIRE ON 08/31/25

Cust # 542574

FOSTER ENGINEERED PRODUCTS
1819 E MORTEN AVE
STE 200
PHOENIX AZ 85020
JEN FOSTER
Phone 602-265-0127

Fax 602-266-0455

JUNCTION STRUCTURE C

Email jen@fosterengineered.com

Qty	Product	Description	Weight Pounds	Unit Price	Extension US Dollars
-----	---------	-------------	------------------	---------------	-------------------------

others in the field.

If Units Not Released To Fabrication By 08/31/25 Pricing Is Subject To Review

<Notes>

If we are favored with an order, it would be necessary for us to furnish drawings for architectural approval prior to fabrication.

Freight is subject to additional charges for appointment/notification, delivery to construction sites, limited access sites, residential deliveries, lift gates and excessive pallet length (greater than 8').

Price is FOB factory and does not include tax or freight.

Any change or addition to above items as quoted will result in a price change.

If an order is placed for the items quoted above, lead time will be reviewed and may change based on plant capacity & schedule and/or our material suppliers lead times at the time of the order.

Price may be subject to change pending review of specifications and/or details.

Note: Pricing is based on all units being shipped at the same time to the same location.

If Units Not Released To Fabrication By 08/31/25 Pricing Is Subject To Review

PLEASE ADVISE IF THE BUY AMERICAN ACT, THE BUY AMERICA ACT, OR THE AMERICAN RECOVERY AND REINVESTMENT ACT APPLIES TO THIS PROJECT



an AmesburyTruth™ company

QUOTE

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Qty	Product	Description	Weight Pounds	Unit Price	Extension US Dollars
-----	---------	-------------	------------------	---------------	-------------------------

In the event that there are product or project details that you would like to discuss further, you may also contact your local BILCO representative (noted below)
Each of our representatives has been factory trained and is fully qualified to answer any questions that you may have.

FOSTER ENGINEERED PRODUCTS
1819 E MORTEN AVE
STE 200
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The Bilco Company

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QUOTE

Date	Quote	Pg
07/17/25	480360	1

THIS QUOTE WILL EXPIRE ON 08/31/25

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1819 E MORTEN AVE
STE 200
PHOENIX AZ 85020
JEN FOSTER
Phone 602-265-0127

Fax 602-266-0455

JUNCTION STRUCTURE E

Email jen@fosterengineered.com

Qty	Product	Description	Weight Pounds	Unit Price	Extension US Dollars
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<Main Products>

1	J-SS H20	BILCO FLOOR/VAULT/SIDEWALK DOOR 3' 0.00 X 6' 8.00 (914mm X 2030mm) HINGED ON 6' 8.00 (2030mm) SIDE Description 1/4" (6.35mm) DIAMOND PATTERN COVER Description DRAINAGE FRAME W/ DRAIN COUPLING Live Load H-20 WHEEL LOAD (OFF-STREET LOCATIONS) Lock STAINLESS STEEL SLAM LOCK Finish MILL FINISH W/ELECTROCHEMICAL PASSIVATION OF WELDS Hdwe 316 STAINLESS STEEL HARDWARE BILCO PART # XJ0095001 HINGED ON THE 6'8" SIDE. MANUFACTURED WITH TYPE 316 STAINLESS STEEL, 1/4" DIAMOND PLATE COVER, 1/4" SMOOTH PLATE FRAME, TYPE 316 STAINLESS STEEL HARDWARE, MILL FINISH WITH ELECTROCHEMICAL PASSIVATION OF WELDS. REINFORCED FOR H-20 LOADING. MODIFIED WITH RECESSED PADLOCK HASP. 8" CHANNEL UNDER FRAME.	1347	16755.00	16755.00
---	----------	--	------	----------	----------

1	PARTS SW	FALL PROTECTION FALL PROTECTION FOR J-SS H20 3'X6'8" UNIT.	49	600.00	600.00
---	----------	---	----	--------	--------

FOB: FACTORY

FABRICATION TIME:

***DUE TO MARKET CONDITIONS RELATED TO MATERIAL AVAILABILITY, LEAD TIME
TO BE DETERMINED WHEN PURCHASE ORDER IS RELEASED FOR FABRICATION.***

<Freight>

FREIGHT TO 79930	EL PASO TX FROM ZIP 72472	653.66
------------------	---------------------------	--------

Freight is based on units and quantities as quoted above and subject to review based on method of shipment. DOES NOT INCLUDE ALTERNATES.

<Terms>

ALL SALES BY THE BILCO COMPANY ARE GOVERNED BY TERMS AND CONDITIONS ATTACHED TO THIS QUOTATION.

PLEASE MAKE P.O. OUT TO THE BILCO COMPANY AS A SUPPLIER WITH NO RETAINAGE

<Taxes>

Above Pricing Does NOT Include Sales Tax. Sales Tax WILL BE CHARGED

Based On The SHIPPING DESTINATION.

IF TAX EXEMPT, PLEASE FORWARD EXEMPTION CERTIFICATE WITH YOUR ORDER



QUOTE

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New Haven, CT. 06505
(203) 934-6363, Email cs@bilco.com

Date	Quote	Pg
07/17/25	480360	2

THIS QUOTE WILL EXPIRE ON 08/31/25

Cust # 542574

FOSTER ENGINEERED PRODUCTS
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STE 200
PHOENIX AZ 85020
JEN FOSTER
Phone 602-265-0127

Fax 602-266-0455

JUNCTION STRUCTURE E

Email jen@fosterengineered.com

Qty	Product	Description	Weight Pounds	Unit Price	Extension US Dollars
-----	---------	-------------	------------------	---------------	-------------------------

<Exceptions>

Fall Protection Grating System consists of aluminum panel(s) with stainless steel hardware (hinges, hold open latch, and all fasteners). Padlock lug provided for security (padlock by others). Fall protection system meets OSHA 29 CFR 1926.502(c)

Stainless Steel material is subject to prior sale. Please consult factory at time of order for material availability.

Important Note: The above doors are suitable for use in off-street locations where not subjected to high density, fast moving traffic

Support required under frame and shelf -designed and installed by others in the field.

If Units Not Released To Fabrication By 08/31/25 Pricing Is Subject To Review

<Notes>

Freight is subject to additional charges for appointment/notification, delivery to construction sites, limited access sites, residential deliveries, lift gates and excessive pallet length (greater than 8').

Prices quoted are list.

Price is FOB factory and does not include tax or freight.

Any change or addition to above items as quoted will result in a price change.

If an order is placed for the items quoted above, lead time will be reviewed and may change based on plant capacity & schedule and/or our material suppliers lead times at the time of the order.

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an AmesburyTruth™ company

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Qty	Product	Description	Weight Pounds	Unit Price	Extension US Dollars
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If Units Not Released To Fabrication By 08/31/25 Pricing Is Subject To Review

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In the event that there are product or project details that you would like to discuss further, you may also contact your local BILCO representative (noted below)
Each of our representatives has been factory trained and is fully qualified to answer any questions that you may have.

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STE 200
PHOENIX AZ 85020
Phone 602-265-0127
Fax 602-266-0455
Email jen@fosterengineered.com



Engineered Solutions. Smart Infrastructure.

JAMES, COOKE & HOBSON, INC.

3501 E Broadway Rd, Phoenix, AZ.85040

Phone: 602-586-1439

Fax: 602-276-5402

E-mail egm@jchinc.com**MARCH 18, 2025**

To: B&F CONTRACTING

Quotation #: 031825-1EGM

Attn: TRAVIS MONK

Job Name: **CITY OF CHANDLER
CMAR 66" SEWER**

Ph:

Location: CHANDLER , AZ

Quotation by: ED MARTIN

Unless otherwise stated: Prices are firm for 30 days from bid date, payment terms are NET 30 DAYS from shipment. Interest shall accrue at 1.5% per month on past due amounts per month. Any taxes are additional. **Items included are only listed below.** Please review carefully

QTY	DESCRIPTION	TOTAL
1	STAINLESS STEEL TYPE 316 AHD 48" X 84" A-NGLE FRAME, H-20 OCCASIONAL LOADING, D-DOUBLE DOOR. OPTIONS INCLUDED: OPEN SPRINGS, SLAMLOCK, OVERSIZED RECESSED PADLOCK, SKIRT FOR 8" TOP SLAB, MILL FINISH.	\$20,848.00
2	STAINLESS STEEL TYPE 316 AHD 36" X 80" A-NGLE FRAME, H-20 OCCASIONAL LOADING, D-DOUBLE DOOR. OPTIONS INCLUDED: OPEN SPRINGS, SLAMLOCK, OVERSIZED RECESSED PADLOCK, SKIRT FOR 8" TOP SLAB, MILL FINISH.	\$36,642.00
3	316SS FRAME AND COVER 24" X 84" FRAME OPENING BOLTED COVER MADE OUT OF 1/4" THICK DIAMOND PLATES FRAME MADE OUT 3 X 3 X 3/8 THICK ANGLE WITH 1/2 X 5 1/2 ANCHORS. LOADING: H-20 - OCCASIONAL TRAFFIC MILL FINISH	\$28,277.00
2	316SS FRAME AND COVER 21" X 84" FRAME OPENING BOLTED COVER MADE OUT OF 1/4" THICK DIAMOND PLATES FRAME MADE OUT 3 X 3 X 3/8 THICK ANGLE WITH 1/2 X 5 1/2 ANCHORS. LOADING: H-20 - OCCASIONAL TRAFFIC MILL FINISH	\$18,327.00
ESTIMATED FREIGHT		\$2,500.00

NOTES:

- The above price is firm for 30 days. After such time it may be subject to review. Shipment has been estimated at Approx. 12 - 14 weeks after receipt of purchase order and/or approved submittal drawings.
- Price includes: Freight to the job site off loaded by-others. Submittal drawings and start-up supervision.
- Prices do not include: Sales tax
- Thank you for this opportunity to be of service. If you have any questions regarding this quotation or any other matter, Please do not hesitate to call.



P.O. Box 727 Caruthers, Ca. 93609 • P 559-864-9444 F 559-864-9256
Contractors License Numbers
California Lic: 553794 | DIR 1000003953 | Nevada Lic: 0042398

**SELECTED BASED ON
COMBINED
QUALIFICATION AND
PRICING SCORING
SYSTEM (SEE
ATTACHED SCORE
SHEET)**

REFERENCE
BID DATE

Chandler – 66-Inch Redundant Sewer Project No. 2402.201
Revised 7/22/2025 (Original 1/20/2025)

The following proposal is submitted for your review:

	Description	Size	Quantity	Unit Price	Total
1	Mobilization		1-EA	200,000.00	200,000.00
2	Furnish & Install Bare Casing	86"x1.00"	639-LF	4,245.00	2,712,555.00
Soil Condition: Sandy Clay				Total Bid	\$2,912,555.00

Special Provisions:

Price based on casing quote dated 7/18/25, 90% drawings, no Buy America requirements, PJP weld on casing joints, no conflicts with existing utilities, backfill of existing utilities will not impact tunneling & a scheduled start date of 1/5/2025. Price includes steel rack for Hobas pipe, brick and mortar bulkhead, low density backfill grout and pushing Contractor furnished Hobas pipe. Jacking pit will need high arch doubler spreaders and a backwall capable of 800 tons with no movement.

**PRICE INCREASE DUE TO STEEL
PRICES. SEE NEXT TWO SHEETS FOR
OLD QUOTE COMPARISON TO
CURRENT**

Jacking Pit:	40'	L	16'	W	24"	Below Bottom of Casing
Receiving Pit:	24'	L	14'	W	grade"	Below Bottom of Casing

Stand-by Crew Rate: \$1,497.00 **per hour, for delays beyond control of Pacific Boring (i.e. pit not ready, line and grade staking not available, obstructions, rock, non-borable ground)**

CONTRACTOR TO FURNISH THE FOLLOWING:

1. This quotation will be incorporated into the subcontract between the parties. Pacific Boring will not sign a contract that doesn't have this quote attached and referenced as included.
2. Bore and Receiving pits shall be to the minimum clear inside dimensions as listed above and will require a vertical clearance from pit floor to walers or cross members of 10-foot minimum & up to 14-foot. Contractor shall consult with Pacific Boring personnel on details of pit design to ensure compatibility of pit and proposed tunneling equipment. In addition to Pacific Borings requirements all pits must comply with OSHA requirements, including landings, handrails, toe boards, etc.
3. Dewatering, as directed by Pacific Boring.
4. Furnish access to Job-site, clear overhead access (no power lines over pits), and traffic control.
5. Provide all required surveying including but not limited to; line and grade staking, surface settlement points and monitoring, survey control, etc.
6. Notify Underground Service Alert 48 hours before commencing bore. Locate all substructures for size, line, and depth prior to starting tunneling operations and specifically hold Pacific Boring, Inc. harmless from all damages of any kind caused by failure to properly locate substructures.
7. Permits, including Cal/Osha classification for all bores 30" and larger to be provided prior to scheduling.
8. Remove spoils from job-site. Pacific Boring to bring to street level.
9. Any required vent pipe, cathodic protection and testing such as, welds, concrete, etc.
10. Furnish carrier pipe or RCP unloaded at job-site with all necessary joint materials, as required.
11. Test and fill carrier pipe with water prior to and during sand or grout backfill of casing.
12. If bond required, Add 1 1/2 %.
13. Pothole existing utilities and confirm elevations are not in conflict with bore.
14. Construction water, concrete washout and sanitary facilities.
15. Pacific Boring shall indemnify and defend the contractor for claims of negligence which arise solely from work performed under the contract to the extent that Pacific Boring is found liable for the claim.
16. *DOES NOT INCLUDE RAILROAD INSURANCE, IF APPLICABLE. Pacific Boring as named insured on Contractor's Railroad protective liability insurance policy.*
17. Pacific Boring will pour a concrete thrust block in jacking. General Contractor is responsible for removal of thrust block.
18. In the event that this agreement is placed with an attorney to enforce its provisions, the prevailing party shall be entitled to reasonable attorney's fees and costs.
19. Price based on one move in, additional move ins to be billed at mobilization unit prices provided.
20. Upon execution of contract Pacific Boring will procure materials for your project. If the project is delayed or canceled after scheduling Contractor will be responsible for all specialty material costs and become owner of said materials.
21. Contractor shall provide at least four weeks advance notice of scheduling requirements.
22. If retention is withheld it shall be limited to 5% and shall be paid 60 days after completion of Pacific Borings work.
23. Quote Good for 30-days

Submitted By:

Ted Miller

Estimator

Reference #

250120sc.r2



The Right Pipe The Right Services Right On Time

Home Office: 2331 Hampton Ave. St. Louis, MO 63139
Plant: 11 Skyview Dr. Litchfield, IL 62056
Telecom & Power Yard: 501 Gimblin St. Louis, MO 63147
 1-800-325-2653 314-383-5300 Fax: 314-383-2322

Quote To
 Pacific Boring
 Steven Gallyer / Ted Miller

Quote Number
 200992

Project Description
 Chandler AZ - 66 in Redundant Sewer

Date
 07/18/2025

Terms
 1/2% 10 DAYS, NET 30

FOB
 Chandler AZ

Sales Consultant
 Marshall Blume

Estimated Ship Date (ARO)
 12-15wk MINIMUM
 Lead times subject to change
 until order placed

We are pleased to offer the following material for your consideration:

Pcs	Total Feet	Diameter (inches)	Wall Thickness (inches)	Description/Specifications	Length	Price/Ft	Ext. Price
34	680	86 OD X	1.00	A252-2; ROLLED & WELDED; BARE; BEVELED BY SQUARE; 20' UL'S;	20 Ft	\$ 898.00	\$ 610,640.00
				1-2 MID WELDS; MELT/MANUFACTURE USA; PP TEST REPORTS;			
				All costing, lead time and availability must be verified prior to us accepting an			
				order.			
				Manufacturing begins when we start receiving plate.			
				We will need every bit of 12-15 weeks to make this order.			

**UPDATED QUOTE
WITH INCREASE**

Subtotal \$ 610,640.00
 Estimated Freight \$
 Total \$ 610,640.00

It is the customer's responsibility to inform Pittsburgh Pipe at the time of quotation what, if any, Material specs are required. Customer agrees the material specs listed above meet customer's job specifications.

TERMS & CONDITIONS

Quotation subject to raw material price and availability at time of order

Charges may apply for restocking, cancellation or storage for orders more than 90 days past original ship date.
 Special orders are non-cancellable and non-returnable.
 All dimensions are nominal, unless otherwise indicated.
 All items are offered subject to prior sale, unless otherwise indicated.
 Freight is an estimate and is subject to actual cost at time of shipment.

If you have any questions, or if I may be of further assistance, please do not hesitate to call me

Marshall Blume

Marshall Blume
 Sales Consultant
 314-659-1919
 mblume@pittsburghpipe.com

Steven Gallyer / Ted Miller
 Pacific Boring

*** Large Diameter Pipe * Bore Casing * Rolled and Welded Pipe * Pipe Piling * Polyethylene Duct**
VISIT OUR WEBSITE AT www.pittsburghpipe.com



The Right Pipe The Right Services Right On Time

*Large Diameter Pipe *Bore Casing *Rolled and Welded Pipe *Fiber Optic Casing *Polyethylene Duct

Date: 01/20/2025
Quote: 00200992 PAC936
To: Pacific Boring
Attn: Ted Miller
From: Kevin Davidson
Reference: Chandler AZ - 66 in Redundant Sewer

We are pleased to offer the following material for your consideration:

**680 F 88 OD; 1.00 WALL; A252-2; ROLLED & WELDED; BARE;
BEVELED BY SQUARE; 20' UL'S; 1-2 MID WELDS;
MELT/MANUFACTURE USA; PP TEST REPORTS;
*L*34@20.00**

Delivered Pricing

Pre-tax delivered total: \$550,800.00

at \$810.0000 per F

**OLD QUOTE UTILIZED IN
ORIGINAL PACIFIC BORING
BID**

FOB: Chandler AZ
Terms: 1/2% 10 DAYS, NET 30
Estimated Ship Date: 8-10 weeks ARO

If you have any questions, or if I may be of further assistance, please do not hesitate to call me.
Charges may apply for restocking, cancellation or storage for orders more than 90 days past original ship date.
Special orders are non-cancellable and non-returnable.
All dimensions are nominal, unless otherwise indicated.
All items are offered subject to prior sale, unless otherwise indicated.

VISIT OUR WEBSITE AT www.pittsburghpipe.com

Kevin Davidson



Phone: (602) 997-6164 Fax: (602) 997-4811
Website: www.sscunderground.com E-mail: Arvid@sscunderground.com
Contractor's License No. AZ 085634 / CA 1006387 / NM 382037

PROPOSAL

January 16, 2025

Specialized Services Company (SSC), 2001 W. North Lane, Phoenix, Arizona, 85021, proposes to furnish and provide all labor, material, and equipment necessary to complete the following work:

Job Title/Location: Chandler Redundant Sewer
Scope of Work: See Attachment "A"

Specialized Services Company agrees to complete the work acting as a sub-contractor. The General Contractor is responsible for all taxes related to the gross revenue sale and not the responsibility of Specialized Services Company.

Any alteration or deviation from the specifications contained in this proposal involving extra cost for material, labor, or other expenses resulting from the alteration or deviation from the specification will only be executed upon written orders for same and will require extra charges over the sum quoted in this proposal. All agreements will be made in writing.

Respectfully submitted,

A handwritten signature in black ink that reads 'Arvid Veidmark, III'.

Arvid Veidmark, III
President

Attachment "A"

This Attachment A is specifically incorporated into and made a part of the Proposal between Specialized Services Company and the General Contractor.

This Scope of Work includes:

- A. SSC will setup in a bore pit dug and shored by B&F, 40' long with 16' wide with arches to provide a minimum 110" vertical clearance for jacking equipment. The front box needs to be a 10' x 28' and the back box 10' x 12'. SSC will utilize specialized equipment to excavate the soils in front of the leading edge of the 90" OD x 1" wall steel casing, 6-12" at a time and jack the casing into the space. The soil is excavated with a precision process to ensure the bottom third of the casing is riding on the soil to control line and grade with the top 2/3rd is over-excavated to allow for line and grade adjustments as needed. This process is repeated until the casing is parked at 650'. The 88" ID steel casing is welded with a full circumference weld at each joint as the casing is advanced into the earth. After the casing is parked, the gap between the casing and earth is filled with a neat cement or 6-sack sand slurry to provide a full contact grout. Two key items that we have considered in our plan are the removal of the existing abandoned RCP storm drain and sewer line from inside the tunnel as we advance north of the 202. B&F and SSC will work through the plan to plug these two lines from inside each line to ensure the section to be removed during the tunneling process are secure and empty and SSC will strategically remove the lines while advancing the steel casing. The 66" flush bell HOBAS will be installed on a railing system to ensure line and grade meet the specification requirements and the annular space will be filled with pea gravel after the 66" has been accepted for line and grade. The ends of the casing will be sealed with a brick-and-mortar bulkhead.
- B. Mobilize jack and bore operation to the job site.

NOTE: PRICING ON MATERIAL GOOD FOR 28 DAYS ONLY, AFTER 28 DAYS PRICE CAN INCREASE BASED ON MARKET CONDITIONS!

Reminder: Budget \$6,800.00 dollars into the job for vacuum utility potholing, the last page list SSC vacuum services.

Schedule of Values

<u>Inclusions</u>	<u>Price</u>
A. Horizontal jack and boring machine, labor, welder, excavator, brick and mortar, steel casing, railing, pea gravel, grout for OD of casing.	A. \$2,780.00 p/f x 650' = \$1,807,000.00 for L&E \$1,125.00 p/f x 650' = \$731,250.00 for 90" x 1" wall steel casing.
B. Mobilization	B. \$25,500.00 LS

Change Orders:

The parties agree to adjust the scope of work at any time. Such adjustments may include, but not be limited to, extending, deleting, or adding bore paths or footage, and decreasing work schedules by up to five (5) percent without renegotiating the labor rates quoted in this proposal. The General Contractor agrees to compensate SSC for adjustments requiring increased labor hours on the same terms and conditions set forth in the proposal. The General Contractor and SSC will mutually determine how many hours of additional labor are required for such adjustments.

The parties agree to adjust material quantities at any time. Such adjustments may include, but not be limited to decreasing quantities by up to ten (10) percent without renegotiating the material charges quoted in this proposal. The General Contractor agrees to compensate SSC for adjustments requiring increasing material quantities on the same terms and conditions set for the in the proposal. The General Contractor and SSC will mutually determine quantity increases required for such adjustments.

Initial: _____ Date: _____

Assumptions:

- ~~A. For any bore pit that SSC is proposing to excavate, this is based conventional shore boxes in soils that stand while the pit is being excavated to depth before shoring is set. If the pits are adjacent to an existing trench line or in other soil conditions that will not stand, an alternate method of shoring such as slide rail will need to be evaluated and pricing negotiated, General Contractor may need to take over the digging & shoring.~~
- B. SSC crews will work 5 days per week, Monday – Friday, 8 hours on site. Price includes no weekend, night or double shift work.
- C. This project is being bid as a Prime Contracting project per Arizona Transaction Privilege Tax law and does not have sales tax included in its material price. An AZ 5005 form is required at contract execution, if one cannot be provided, sales tax on materials will be added to SSC's invoices to General Contractor.
- D. SSC is not responsible for any damage to the carrier line in any form or fashion, the General Contractor has the option to provide an operator to jack the carrier line through the steel casing if there is a concern over damage to the carrier line.
- E. The General Contractor agrees to provide 24-hour security personnel to safeguard SSC's property against vandalism, theft, and unauthorized use if any of these issues occurs.
- F. If unknown utility or structure is encountered and/or if existing AZ-Builts are not true, and these conditions result in the delay of the project or stand-by time, the General Contractor and/or owner will agree to absorb SSC's additional costs.
- G. If rock or cobbles are encountered which become too dense for the bore machine to bore through, or if the soil conditions result in a delay of the estimated job completion, the General Contractor agrees to compensate Specialized Services Company for additional labor and materials according to the same terms and conditions set forth in this proposal. The General Contractor and Specialized Service Company will mutually determine in writing additional labor hours, materials, and other resources required for such adjustments.
- H. SSC is not responsible for removing or replacing any asphalt or concrete damaged during mobilization, construction or demobilization.
- I. The pressure testing of the carrier line is the sole responsibility of the General Contractor and owner. Specialized Services Company is not responsible for any failure of the carrier line regardless of the nature of the failure.
- J. The parties agree that Specialized Services Company will not be held responsible or liable for any damage to existing utilities, structures, roadways, or the like caused by cave-ins or flowing material which was not chemically stabilized prior to the bore under any circumstances.
- K. The General Contractor agrees to compensate SSC for any costs incurred in mobilizing or for materials ordered if the project is delayed by unforeseen circumstances, including but not limited to, flowing, running caving material.
- L. The General Contractor agrees to accept the footages listed by the SSC job foreman on site when bores are completed. The General Contractor's representative may verify and sign-off on footages as bores are completed if on site at the time work is completed.
- ~~M. Specialized Services Company will not provide any certified welders or certified welding for the joints of the steel casing.~~
- N. Specialized Services Company will not provide coatings or linings of the steel casings.
- O. The storm drain and sewer line that are in the bore path are sealed off and ready for SSC to safely tunnel through/remove by the time they are encountered in the bore path.

Initial: _____ Date: _____

Exclusions:

Specialized Services Company is not responsible for the following:

- A. Traffic control, dead-man tie-offs, fall protection equipment, temporary fencing and barricades
- ~~B. Any costs or requirements related to work around or under railroad tracks including Railpros, flaggers or observers~~
- C. Excavating the bore pit (40' long x 16' wide) and the reception pit as needed by B&F
- D. Backfilling of bore and reception pits, all spoils removal, and restoration of landscaping
- E. Surveying for proper elevation of casing or carrier line
- F. Cost for re-staking hubs regardless of the amount needed to complete the bore & carrier line install
- G. De-watering of bore and reception pits and bore line
- H. Supply, assembling, and any special equipment to set carrier pipe and install
- I. Any cathodic protection, coatings, or linings of the steel casing and poly wrapping of carrier line
- J. Removal of any asphalt concrete or metal
- K. Boring through any rock, cobbles or unstable material
- L. Replacement of asphalt with hot patch or any concrete replacement such as sidewalk or driveways
- M. Removal and replacement of any permanent fencing
- N. Supply, setup, tear down, and removal of any temporary fencing of entire job site, pits & equipment
- O. Permits, bonds and sales taxes (SSC Bond rate 1.5%)
- P. Geo-technical reports for SSC and grout company (if necessary)
- Q. Chemical grout to stabilize the soil conditions for the project (if necessary)
- ~~R. Grout to fill any annular space inside the casing~~
- S. Construction water and/or fire hydrant hook-ups and any meters
- T. Security personnel to protect the job site
- U. Inspection or testing of any carrier lines, any equipment needed to set the carrier into the bore pit
- V. Pipe fitters, welders, welding machine, linings, wrappings and any items required for the assembly of the carrier line or lines.
- W. Coating and lining of the casing
- X. SWPPP (Storm Water Pollution Prevention Plan)
- Y. Camera and water test of any gravity carrierline
- Z. Potholing any utilities crossed by the proposed bore. (SSC can provide this service, see rates on last page)

Initial: _____ Date: _____

Attachment "B"

This Attachment B is specifically incorporated into and made a part of the Service Agreement between Specialized Services Company and the Client.

Scope of Work:

- A. SSC will provide an air vacuum truck and crew to vacuum excavate and expose utilities or provide vacuum excavation as directed by the Client. SSC will temporarily backfill all holes temporarily with native material. SSC will patch all holes in asphalt or concrete with temporary cold patch.
- B. SSC will provide a hydro-excavation truck and 2 man crew to vacuum excavate as directed. Holes will be left open/responsibility of client to backfill and/or cover.

Schedule of Values:

A. Air Excavation (Vacmaster System 4000)

\$250.00 per hour portal to portal, 4 hr. min.

B. Hydro Excavation (Truvac HXX)

\$285.00 per hour Port to Port (SSC Yard to Jobsite to SSC Yard, includes dump time), 6 hr. min.

Dump Fees – Cost +15% (If onsite dump is not available)
Water Fee - \$50/load (If not available onsite)

- If client request that SSC hauls spoils offsite a removal fee of \$150.00 will be charged per load.
- SSC will supply the Client with a hand-written Utility Report for each utility located, included in hourly rate.
- All holes in asphalt or concrete will be patched with temporary cold patch and backfilled with native soil unless noted otherwise, backfilling is temporary.
- If requested, SSC can core the holes to be potholed and reinstate the cores. Coring will be billed at \$350.00 per pothole.
- If requested, Slurry can be ordered and will be billed at \$200.00 per pothole with a \$1000.00 per day minimum.
- Any utility we pothole where we encounter slurry backfill or concrete encasement will require 2 holes, one on either side of the encasement to confirm the depth on each side.
- The Client or Owner is responsible for all compaction testing on holes.
- Excludes traffic control, temporary fencing, shoring, permits and barricades.
- Set Control for potholes and provide sealed pothole report is \$225.00 per hole, 10 hole minimum.

Note: Billed quantities will be based on actual hours recorded by crews and signed off on by client.

Initial: _____ Date: _____

PROPOSAL



CS CONSTRUCTION, INC.

22023 NORTH 20TH AVENUE

PHOENIX, ARIZONA 85027

PHONE: (623) 889-7650 | FAX: (623) 889-7651

AZ ROC NO. 071662 | A- GENERAL ENGINEERING

PROJECT: LOOP 101 7 202 FREEWAY 66" REDUNDANT SEWER

TO: B&F CONTRACTING, INC.
ATTN: TRAVIS MONK
BID DATE: 7/18/2025

PROJECT NO. WW2402.401
LOCATION: CHANDLER, AZ
ADDENDUMS:

ITEM	DESCRIPTION	QUANTITY	UNIT	UNIT PRICE	AMOUNT
1	MOB/DEMOB	1.00	LS	54,010.00	54,010.00
2	FURNISH AND INSTALL JUNCTION STRUCTURE B	1.00	LS	913,120.00	913,120.00
GRAND TOTAL					\$967,130.00

NOTES:

As per 90% plans and specs, we have prepared a proposal on the above referenced project. We propose to furnish all labor, equipment, supervision and materials to complete the following items for the indicated price.

This proposal includes the following:

1. Requirements per Special Instructions To Bidders
2. Prior to starting the 96 Working Days, C S Construction will need a minimum of 3 months to start the submittal process and purchase long lead items
3. Duration includes 15 Total Working Days of Cure Time. 5 Working Days for the Walls, 5 Working Days for the Grout Fill, and 5 Working Days for the Deck.
4. Time required for Water-Tightness Testing to be added to C S Construction Working Days
5. C S Construction will not be Liable for Delays Caused by Material Provided by Others
6. 1 Mobilization

This proposal excludes the following:

1. Bond, Permits, Sales Tax or Fees
2. Engineering, Testing (including Water-Tightness Testing), and Surveying
3. Excavation
4. Backfill
5. Furnish/Install Bedding Material (if required)
6. All Shoring/Soil Nails for Excavation (if required)
7. Shoring, removal or replacement of existing walls or structures
8. Pipe installation, excavation, encasement, protection, bracing, support, relocation, grouting or cutting
9. Utility adjustments, protect-in-place, and relocations
10. Traffic Control/Dust Control/SWPPP
11. Water Diversion/Water Management/Dewatering
12. Paint, Stain, or Integral Color for Concrete
13. Sealants, caulking, coatings, waterproofing (other than curing compound)
14. Continuous Access to work areas on both sides of structures for equipment/crane/concrete trucks
15. GC to provide secure yard for Materials and Equipment
16. All scope of work not associated with bid items listed/includes

Progress billings shall be submitted by the 30th of the month with payment due by the 15th of the month following. Retention

shall be due to CSCI within 10 days of completion of our work. Sales tax will be invoiced separately unless a signed and completed Arizona Department of Revenue Prime Contractor's Certificate with a valid Transaction Privilege License # is received prior to starting work on this project. If the owner holds retention from the General Contractor, then retention may be held from CS Construction in an equal percentage. Retention must be released 30 days after work has been completed by CS Construction, Inc.

This proposal is valid for a period of thirty (30) days from this date.

Thank you for the opportunity to bid this work. We hope that we may be of service.

Sincerely,
C S CONSTRUCTION, INC.

Stephen Rogers

Stephen Rogers
Estimator

 Digitally signed by Stephen Rogers
Date: 2025.07.18 12:42:51 -07'00'



**CIP STRUCTURE B
QUOTE (NOT SELECTED)**

Bid

Attn: Travis Monk
Date: 07/18/25

Bid# 25-058 R-1
CA. LIC # 621586
AZ. LIC # ROC 253213
DIR # 1000007798
DBE # 39828
PHONE(951)360-7531
FAX (951)360-7561

Job: Redundant Sewer Project, City of Chandler, AZ

www.rduganconst.com

Item #	Description	Quan.	Units	Working Days	Price	Amount
	Junction Structure B per Detail on Sheet S-2.0, S-2.1, S-3.0 & S-3.1 *CONCRETE & REBAR ONLY* *CAST-IN-PLACE HARDWARE SET BY R. DUGAN* *BOLTED HARDWARE SET BY OTHERS*					
	Box 33.00' x 29.11' x 24.46'	1.00	Ea			
	Grout - 900.77 sqft *INSIDE GROUT CHANNEL BY R. DUGAN ONLY*	72.00	CY			
		1.00	LS	47	\$ 1,336,898.00	\$ 1,336,898.00
Total						\$ 1,336,898.00

Excludes:

Excavation, Backfill, Compaction, Demo, Removals, Survey, Testing, Permits, Rip Rap, Traffic Control, Erosion Control
Dewatering, Stencils, Placards, Waterproofing, Bonds, Fossil Filters, Stamps and Fencing.

Notes:

01. Must have 12' access road to all structures.
02. Colored concrete not included unless otherwise noted.
03. Concrete washout pits supplied by others.
04. This proposal is good for 30 days, if awarded after the 30 days price may need to be adjusted.
05. Please add 2.5% if bond is required. To be paid in full. No retention held.
06. Bid is good for standard daytime work hours and does not include night-time or weekend work.
07. Concrete is compliant with AB 219.
08. Cast-in-Place hardware set by R. Dugan.
09. Bolted hardware set by others.
10. Inside grout channel by R. Dugan only.

Contract:

This proposal is good for 30 days. if awarded after 30 days prices may need to be adjusted. Should this proposal be accepted, and if our company must pursue the collection of monies due for work performed, we shall be entitled to recover our attorneys fees and court costs.

Proposal is made from Emailed Request, based on **Prevailing** rates, and standard insurance (GL) of \$ 5,000,000.

If this proposal results in a contract, please give us as much time (1 Week) as possible so we can schedule our crews to accommodate your needs.

Please accept this proposal for the work described above. If this is acceptable, please sign and fax back along with the legal information, your job number and / or a Purchase Order Number.

R. Dugan Construction, Inc.

Marcos Cordova

(951) 360-7531 x 26

Sign.

Date

B&F Contracting, Inc



1575 W Horizon Ridge Parkway
#530427
Henderson, NV 89012

207 Heritage Court
Sulphur Springs, TX 75482

1708 Turkey Creek Road
Plant City, FL 33566

702-824-9702
www.armorock.com

Quote Date: 7/23/2025
To: All Bidders
Reference: Price Road 66" Rehab (Insert Material)
Chandler, AZ
Maricopa County
Sales Manager: Nicholas Drago

Q U O T A T I O N

Notes:

1. Quote is subject to our standard terms, conditions, and shipping policies.
2. Payment is due at Net 30. Late Payment service charge for over 30 days will be charged at 18% APR
3. Quote is valid for 60 days from issue date to receipt of PO after 60 days quote is subject to change based off market value of materials.
4. Sales tax not included. All applicable taxes are to be paid by purchaser
5. If owner requires items to be grouted in field contractor is responsible for materials and labor costs incurred.
6. All loads will be billed at \$2,000 for each legal load out of Boulder City, NV, \$5,300 for each legal load out of Sulphur Springs, TX, \$6,500 for 208" troughing out of Sulphur Springs, TX, \$5,850 for each oversized load at 108" out of SS, \$22,500 for each oversized load at 208" out of SS. Total of 12 legal loads (BC), 1 legal loads (SS), 4 oversized for troughing loads (SS), 2 oversized loads at 108" (SS), and 11 oversized loads at 208" (SS) needed for job. Customer will only be billed for trucks used. Additional trucks will be billed at market rate.
7. Freight requested under 3 days notice may be subject to increased freight rates
8. Products will be billed at unit pricing per quotation
9. Rush orders will be priced accordingly.
10. Contractor shall be responsible for joint sealing and performance.
11. Miscellaneous items such as grout, epoxy, and hardware are not included with the structures unless a specific line item is shown on this quote. Additional charges will apply for any extra items that are not shown on this quote.
12. Final lead time is determined at date of approved submittal.
13. Project retention is not allowed.
14. Summation of structure price is for convenience of bidding only. Structures will be billed by components per quotation detail.

From: JJohnson



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For: All Bidders

Ref: Price Road 66" Rehab (Insert Material)
Chandler, AZ
Maricopa County

Quote Date: 7/23/2025

Structure	Description	Height	Weight	Price
84" CIP	84"-30" Polymer Manhole	19.55'	26,800	\$54,580
1 PR30X2FN	30 IN X 2 IN FINISH PRO-RING	1 PR30X4G	30 IN X 4 IN GRADE PRO-RING	
1 MHL8430	84 IN ECC POLYMER LID FLAT W/ 30" ACCESS	1 MHS8424	84 IN X 2 FT POLYMER MH SECTION	
1 MHS8460	84 IN X 5 FT POLYMER MH SECTION	1 MHS8472	84 IN X 6 FT POLYMER MH SECTION	
1 MHS8424NB	84 IN X 2 FT POLYMER MH SECTION NO BELL	16 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	
Insert MH 756004800	60" Polymer Rehab	18.68'	6,510	\$13,460
1 MHGR30X5	30 IN X 5 IN POLYMER GRADE RING	1 MHGR30X6	30 IN X 6 IN POLYMER GRADE RING	
1 MHTLC3630	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	1 RMHC6036	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	
1 RMH6036	60 IN X 3 FT POLYMER MH SECTION - REHAB	2 RMH6048	60 IN X 4 FT POLYMER MH SECTION - REHAB	
6 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	1 PGROUT	POLYMER GROUT KIT	
Insert MH 756005162	60" Polymer Rehab	17.54'	6,010	\$12,460
1 MHGR30X3	30 IN X 3 IN POLYMER GRADE RING	1 MHGR30X6	30 IN X 6 IN POLYMER GRADE RING	
1 MHTLC3630	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	1 RMHC6036	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	
1 RMH6024	60 IN X 2 FT POLYMER MH SECTION - REHAB	2 RMH6048	60 IN X 4 FT POLYMER MH SECTION - REHAB	
6 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	1 PGROUT	POLYMER GROUT KIT	
Insert MH 756005216	60" Polymer Rehab	9.66'	3,310	\$6,940
1 MHGR30X5	30 IN X 5 IN POLYMER GRADE RING	1 MHGR30X6	30 IN X 6 IN POLYMER GRADE RING	
1 MHTLC3630	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	1 RMHC6036	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	
1 RMH6024	60 IN X 2 FT POLYMER MH SECTION - REHAB	2 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	
1 PGROUT	POLYMER GROUT KIT			
Insert MH 756009519	60" Polymer Rehab	18.51'	6,410	\$13,260
1 MHGR30X3	30 IN X 3 IN POLYMER GRADE RING	1 MHGR30X6	30 IN X 6 IN POLYMER GRADE RING	
1 MHTLC3630	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	1 RMHC6036	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	
1 RMH6036	60 IN X 3 FT POLYMER MH SECTION - REHAB	2 RMH6048	60 IN X 4 FT POLYMER MH SECTION - REHAB	
6 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	1 PGROUT	POLYMER GROUT KIT	
Insert MH 756020514	60" Polymer Rehab	30.25'	10,470	\$21,560
1 MHGR30X6	30 IN X 6 IN POLYMER GRADE RING	1 MHTLC3630	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	
1 RMHC6036	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	1 RMH6036	60 IN X 3 FT POLYMER MH SECTION - REHAB	
5 RMH6048	60 IN X 4 FT POLYMER MH SECTION - REHAB	12 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	
1 PGROUT	POLYMER GROUT KIT			
Insert MH 756023918	60" Polymer Rehab	23.30'	8,020	\$16,540
1 MHGR30X3	30 IN X 3 IN POLYMER GRADE RING	1 MHGR30X4	30 IN X 4 IN POLYMER GRADE RING	
1 MHTLC3630	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	1 RMHC6036	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	
4 RMH6048	60 IN X 4 FT POLYMER MH SECTION - REHAB	8 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	
1 PGROUT	POLYMER GROUT KIT			



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#530427
Henderson, NV 89012
207 Heritage Court
Sulphur Springs, TX 75482
1708 Turkey Creek Road
Plant City, FL 33566
702-824-9702
www.armorock.com

For: All Bidders

Ref: Price Road 66" Rehab (Insert Material)
Chandler, AZ
Maricopa County

Quote Date: 7/23/2025

Structure	Description	Height	Weight	Price
Insert MH 756023919	60" Polymer Rehab	23.23'	7,970	\$16,440
1 MHGR30X6	30 IN X 6 IN POLYMER GRADE RING	1 MHTLC3630	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	
1 RMHC6036	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	4 RMH6048	60 IN X 4 FT POLYMER MH SECTION - REHAB	
8 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	1 PGROUT	POLYMER GROUT KIT	
Insert MH 756023920	60" Polymer Rehab	33.42'	11,570	\$23,820
1 MHGR30X2	30 IN X 2 IN POLYMER GRADE RING	1 MHGR30X6	30 IN X 6 IN POLYMER GRADE RING	
1 MHTLC3630	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	1 RMHC6036	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	
1 RMH6024	60 IN X 2 FT POLYMER MH SECTION - REHAB	6 RMH6048	60 IN X 4 FT POLYMER MH SECTION - REHAB	
14 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	1 PGROUT	POLYMER GROUT KIT	
MH 01 STA 99+03.81	96"-Reduced-36" Polymer Manhole	23.05'	44,530	\$88,840
1 MHGR30X4	30 IN X 4 IN POLYMER GRADE RING	1 MHGR30X5	30 IN X 5 IN POLYMER GRADE RING	
1 MHC6030	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	1 MHS6072	60 IN X 6 FT POLYMER MH SECTION	
1 MHS6060NB	60 IN X 5 FT POLYMER MH SECTION NO BELL	1 MHTL9660	96 IN ECC POLYMER TRANSITION LID W/ 60" ACCESS	
1 MHB9696.133	96 IN X 8 FT POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	2	S206-74	
3 60" Joint Gasket	60" JOINT GASKET	10 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	
1 96" Joint Gasket	96" JOINT GASKET			
MH 02 STA 701+21.86	96"-Reduced-36" Polymer Manhole	23.14'	44,580	\$88,940
2 MHGR30X5	30 IN X 5 IN POLYMER GRADE RING	1 MHC6030	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	
1 MHS6060	60 IN X 5 FT POLYMER MH SECTION	1 MHS6072NB	60 IN X 6 FT POLYMER MH SECTION NO BELL	
1 MHTLC9660	96 IN CONCENTRIC POLYMER TRANSITION LID W/ 60" ACCESS	1 MHB9696.133	96 IN X 8 FT POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	
2	S206-74	3 60" Joint Gasket	60" JOINT GASKET	
10 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	1 96" Joint Gasket	96" JOINT GASKET	
MH 03 STA 102+33.52	96"-Reduced-36" Polymer Manhole	22.79'	44,380	\$88,540
1 MHGR30X6	30 IN X 6 IN POLYMER GRADE RING	1 MHC6030	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	
1 MHS6072	60 IN X 6 FT POLYMER MH SECTION	1 MHS6060NB	60 IN X 5 FT POLYMER MH SECTION NO BELL	
1 MHTL9660	96 IN ECC POLYMER TRANSITION LID W/ 60" ACCESS	1 MHB9696.133	96 IN X 8 FT POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	
2	S206-74	3 60" Joint Gasket	60" JOINT GASKET	
10 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	1 96" Joint Gasket	96" JOINT GASKET	
MH 04 STA 200+00.00	96"-Reduced-36" Polymer Manhole	22.44'	48,400	\$96,065
1 MHGR30X2	30 IN X 2 IN POLYMER GRADE RING	1 MHC6030	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	
1 MHS6072	60 IN X 6 FT POLYMER MH SECTION	1 MHS6060NB	60 IN X 5 FT POLYMER MH SECTION NO BELL	
1 MHTL9660	96 IN ECC POLYMER TRANSITION LID W/ 60" ACCESS	1 MHB9696.133	96 IN X 8 FT POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	
1	S206-54L BOOT	1	S206-74	
3 60" Joint Gasket	60" JOINT GASKET	10 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	
1 96" Joint Gasket	96" JOINT GASKET			
Spreader Bar		0.00'	0	\$3,680
1 RMH60SPREA DER	60" REHAB SPREADER BAR	1 Spreader Bar Kit-48"-72"	SPREADER BAR & RIGGING	



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For: All Bidders

Ref: Price Road 66" Rehab (Insert Material)
Chandler, AZ
Maricopa County

Quote Date: 7/23/2025

Structure	Description	Height	Weight	Price
Structure C Full Depth+	192" Polymer Manhole	22.62'	271,700	\$561,245
1 MHL19230	192 IN ECC SPLIT POLYMER LID FLAT W/ 30" ACCESS	1 MHS19272	192 IN X 6 FT POLYMER MH SECTION	
1 MHS19296	192 IN X 8 FT POLYMER MH SECTION	1 MHS19296H	192 IN X 8 FT POLYMER MH SECTION W/ CORED HOLE	
3	S206-74	1 MHB19215.133	192 IN X 1 FT 3 IN POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	
72 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	3 Gasket	GASKET FOR 192" MANHOLE JOINTS	
1	LABOR FOR FIELD GROUTING	1 PGROUT	POLYMER GROUT KIT	
Structure D	84" Hatch Polymer Manhole	19.79'	31,140	\$53,160
1	30" CITY OF CHANDLER SEWER A-1700 R&C	1 MHGR30X3	30 IN X 3 IN POLYMER GRADE RING	
1 MHC6030	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	1 MHS6060	60 IN X 5 FT POLYMER MH SECTION	
1 MHS6060NB	60 IN X 5 FT POLYMER MH SECTION NO BELL	1 MHTL8460	84 IN ECC POLYMER TRANSITION LID W/ 60" ACCESS	
1 MHB8473.133	84 IN X 6 FT 1 IN POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	2	S206-54L BOOT	
10 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	3 Gasket	GASKET FOR 192" MANHOLE JOINTS	
2 Gasket	GASKET FOR 48" MANHOLE JOINTS	1 PGROUT	POLYMER GROUT KIT	
Structure E Full Depth +	192" Polymer Manhole	22.21'	271,800	\$552,735
1	30" CITY OF CHANDLER SEWER A-1700 R&C	1 MHL19230	192 IN ECC SPLIT POLYMER LID FLAT W/ 30" ACCESS	
1 MHS19267	192 IN X 5 FT 7 IN POLYMER MH SECTION	1 MHS19296	192 IN X 8 FT POLYMER MH SECTION	
1 MHS19296H	192 IN X 8 FT POLYMER MH SECTION W/ CORED HOLE	3	S206-74	
1 MHB19215.133	192 IN X 1 FT 3 IN POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	72 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	
3 Gasket	GASKET FOR 192" MANHOLE JOINTS	1	LABOR FOR FIELD GROUTING	
1 PGROUT	POLYMER GROUT KIT			
Tee 756004806	48"-30" Polymer Manhole	17.40'	6,050	\$12,752
1 MHGR30X4	30 IN X 4 IN POLYMER GRADE RING	1 MHGR30X5	30 IN X 5 IN POLYMER GRADE RING	
1 MHL4830	48 IN ECC POLYMER LID FLAT W/ 30" ACCESS	1 MHS4848	48 IN X 4 FT POLYMER MH SECTION	
1 MHS4872	48 IN X 6 FT POLYMER MH SECTION	1 MHS4812NB	48 IN X 1 FT POLYMER MH SECTION NO BELL	
3 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL			
Tee 756007414	60"-30" Polymer Manhole w/36" Tee	35.46'	14,040	\$29,120
1 MHGR30X5	30 IN X 5 IN POLYMER GRADE RING	1 MHC6030	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	
4 MHS6072	60 IN X 6 FT POLYMER MH SECTION	1 56003-TL	36 IN TEE ADAPTOR 60 IN	
10 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL			
Tee 756009377	60"-30" Polymer Manhole w/36" Tee	15.70'	5,290	\$11,440
1 MHGR30X4	30 IN X 4 IN POLYMER GRADE RING	1 MHC6030	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	
1 MHS6072	60 IN X 6 FT POLYMER MH SECTION	1 56003-TL	36 IN TEE ADAPTOR 60 IN	
4 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL			
Tee 756009378	48"-30" Polymer Manhole	20.82'	7,750	\$16,325
1 MHGR30X3	30 IN X 3 IN POLYMER GRADE RING	2 MHGR30X6	30 IN X 6 IN POLYMER GRADE RING	
1 MHL4830	48 IN ECC POLYMER LID FLAT W/ 30" ACCESS	1 MHS4812	48 IN X 1 FT POLYMER MH SECTION	
2 MHS4872	48 IN X 6 FT POLYMER MH SECTION	1 MHS4812NB	48 IN X 1 FT POLYMER MH SECTION NO BELL	
4 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL			



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For: All Bidders

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Maricopa County

Quote Date: 7/23/2025

Structure	Description	Height	Weight	Price
Tee 756011281	48"-30" Polymer Manhole	18.06'	6,250	\$13,182
1 MHGR30X3	30 IN X 3 IN POLYMER GRADE RING	1 MHGR30X4	30 IN X 4 IN POLYMER GRADE RING	
1 MHL4830	48 IN ECC POLYMER LID FLAT W/ 30" ACCESS	1 MHS4860	48 IN X 5 FT POLYMER MH SECTION	
1 MHS4872	48 IN X 6 FT POLYMER MH SECTION	1 MHS4812NB	48 IN X 1 FT POLYMER MH SECTION NO BELL	
3 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL			
Tee 756013317	48"-30" Polymer Manhole	19.13'	6,650	\$14,022
1 MHGR30X3	30 IN X 3 IN POLYMER GRADE RING	1 MHGR30X4	30 IN X 4 IN POLYMER GRADE RING	
1 MHL4830	48 IN ECC POLYMER LID FLAT W/ 30" ACCESS	2 MHS4872	48 IN X 6 FT POLYMER MH SECTION	
1 MHS4812NB	48 IN X 1 FT POLYMER MH SECTION NO BELL	3 MASJ	1.25 IN X 14 FT JOINT MASTIC ROLL	
Sub-Total				\$1,809,106
Freight/Delivery				\$314,500
Total Price for Price Road 66" Rehab (Insert Material)				\$2,123,606



1575 W Horizon Ridge Parkway
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Henderson, NV 89012

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APPROVED BY: _____

Structure: 84" CIP
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County

Based on:
Take Off By: 7/23/2025 Joshua Johnson

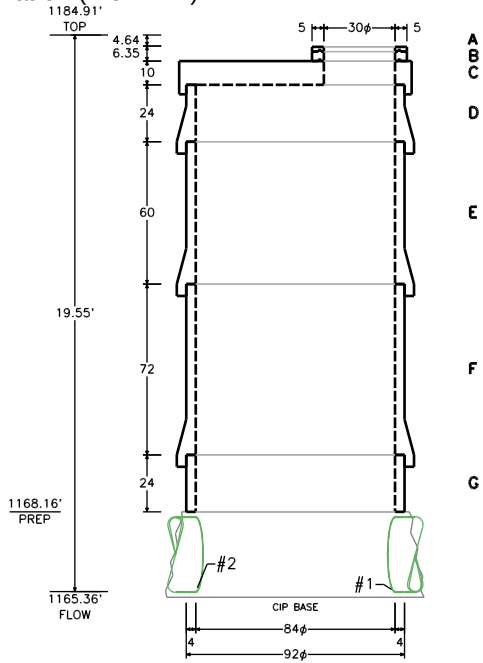
Contractor: All Bidders
Agency:
Engineer:

Design Build Height			Stack Build Height			Ship Date:	
Rim	+	1,184.91'	Ring	+	0.39'	Ring:	30" City of Chandler CAP R&C By Others
Outlet Invert	-	1,165.36'	Adjustment	+	0.53'	Channel:	BASE POURED IN FIELD BY CONTRACTOR
Design Height	=	19.55'	Eccentric Slab	+	0.83'	Reduction:	
Pipe Inner Dia.	-	2.60'	Risers	+	15.00'	Notes:	
Pipe Wall	-	0.03'	Above Base	=	16.75'		
Pour Above	-	0.17'	Contractor Base	+	2.80'		
Above Base	=	16.75'	Design Height	=	19.55'		
Openings							

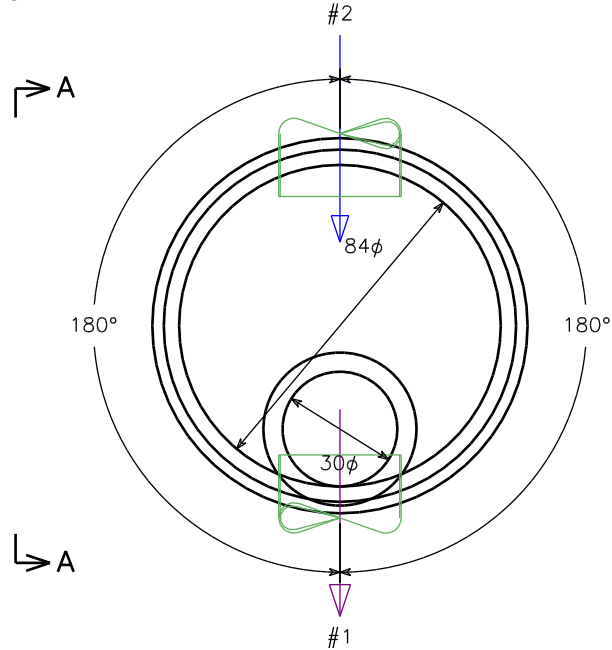
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.36'	30" Ductile Iron	In 84 IN CIP Base	N/A	0°	18"	0"	30 DIP
#2	1165.36'	30" Ductile Iron	In 84 IN CIP Base	N/A	180°	18"	0"	30 DIP

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	16	
A	30 IN X 2 IN FINISH PRO-RING	PR30X2FN	1	
B	30 IN X 4 IN GRADE PRO-RING	PR30X4G	1	
C	84 IN ECC POLYMER LID FLAT W/ 30" ACCESS	MHL8430	1	6,100
D	84 IN X 2 FT POLYMER MH SECTION	MHS8424	1	3,500
E	84 IN X 5 FT POLYMER MH SECTION	MHS8460	1	6,900
F	84 IN X 6 FT POLYMER MH SECTION	MHS8472	1	8,000
G	84 IN X 2 FT POLYMER MH SECTION NO BELL	MHS8424NB	1	2,300
Totals (precast, lbs.)			7	26,800



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APPROVED BY: _____

Structure: Insert MH 756004800

Job Number: AZ2402

Job Name: Price Road 66" Rehab (Insert Material)

Job Location: Chandler, AZ

Maricopa County

Contractor: All Bidders

Agency:

Engineer:

Based on:

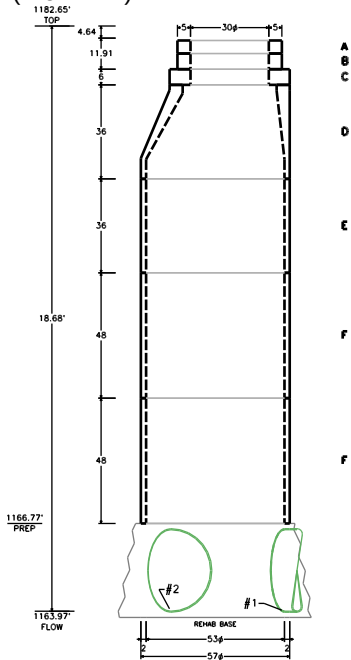
Take Off By: 7/17/2025 PMcRae

Design Build Height	Stack Build Height	Ship Date:
Top of Casting + 1,182.65'	Ring + 0.39'	Ring: 30" City of Chandler CAP R&C By Others
Outlet Invert - 1,163.97'	Adjustment + 0.99'	Channel: BASE POURED IN FIELD BY CONTRACTOR
Design Height = 18.68'	Concentric Slab + 0.50'	Reduction:
Pipe Inner Dia. - 2.60'	Eccentric Cone + 3.00'	Notes:
Pipe Wall - 0.03'	Risers + 11.00'	
Pour Above - 0.17'	Above Base = 15.88'	
Above Base = 15.88'	Contractor Base + 2.80'	
	Design Height = 18.68'	

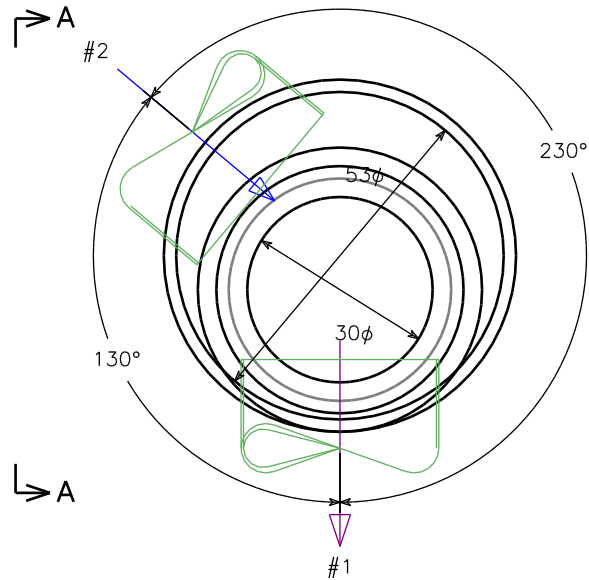
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1163.97'	30" Ductile Iron	In 53 IN Rehab Base	N/A	0°	18"	0"	30 DIP
#2	1163.97'	30" Ductile Iron	In 53 IN Rehab Base	N/A	130°	18"	0"	30 DIP

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	6	
A	30 IN X 5 IN POLYMER GRADE RING	MHGR30X5	1	240
B	30 IN X 6 IN POLYMER GRADE RING	MHGR30X6	1	280
C	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	MHTLC3630	1	490
D	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	RMHC6036	1	1,600
E	60 IN X 3 FT POLYMER MH SECTION - REHAB	RMH6036	1	1,100
F	60 IN X 4 FT POLYMER MH SECTION - REHAB	RMH6048	2	2,800
Totals (precast, lbs.)			7	6,510



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APPROVED BY: _____

Structure: Insert MH 756005162
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County
Contractor: All Bidders
Agency:
Engineer:

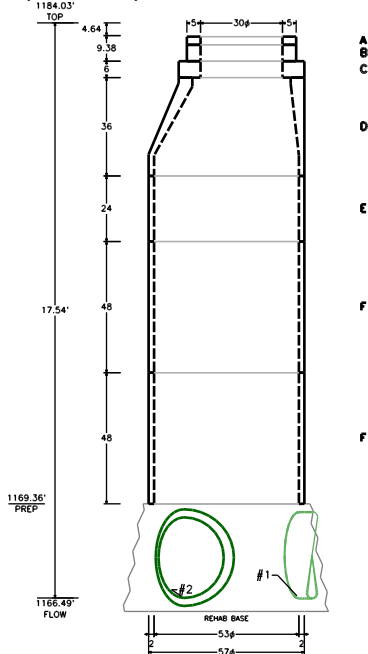
Based on:
Take Off By: 7/17/2025 PMcRae

Design Build Height	Stack Build Height	Ship Date:
Top of Casting + 1,184.03'	Ring + 0.39'	Ring: 30" City of Chandler CAP R&C By Others
Outlet Invert - 1,166.49'	Adjustment + 0.78'	Channel: BASE POURED IN FIELD BY CONTRACTOR
Design Height = 17.54'	Concentric Slab + 0.50'	Reduction:
Pipe Inner Dia. - 2.46'	Eccentric Cone + 3.00'	Notes:
Pipe Wall - 0.24'	Risers + 10.00'	
Pour Above - 0.17'	Above Base = 14.67'	
Above Base = 14.67'	Contractor Base + 2.87'	
	Design Height = 17.54'	

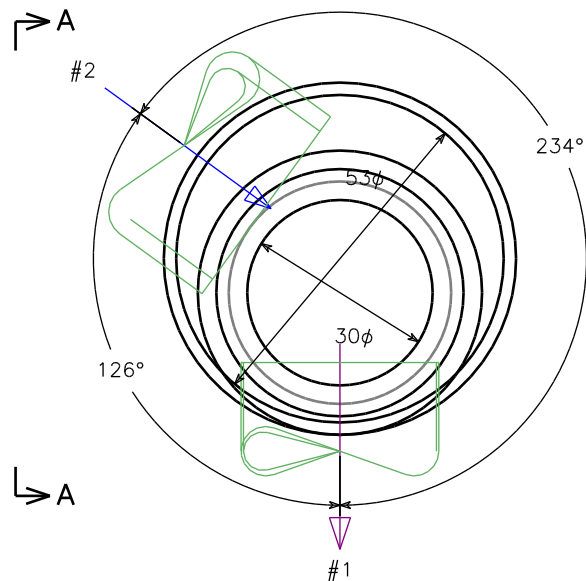
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1166.49'	30" Ductile Iron	In 53 IN Rehab Base	N/A	0°	20.54"	0.85"	30 DIP
#2	1166.49'	30" Vitrified Clay	In 53 IN Rehab Base	N/A	126°	19.69"	-0.31"	30 VCP

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	6	
A	30 IN X 3 IN POLYMER GRADE RING	MHGR30X3	1	140
B	30 IN X 6 IN POLYMER GRADE RING	MHGR30X6	1	280
C	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	MHTLC3630	1	490
D	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	RMHC6036	1	1,600
E	60 IN X 2 FT POLYMER MH SECTION - REHAB	RMH6024	1	700
F	60 IN X 4 FT POLYMER MH SECTION - REHAB	RMH6048	2	2,800
Totals (precast, lbs.)			7	6,010



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APPROVED BY: _____

Structure: Insert MH 756005216
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County
Contractor: All Bidders
Agency:
Engineer:

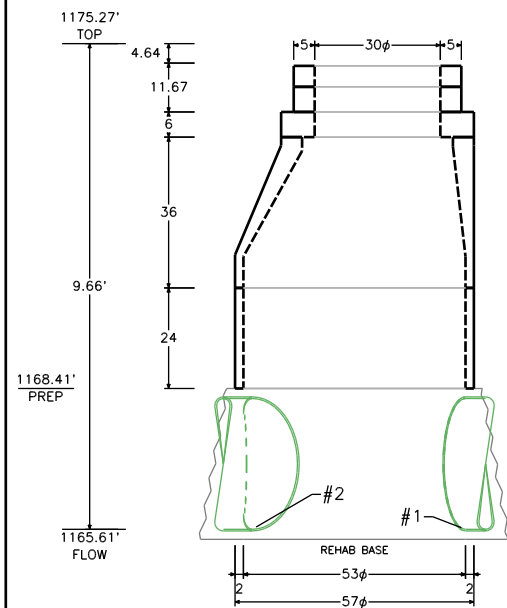
Based on:
Take Off By: 7/17/2025 PMcRae

Design Build Height	Stack Build Height	Ship Date:
Top of Casting + 1,175.27'	Ring + 0.39'	Ring: 30" City of Chandler CAP R&C By Others
Outlet Invert - 1,165.61'	Adjustment + 0.97'	Channel: BASE POURED IN FIELD BY CONTRACTOR
Design Height = 9.66'	Concentric Slab + 0.50'	Reduction:
Pipe Inner Dia. - 2.60'	Eccentric Cone + 3.00'	Notes:
Pipe Wall - 0.03'	Risers + 2.00'	
Pour Above - 0.17'	Above Base = 6.86'	
Above Base = 6.86'	Contractor Base + 2.80'	
	Design Height = 9.66'	

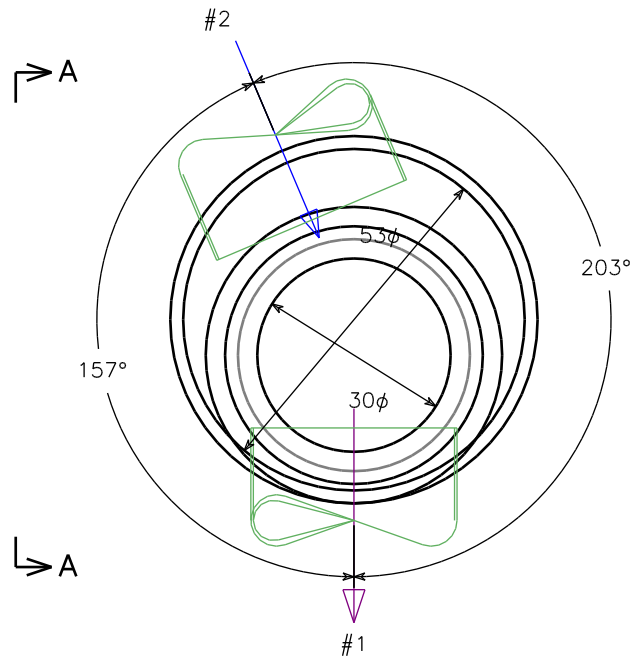
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.61'	30" Ductile Iron	In 53 IN Rehab Base	N/A	0°	18"	0"	30 DIP
#2	1165.61'	30" Ductile Iron	In 53 IN Rehab Base	N/A	157°	18"	0"	30 DIP

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	2	
A	30 IN X 5 IN POLYMER GRADE RING	MHGR30X5	1	240
B	30 IN X 6 IN POLYMER GRADE RING	MHGR30X6	1	280
C	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	MHTLC3630	1	490
D	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	RMHC6036	1	1,600
E	60 IN X 2 FT POLYMER MH SECTION - REHAB	RMH6024	1	700
Totals (precast, lbs.)			5	3,310



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APPROVED BY: _____

Structure: Insert MH 756009519
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County

Based on:
Take Off By: 7/17/2025 PMcRae

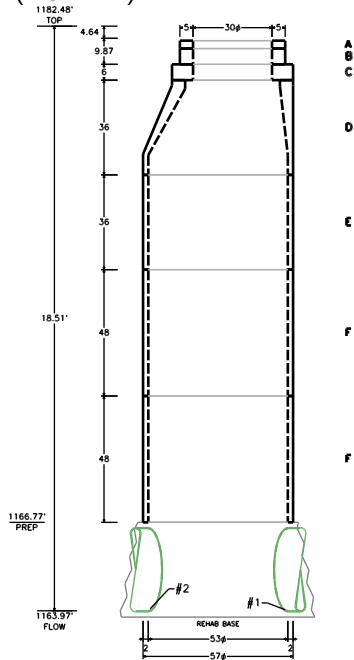
Contractor: All Bidders
Agency:
Engineer:

Design Build Height	Stack Build Height	Ship Date:
Top of Casting + 1,182.48'	Ring + 0.39'	Ring: 30" City of Chandler CAP R&C By Others
Outlet Invert - 1,163.97'	Adjustment + 0.82'	Channel: BASE POURED IN FIELD BY CONTRACTOR
Design Height = 18.51'	Concentric Slab + 0.50'	Reduction:
Pipe Inner Dia. - 2.60'	Eccentric Cone + 3.00'	Notes:
Pipe Wall - 0.03'	Risers + 11.00'	
Pour Above - 0.17'	Above Base = 15.71'	
Above Base = 15.71'	Contractor Base + 2.80'	
	Design Height = 18.51'	

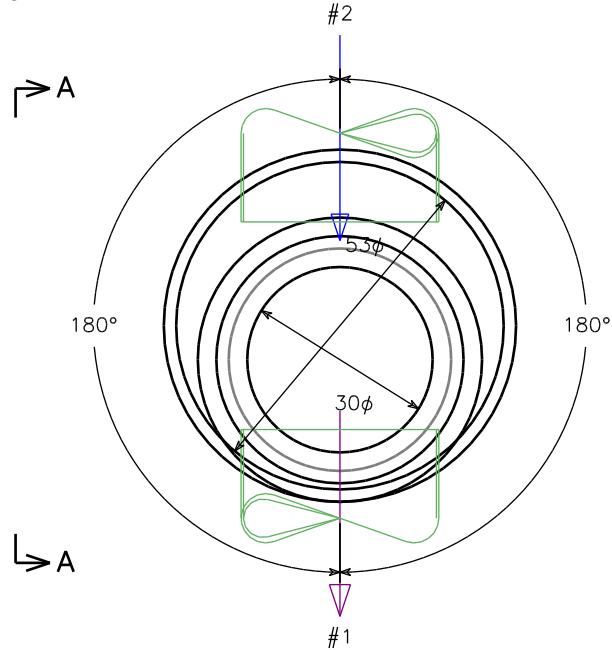
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1163.97'	30" Ductile Iron	In 53 IN Rehab Base	N/A	0°	18"	0"	30 DIP
#2	1163.97'	30" Ductile Iron	In 53 IN Rehab Base	N/A	180°	18"	0"	30 DIP

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	6	
A	30 IN X 3 IN POLYMER GRADE RING	MHGR30X3	1	140
B	30 IN X 6 IN POLYMER GRADE RING	MHGR30X6	1	280
C	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	MHTLC3630	1	490
D	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	RMHC6036	1	1,600
E	60 IN X 3 FT POLYMER MH SECTION - REHAB	RMH6036	1	1,100
F	60 IN X 4 FT POLYMER MH SECTION - REHAB	RMH6048	2	2,800
Totals (precast, lbs.)			7	6,410



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APPROVED BY: _____

Structure: Insert MH 756020514

Job Number: AZ2402

Job Name: Price Road 66" Rehab (Insert Material)

Job Location: Chandler, AZ

Maricopa County

Contractor: All Bidders

Agency:

Engineer:

Based on:

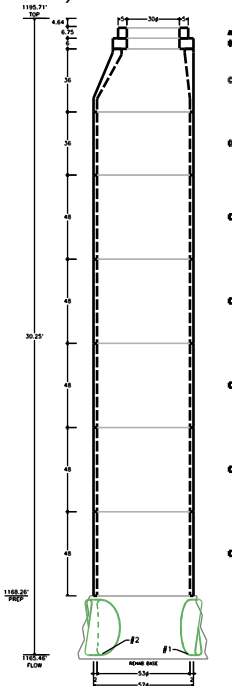
Take Off By: 7/17/2025 PMcRae

Design Build Height	Stack Build Height	Ship Date:
Top of Casting + 1,195.71'	Ring + 0.39'	Ring: 30" City of Chandler CAP R&C By Others
Outlet Invert - 1,165.46'	Adjustment + 0.56'	Channel: BASE POURED IN FIELD BY CONTRACTOR
Design Height = 30.25'	Concentric Slab + 0.50'	Reduction:
Pipe Inner Dia. - 2.60'	Eccentric Cone + 3.00'	Notes:
Pipe Wall - 0.03'	Risers + 23.00'	
Pour Above - 0.17'	Above Base = 27.45'	
Above Base = 27.45'	Contractor Base + 2.80'	
	Design Height = 30.25'	

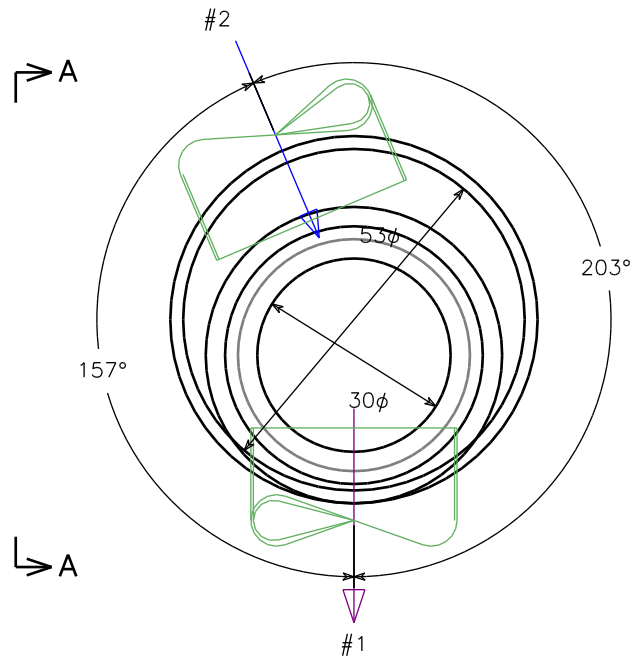
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.46'	30" Ductile Iron	In 53 IN Rehab Base	N/A	0°	18"	0"	30 DIP
#2	1165.46'	30" Ductile Iron	In 53 IN Rehab Base	N/A	157°	18"	0"	30 DIP

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	12	
A	30 IN X 6 IN POLYMER GRADE RING	MHGR30X6	1	280
B	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	MHTLC3630	1	490
C	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	RMHC6036	1	1,600
D	60 IN X 3 FT POLYMER MH SECTION - REHAB	RMH6036	1	1,100
E	60 IN X 4 FT POLYMER MH SECTION - REHAB	RMH6048	5	7,000
Totals (precast, lbs.)			9	10,470



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APPROVED BY: _____

Structure: Insert MH 756023918
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County
Contractor: All Bidders
Agency:
Engineer:

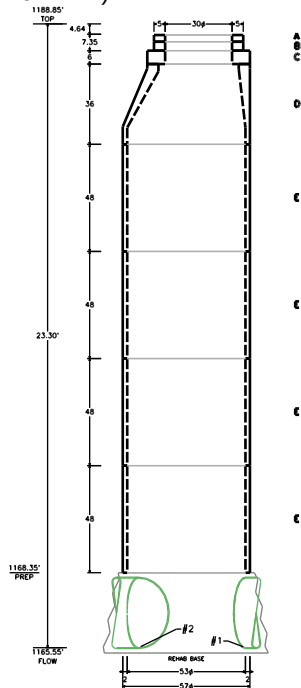
Based on:
Take Off By: 7/17/2025 PMcRae

Design Build Height	Stack Build Height	Ship Date:
Top of Casting + 1,188.85'	Ring + 0.39'	Ring: 30" City of Chandler CAP R&C By Others
Outlet Invert - 1,165.55'	Adjustment + 0.61'	Channel: BASE POURED IN FIELD BY CONTRACTOR
Design Height = 23.30'	Concentric Slab + 0.50'	Reduction:
Pipe Inner Dia. - 2.60'	Eccentric Cone + 3.00'	Notes:
Pipe Wall - 0.03'	Risers + 16.00'	
Pour Above - 0.17'	Above Base = 20.50'	
Above Base = 20.50'	Contractor Base + 2.80'	
	Design Height = 23.30'	

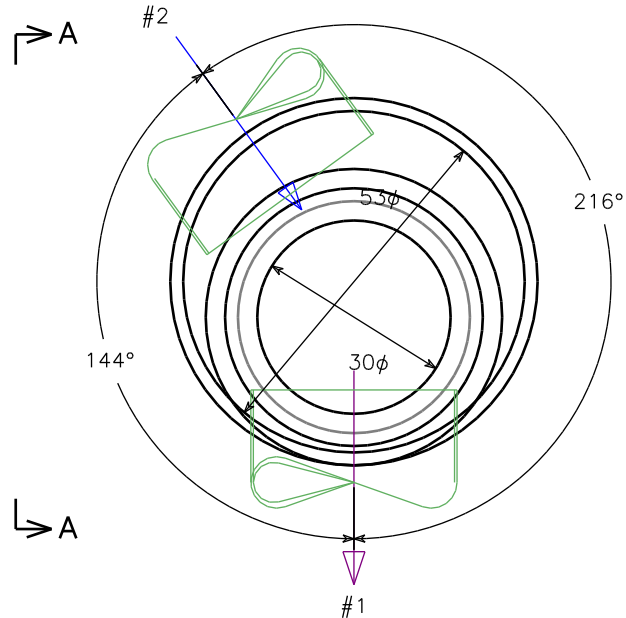
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.55'	30" Ductile Iron	In 53 IN Rehab Base	N/A	0°	18"	0"	30 DIP
#2	1165.55'	30" Ductile Iron	In 53 IN Rehab Base	N/A	144°	18"	0"	30 DIP

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	8	
A	30 IN X 3 IN POLYMER GRADE RING	MHGR30X3	1	140
B	30 IN X 4 IN POLYMER GRADE RING	MHGR30X4	1	190
C	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	MHTLC3630	1	490
D	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	RMHC6036	1	1,600
E	60 IN X 4 FT POLYMER MH SECTION - REHAB	RMH6048	4	5,600
Totals (precast, lbs.)			8	8,020



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APPROVED BY: _____

Structure: Insert MH 756023919

Job Number: AZ2402

Job Name: Price Road 66" Rehab (Insert Material)

Job Location: Chandler, AZ

Maricopa County

Contractor: All Bidders

Agency:

Engineer:

Based on:

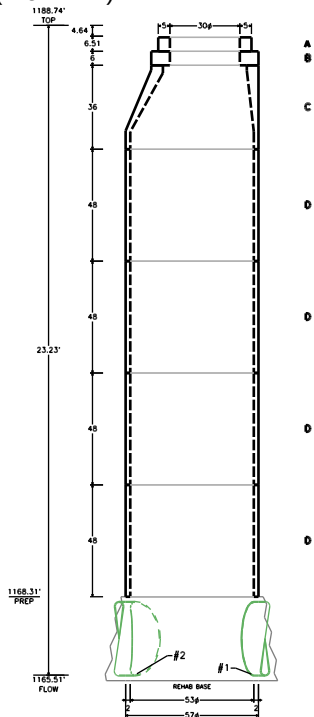
Take Off By: 7/17/2025 PMcRae

Design Build Height	Stack Build Height	Ship Date:
Top of Casting + 1,188.74'	Ring + 0.39'	Ring: 30" City of Chandler CAP R&C By Others
Outlet Invert - 1,165.51'	Adjustment + 0.54'	Channel: BASE POURED IN FIELD BY CONTRACTOR
Design Height = 23.23'	Concentric Slab + 0.50'	Reduction:
Pipe Inner Dia. - 2.60'	Eccentric Cone + 3.00'	Notes:
Pipe Wall - 0.03'	Risers + 16.00'	
Pour Above - 0.17'	Above Base = 20.43'	
Above Base = 20.43'	Contractor Base + 2.80'	
	Design Height = 23.23'	

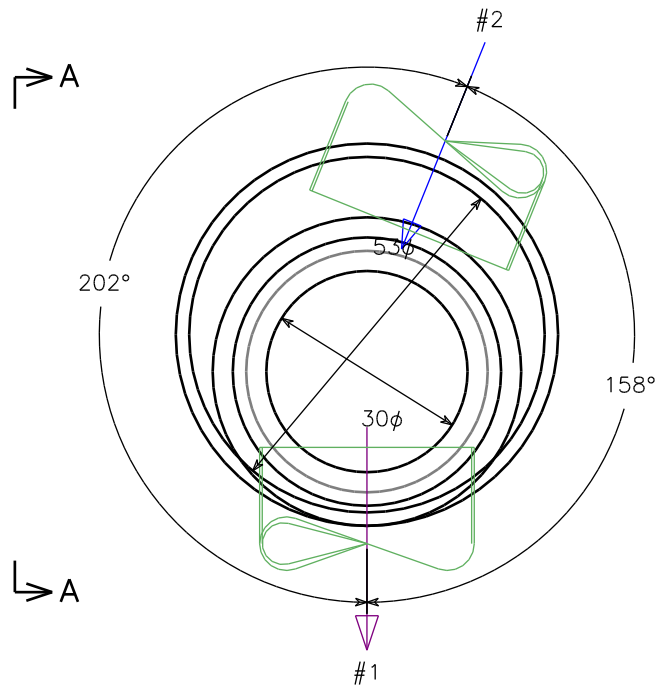
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.51'	30" Ductile Iron	In 53 IN Rehab Base	N/A	0°	18"	0"	30 DIP
#2	1165.51'	30" Ductile Iron	In 53 IN Rehab Base	N/A	202°	18"	0"	30 DIP

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	8	
A	30 IN X 6 IN POLYMER GRADE RING	MHGR30X6	1	280
B	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	MHTLC3630	1	490
C	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	RMHC6036	1	1,600
D	60 IN X 4 FT POLYMER MH SECTION - REHAB	RMH6048	4	5,600
Totals (precast, lbs.)			7	7,970



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APPROVED BY: _____

Structure: Insert MH 756023920
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County
Contractor: All Bidders
Agency:
Engineer:

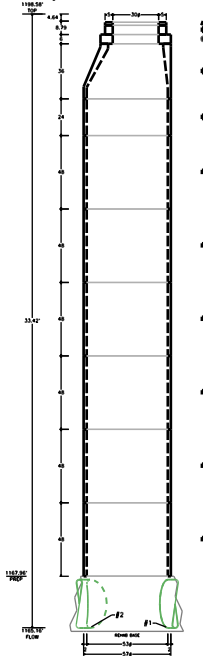
Based on:
Take Off By: 7/17/2025 PMcRae

Design Build Height	Stack Build Height	Ship Date:
Top of Casting + 1,198.58'	Ring + 0.39'	Ring: 30" City of Chandler CAP R&C By Others
Outlet Invert - 1,165.16'	Adjustment + 0.73'	Channel: BASE POURED IN FIELD BY CONTRACTOR
Design Height = 33.42'	Concentric Slab + 0.50'	Reduction:
Pipe Inner Dia. - 2.60'	Eccentric Cone + 3.00'	Notes:
Pipe Wall - 0.03'	Risers + 26.00'	
Pour Above - 0.17'	Above Base = 30.62'	
Above Base = 30.62'	Contractor Base + 2.80'	
	Design Height = 33.42'	

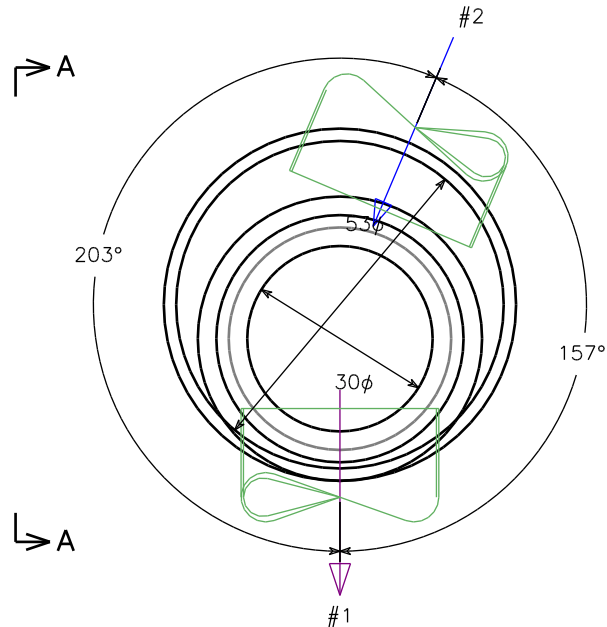
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.16'	30" Ductile Iron	In 53 IN Rehab Base	N/A	0°	18"	0"	30 DIP
#2	1165.16'	30" Ductile Iron	In 53 IN Rehab Base	N/A	203°	18"	0"	30 DIP

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	14	
A	30 IN X 2 IN POLYMER GRADE RING	MHGR30X2	1	100
B	30 IN X 6 IN POLYMER GRADE RING	MHGR30X6	1	280
C	36 IN CONCENTRIC POLYMER TRANSITION LID W/ 30" ACCESS	MHTLC3630	1	490
D	60 IN X 3 FT ECC POLYMER CONE - REHAB W/ 36" ACCESS	RMHC6036	1	1,600
E	60 IN X 2 FT POLYMER MH SECTION - REHAB	RMH6024	1	700
F	60 IN X 4 FT POLYMER MH SECTION - REHAB	RMH6048	6	8,400
Totals (precast, lbs.)			11	11,570



1575 W Horizon Ridge Parkway
#530427
Henderson, NV 89012

207 Heritage Court
Sulphur Springs, TX 75482

1708 Turkey Creek Road
Plant City, FL 33566

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APPROVED BY: _____

Structure: MH 01 STA 99+03.81
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County

Based on:
Take Off By: 7/17/2025 PMcRae

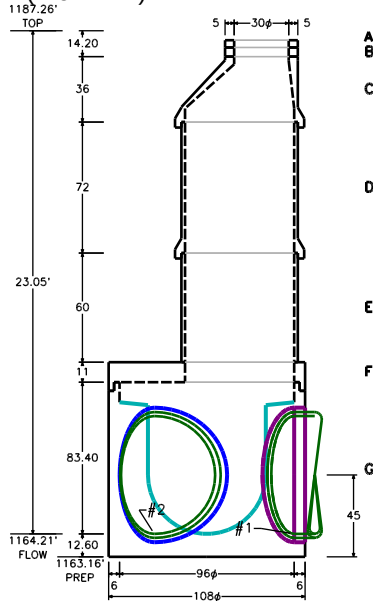
Contractor: All Bidders
Agency:
Engineer:

Design Build Height		Stack Build Height		Ship Date:	
Rim	+ 1,187.26'	Ring	+ 0.39'	Ring:	30" City of Chandler CAP R&C By Others
Outlet Invert	- 1,164.21'	Adjustment	+ 0.80'	Channel:	FULL DEPTH PLUS 6" TROUGHING
Design Height	= 23.05'	Eccentric Cone	+ 3.00'	Reduction:	
		Risers	+ 11.00'	Notes:	
		Eccentric Slab	+ 0.92'		
		Base Gain	+ 6.95'		
		Design Height	= 23.05'		
		Floor Thickness	+ 1.05'		
		Outside Height	= 24.10'		

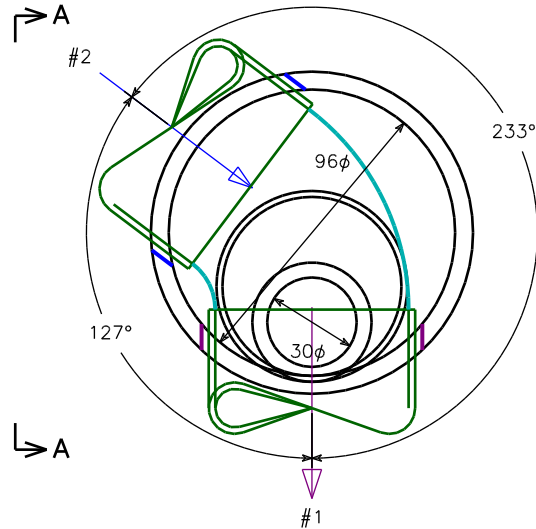
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1164.21'	66 HOBAS	S206-74	74" Ø (84.49" v	0°	45"	14"	
#2	1164.21'	66 HOBAS	S206-74	74" Ø (84.49" v	127°	45"	14"	

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	60" JOINT GASKET	60" Joint Gaske	3	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	10	
	96" JOINT GASKET	96" Joint Gaske	1	
A	30 IN X 4 IN POLYMER GRADE RING	MHGR30X4	1	190
B	30 IN X 5 IN POLYMER GRADE RING	MHGR30X5	1	240
C	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	MHC6030	1	2,200
D	60 IN X 6 FT POLYMER MH SECTION	MHS6072	1	2,900
E	60 IN X 5 FT POLYMER MH SECTION NO BELL	MHS6060NB	1	2,000
F	96 IN ECC POLYMER TRANSITION LID W/ 60" ACCESS	MHTL9660	1	6,300
G	96 IN X 8 FT POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	MHB9696.133	1	30,700
Totals (precast, lbs.)			7	44,530



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APPROVED BY: _____

Structure: MH 02 STA 701+21.86

Job Number: AZ2402

Job Name: Price Road 66" Rehab (Insert Material)

Job Location: Chandler, AZ
Maricopa County

Contractor: All Bidders

Agency:

Engineer:

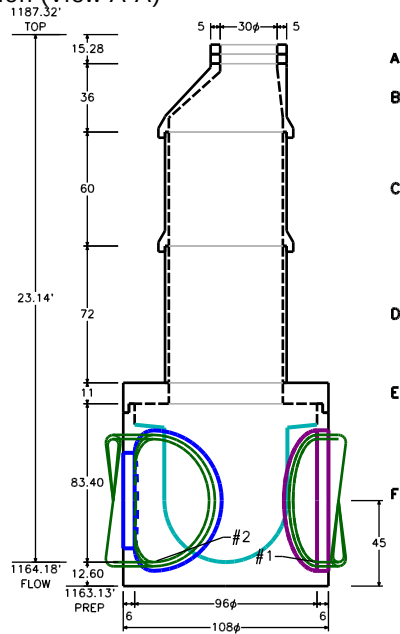
Based on:
Take Off By: 7/17/2025 PMcRae

Design Build Height		Stack Build Height		Ship Date:	
Rim	+ 1,187.32'	Ring	+ 0.39'	Ring:	30" City of Chandler CAP R&C By Others
Outlet Invert	- 1,164.18'	Adjustment	+ 0.89'	Channel:	FULL DEPTH PLUS 6" TROUGHING
Design Height	= 23.14'	Eccentric Cone	+ 3.00'	Reduction:	
		Risers	+ 11.00'	Notes:	
		Concentric Slab	+ 0.92'		
		Base Gain	+ 6.95'		
		Design Height	= 23.14'		
		Floor Thickness	+ 1.05'		
		Outside Height	= 24.19'		

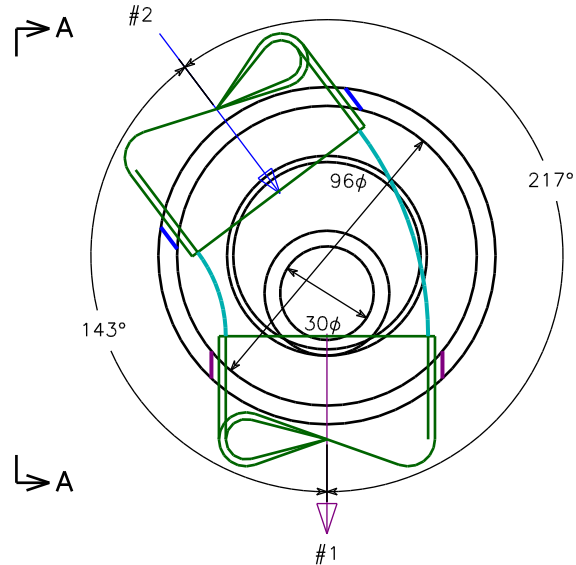
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1164.18'	66 HOBAS	S206-74	74" Ø (84.49" v	0°	45"	14"	
#2	1164.18'	66 HOBAS	S206-74	74" Ø (84.49" v	143°	45"	14"	

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	60" JOINT GASKET	60" Joint Gaske	3	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	10	
	96" JOINT GASKET	96" Joint Gaske	1	
A	30 IN X 5 IN POLYMER GRADE RING	MHGR30X5	2	480
B	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	MHC6030	1	2,200
C	60 IN X 5 FT POLYMER MH SECTION	MHS6060	1	2,500
D	60 IN X 6 FT POLYMER MH SECTION NO BELL	MHS6072NB	1	2,400
E	96 IN CONCENTRIC POLYMER TRANSITION LID W/ 60" ACCESS	MHTLC9660	1	6,300
F	96 IN X 8 FT POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	MHB9696.133	1	30,700
Totals (precast, lbs.)			7	44,580



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APPROVED BY: _____

Structure: MH 03 STA 102+33.52
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County

Based on:
Take Off By: 7/17/2025 PMcRae

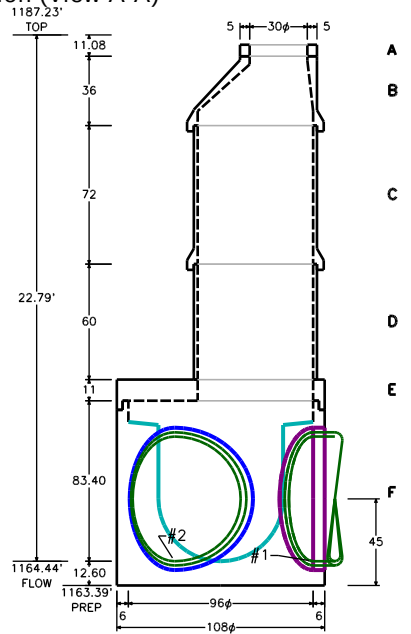
Contractor: All Bidders
Agency:
Engineer:

Design Build Height		Stack Build Height		Ship Date:	
Rim	+ 1,187.23'	Ring	+ 0.39'	Ring:	30" City of Chandler CAP R&C By Others
Outlet Invert	- 1,164.44'	Adjustment	+ 0.54'	Channel:	FULL DEPTH PLUS 6" TROUGHING
Design Height	= 22.79'	Eccentric Cone	+ 3.00'	Reduction:	
		Risers	+ 11.00'	Notes:	
		Eccentric Slab	+ 0.92'		
		Base Gain	+ 6.95'		
		Design Height	= 22.79'		
		Floor Thickness	+ 1.05'		
		Outside Height	= 23.84'		

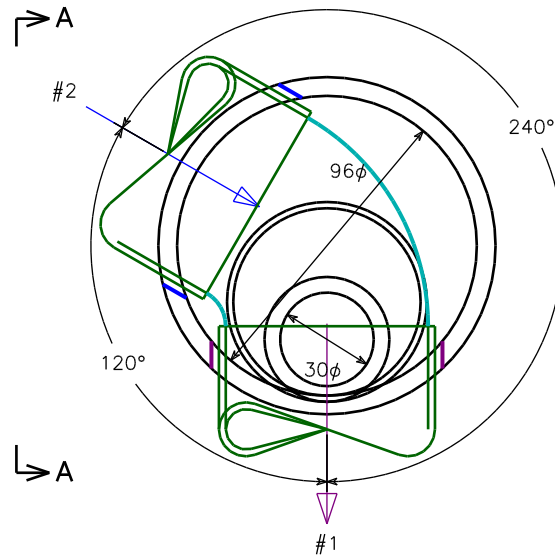
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1164.44'	66 HOBAS	S206-74	74" Ø (84.49" v	0°	45"	14"	
#2	1164.44'	66 HOBAS	S206-74	74" Ø (84.49" v	120°	45"	14"	

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	60" JOINT GASKET	60" Joint Gaske	3	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	10	
	96" JOINT GASKET	96" Joint Gaske	1	
A	30 IN X 6 IN POLYMER GRADE RING	MHGR30X6	1	280
B	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	MHC6030	1	2,200
C	60 IN X 6 FT POLYMER MH SECTION	MHS6072	1	2,900
D	60 IN X 5 FT POLYMER MH SECTION NO BELL	MHS6060NB	1	2,000
E	96 IN ECC POLYMER TRANSITION LID W/ 60" ACCESS	MHTL9660	1	6,300
F	96 IN X 8 FT POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	MHB9696.133	1	30,700
Totals (precast, lbs.)			6	44,380



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APPROVED BY: _____

Structure: MH 04 STA 200+00.00
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County

Based on:
Take Off By: 7/17/2025 PMcRae

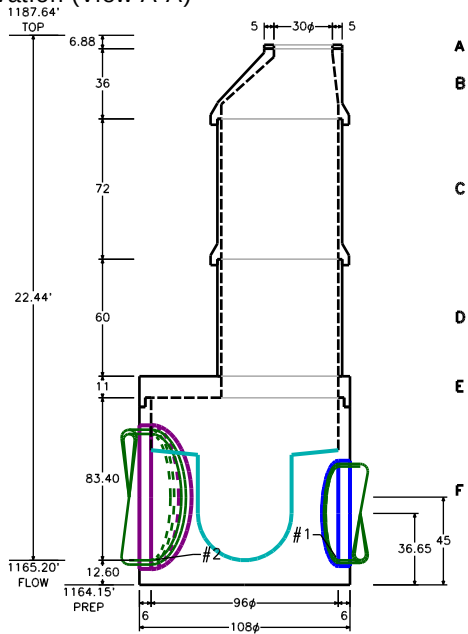
Contractor: All Bidders
Agency:
Engineer:

Design Build Height Rim + 1,187.64' Outlet Invert - 1,165.20' Design Height = 22.44'		Stack Build Height Ring + 0.39' Adjustment + 0.19' Eccentric Cone + 3.00' Risers + 11.00' Eccentric Slab + 0.92' Base Gain + 6.95' Design Height = 22.44' Floor Thickness + 1.05' Outside Height = 23.49'		Ship Date: Ring: 30" City of Chandler CAP R&C By Others Channel: FULL DEPTH PLUS 6" TROUGHING Reduction: Notes: Verify Opening #1 hole size that its correct for 50.8" O.D.
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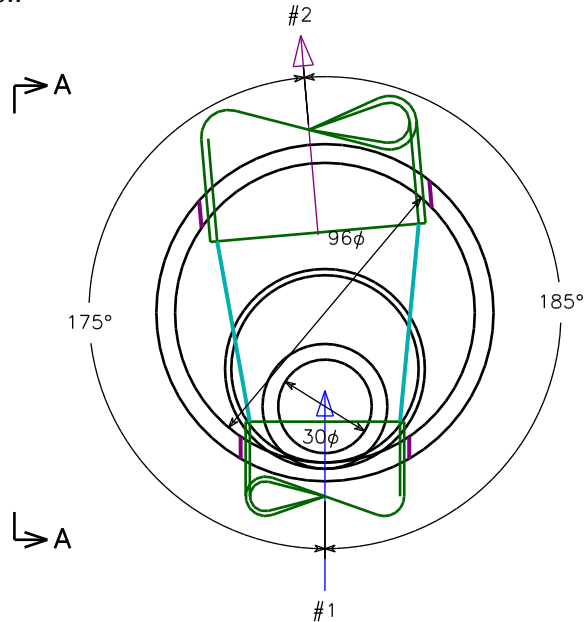
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.20'	48" PVC Sdr 35	S206-54L Boot	54" Ø	0°	36.65"	32.35"	48 Sdr35
#2	1165.20'	66 HOBAS	S206-74	74" Ø (84.49" v	175°	45"	14"	

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	60" JOINT GASKET	60" Joint Gaske	3	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	10	
	96" JOINT GASKET	96" Joint Gaske	1	
A	30 IN X 2 IN POLYMER GRADE RING	MHGR30X2	1	100
B	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	MHC6030	1	2,200
C	60 IN X 6 FT POLYMER MH SECTION	MHS6072	1	2,900
D	60 IN X 5 FT POLYMER MH SECTION NO BELL	MHS6060NB	1	2,000
E	96 IN ECC POLYMER TRANSITION LID W/ 60" ACCESS	MHTL9660	1	6,300
F	96 IN X 8 FT POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	MHB9696.133	1	34,900
Totals (precast, lbs.)			6	48,400



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APPROVED BY: _____

Structure: Structure C Full Depth+
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County
Contractor: All Bidders
Agency:
Engineer:

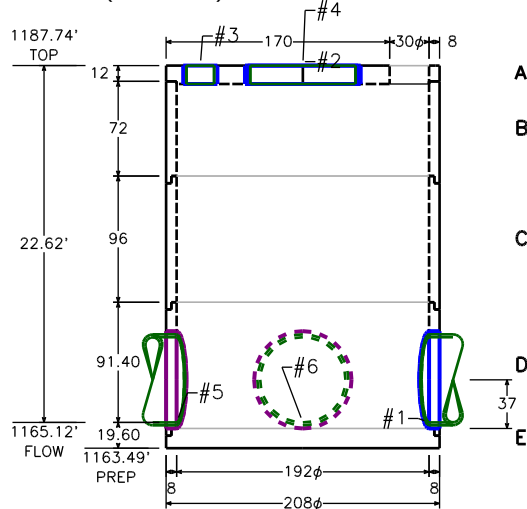
Based on:
Take Off By: 7/17/2025 PMcRae

Design Build Height Rim + 1,187.74' Outlet Invert - 1,165.12' Design Height = 22.62'	Stack Build Height Eccentric Slab + 1.00' Risers + 14.00' Last Riser Gain + 7.62' Design Height = 22.62' Floor Thickness + 1.63' Outside Height = 24.25'	Ship Date: Ring: 30" City of Chandler Sewer A-1700 R&C By Others Channel: FULL DEPTH PLUS 4" TROUGHING Reduction: Notes: Stop log storage racks supplied and installed by others.
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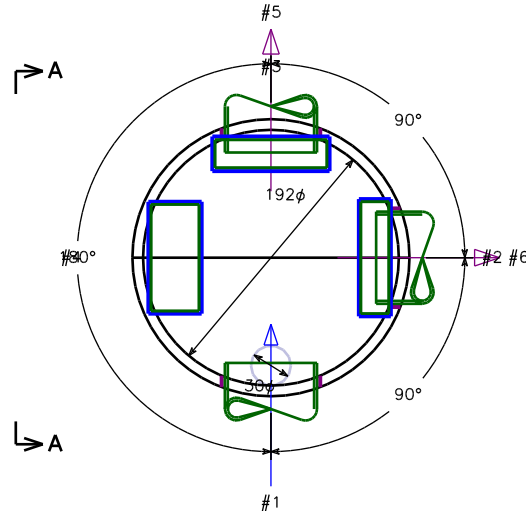
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.12'	66 HOBAS	S206-74	74" Ø (75.97" v	0°	37"	22"	
#2	1187.75'	21x84 Hole Box	Mortar	88"Wx24"H Bo	180°	6"	0"	
#3	1187.75'	84x21 Hole Box	Mortar	26"Wx88"H Bo	180°	6"	0"	
#4	1187.75'	80x36 Hole Box	Mortar	40"Wx84"H Bo	90°	6"	0"	
#5	1165.12'	66 HOBAS	S206-74	74" Ø (75.97" v	180°	37"	22"	
#6	1165.12'	66 HOBAS	S206-74	74" Ø (75.97" v	270°	37"	22"	

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	GASKET FOR 192" MANHOLE JOINTS	Gasket	3	
	LABOR FOR FIELD GROUTING		1	
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	72	
A	192 IN ECC SPLIT POLYMER LID FLAT W/ 30" ACCESS	MHL19230	1	30,400
B	192 IN X 6 FT POLYMER MH SECTION	MHS19272	1	31,600
C	192 IN X 8 FT POLYMER MH SECTION	MHS19296	1	40,600
D	192 IN X 8 FT POLYMER MH SECTION W/ CORED HOLE	MHS19296H	1	31,700
E	192 IN X 1 FT 3 IN POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	MHB19215.133	1	137,400
Totals (precast, lbs.)			5	271,700



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APPROVED BY: _____

Structure: Structure D

Job Number: AZ2402

Job Name: Price Road 66" Rehab (Insert Material)

Job Location: Chandler, AZ

Maricopa County

Contractor: All Bidders

Agency:

Engineer:

Based on:

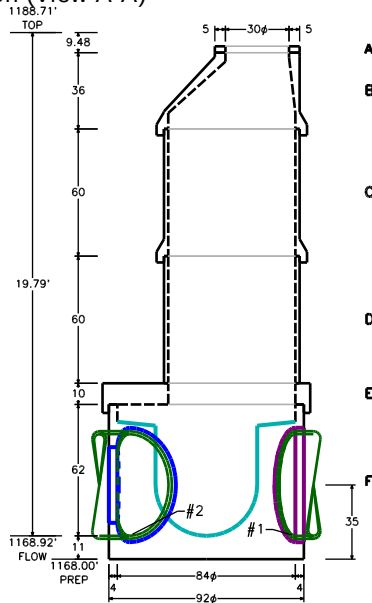
Take Off By: 7/17/2025 PMcRae

Design Build Height		Stack Build Height		Ship Date:	
Rim	+ 1,188.71'	Ring	+ 0.50'	Ring:	30" City of Chandler Sewer A-1700 R&C
Outlet Invert	- 1,168.92'	Adjustment	+ 0.29'	Channel:	FULL DEPTH PLUS 4" TROUGHING
Design Height	= 19.79'	Eccentric Cone	+ 3.00'	Reduction:	
		Risers	+ 10.00'	Notes:	
		Eccentric Slab	+ 0.83'		
		Base Gain	+ 5.17'		
		Design Height	= 19.79'		
		Floor Thickness	+ 0.92'		
		Outside Height	= 20.71'		

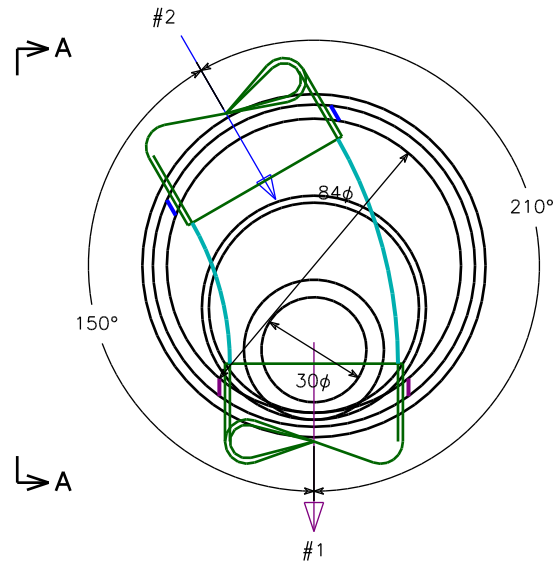
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1168.92'	48" HOBAS	S206-54L Boot	54" Ø	0°	35"	11"	48 HOBAS
#2	1168.92'	48" HOBAS	S206-54L Boot	54" Ø	150°	35"	11"	48 HOBAS

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	GASKET FOR 192" MANHOLE JOINTS	Gasket	3	
	GASKET FOR 48" MANHOLE JOINTS	Gasket	2	
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	10	
A	30 IN X 3 IN POLYMER GRADE RING	MHGR30X3	1	140
B	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	MHC6030	1	2,200
C	60 IN X 5 FT POLYMER MH SECTION	MHS6060	1	2,500
D	60 IN X 5 FT POLYMER MH SECTION NO BELL	MHS6060NB	1	2,000
E	84 IN ECC POLYMER TRANSITION LID W/ 60" ACCESS	MHTL8460	1	4,300
F	84 IN X 6 FT 1 IN POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	MHB8473.133	1	20,000
Totals (precast, lbs.)			6	31,140



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APPROVED BY: _____

Structure: Structure E Full Depth +
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County

Based on:

Take Off By: 7/16/2025 Joshua Johnson

Contractor: All Bidders

Agency:

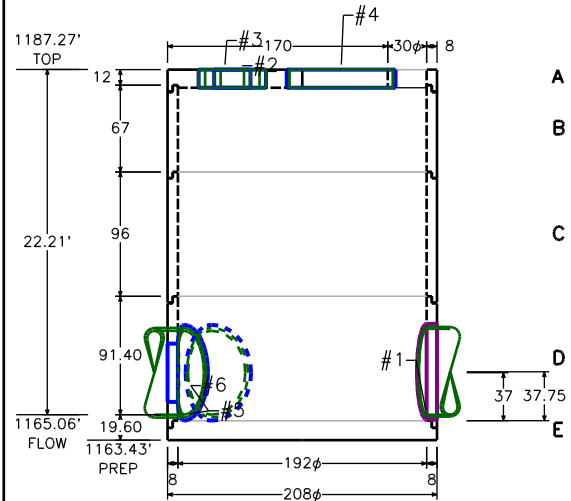
Engineer:

Design Build Height Rim + 1,187.27' Outlet Invert - 1,165.06' Design Height = 22.21'	Stack Build Height Adjustment + 0.01' Eccentric Slab + 1.00' Risers + 13.58' Last Riser Gain + 7.62' Design Height = 22.21' Floor Thickness + 1.63' Outside Height = 23.84'	Ship Date: Ring: 30" City of Chandler Sewer A-1700 R&C Channel: FULL DEPTH PLUS 4" TROUGHING Reduction: Notes:
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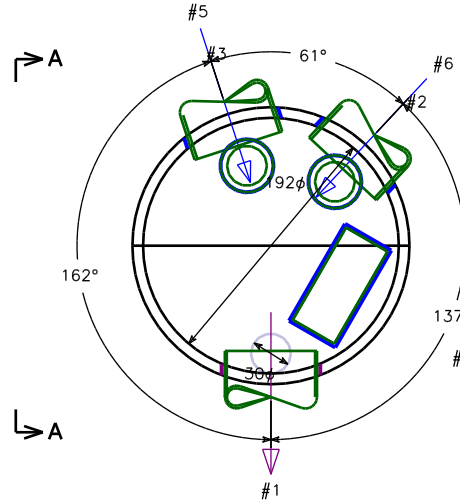
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.06'	66 FRP (Fiberglass)	S206-74	74" Ø (75.97" v	0°	37.75"	21.25"	66 FRP
#2	1187.00'	30"	Mortar	40" Ø in Lid	180°	6"	0"	
#3	1187.00'	30"	Mortar	40" Ø in Lid	180°	6"	0"	
#4	1187.00'	36"x80" Hole Box	Mortar	84"Wx40"H Bo	30°	6"	0"	
#5	1165.06'	66 HOBAS	S206-74	74" Ø (75.97" v	162°	37"	22"	
#6	1165.06'	66 HOBAS	S206-74	74" Ø (75.97" v	223°	37"	22"	

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	GASKET FOR 192" MANHOLE JOINTS	Gasket	3	
	LABOR FOR FIELD GROUTING		1	
	POLYMER GROUT KIT	PGROUT	1	
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	72	
A	192 IN ECC SPLIT POLYMER LID FLAT W/ 30" ACCESS	MHL19230	1	33,400
B	192 IN X 5 FT 7 IN POLYMER MH SECTION	MHS19267	1	28,300
C	192 IN X 8 FT POLYMER MH SECTION	MHS19296	1	40,600
D	192 IN X 8 FT POLYMER MH SECTION W/ CORED HOLE	MHS19296H	1	31,700
E	192 IN X 1 FT 3 IN POLYMER PRECAST BASE FULL PLUS DEPTH TROUGHING	MHB19215.133	1	137,800
Totals (precast, lbs.)			5	271,800



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APPROVED BY: _____

Structure: Tee 756004806
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County

Based on:
Take Off By: 7/17/2025 PMcRae

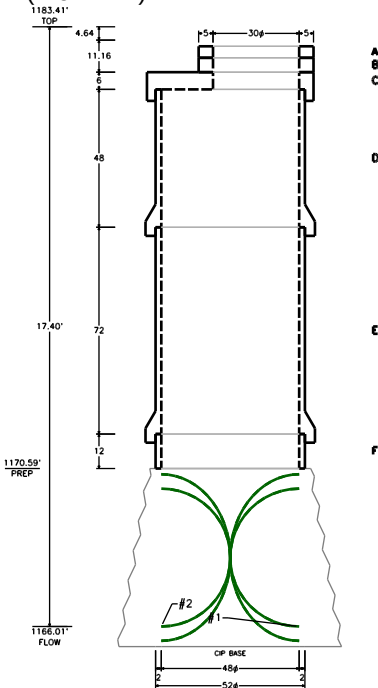
Contractor: All Bidders
Agency:
Engineer:

Design Build Height			Stack Build Height			Ship Date:	
Rim	+	1,183.41'	Ring	+	0.39'	Ring:	30" City of Chandler CAP R&C By Others
Outlet Invert	-	1,166.01'	Adjustment	+	0.93'	Channel:	BASE POURED IN FIELD BY CONTRACTOR
Design Height	=	17.40'	Eccentric Slab	+	0.50'	Reduction:	
Pipe Inner Dia.	-	4.00'	Risers	+	11.00'	Notes:	Confirm structure design
Pipe Wall	-	0.42'	Above Base	=	12.82'		
Pour Above	-	0.17'	Contractor Base	+	4.58'		
Above Base	=	12.82'	Design Height	=	17.40'		
Openings							

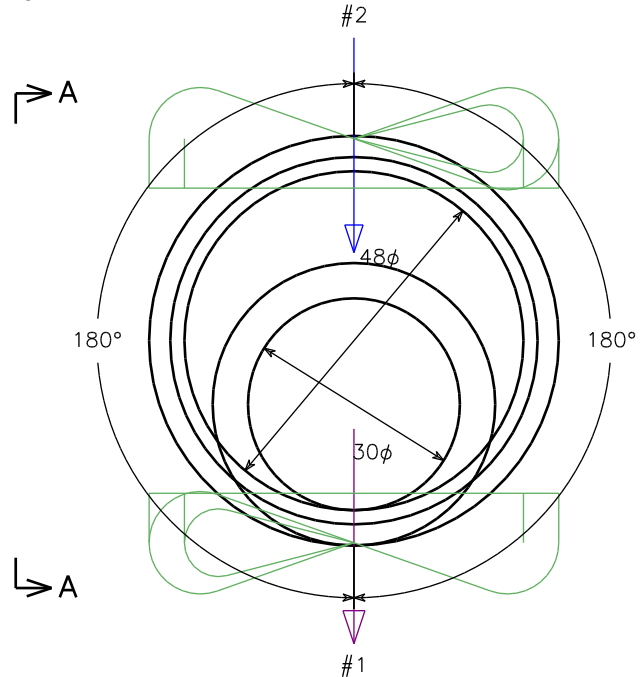
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1166.01'	48" RCP B	In 48 IN CIP Base	N/A	0°	31"	-6"	48 RCP(B)
#2	1166.01'	48" RCP B	In 48 IN CIP Base	N/A	180°	31"	-6"	48 RCP(B)

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	3	
A	30 IN X 4 IN POLYMER GRADE RING	MHGR30X4	1	190
B	30 IN X 5 IN POLYMER GRADE RING	MHGR30X5	1	240
C	48 IN ECC POLYMER LID FLAT W/ 30" ACCESS	MHL4830	1	1,200
D	48 IN X 4 FT POLYMER MH SECTION	MHS4848	1	1,700
E	48 IN X 6 FT POLYMER MH SECTION	MHS4872	1	2,400
F	48 IN X 1 FT POLYMER MH SECTION NO BELL	MHS4812NB	1	320
Totals (precast, lbs.)			6	6,050



1575 W Horizon Ridge Parkway
#530427
Henderson, NV 89012

207 Heritage Court
Sulphur Springs, TX 75482

1708 Turkey Creek Road
Plant City, FL 33566

702-824-9702
www.armorock.com

APPROVED BY: _____

Structure: Tee 756007414
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County
Contractor: All Bidders
Agency:
Engineer:

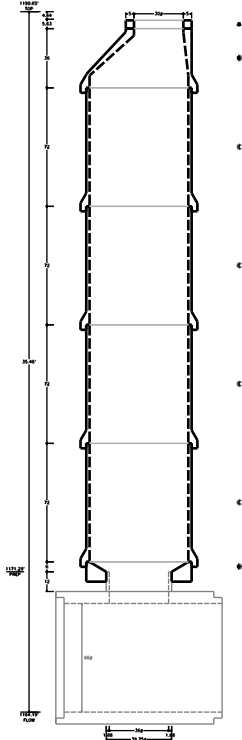
Based on:
Take Off By: 7/17/2025 PMcRae

Design Build Height		Stack Build Height		Ship Date:	
Rim	+ 1,199.65'	Ring	+ 0.39'	Ring:	30" City of Chandler CAP R&C By Others
Outlet Invert	- 1,164.19'	Adjustment	+ 0.47'	Channel:	BASE IS A MANHOLE TEE
Design Height	= 35.46'	Eccentric Cone	+ 3.00'	Reduction:	
		Risers	+ 24.00'	Notes:	Confirm structure design
		Concentric Slab	+ 0.50'		
		Tee Height	+ 1.00'		
		Above Crown	= 29.36'		
		Pipe ID & Wall	+ 6.10'		
		Design Height	= 35.46'		

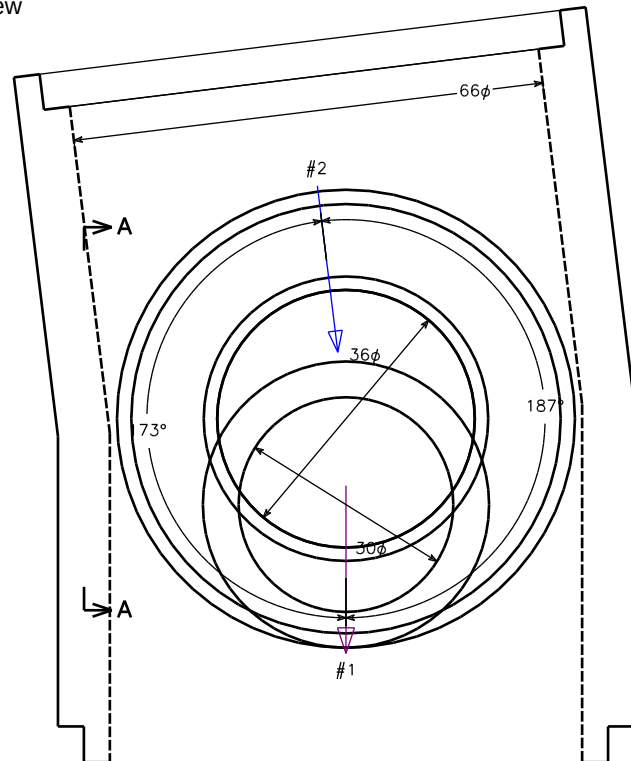
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1164.19'	66" RCP C	In 36 IN TEE	N/A	0°	40.25"	10.25"	66 RCP(C)
#2	1164.19'	66" RCP C	In 36 IN TEE	N/A	173°	40.25"	10.25"	66 RCP(C)

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	10	
A	30 IN X 5 IN POLYMER GRADE RING	MHGR30X5	1	240
B	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	MHC6030	1	2,200
C	60 IN X 6 FT POLYMER MH SECTION	MHS6072	4	11,600
D	36 IN TEE ADAPTOR 60 IN	56003-TL	1	
Totals (precast, lbs.)			7	14,040



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702-824-9702
www.armorock.com

APPROVED BY: _____

Structure: Tee 756009377

Job Number: AZ2402

Job Name: Price Road 66" Rehab (Insert Material)

Job Location: Chandler, AZ

Maricopa County

Contractor: All Bidders

Agency:

Engineer:

Based on:

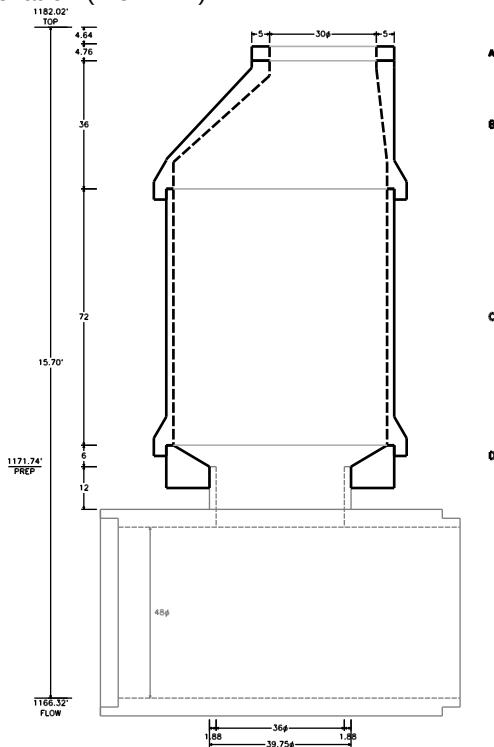
Take Off By: 7/17/2025 PMcRae

Design Build Height			Stack Build Height			Ship Date:		
Rim	+	1,182.02'	Ring	+	0.39'	Ring:	30" City of Chandler CAP R&C By Others	
Outlet Invert	-	1,166.32'	Adjustment	+	0.40'	Channel:	BASE IS A MANHOLE TEE	
Design Height	=	15.70'	Eccentric Cone	+	3.00'	Reduction:		
			Risers	+	6.00'	Notes:	Confirm structure design	
			Concentric Slab	+	0.50'			
			Tee Height	+	1.00'			
			Above Crown	=	11.28'			
			Pipe ID & Wall	+	4.42'			
			Design Height	=	15.70'			

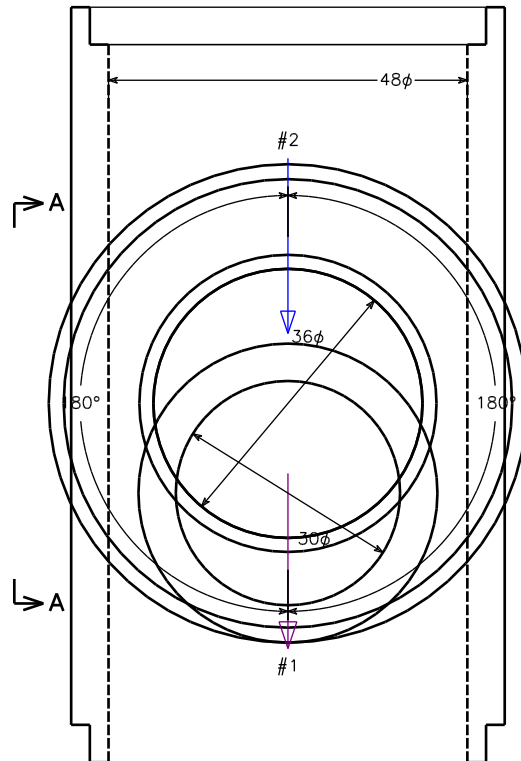
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1166.32'	48" RCP B	In 36 IN TEE	N/A	0°	29"	4"	48 RCP(B)
#2	1166.32'	48" RCP B	In 36 IN TEE	N/A	180°	29"	4"	48 RCP(B)

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	4	
A	30 IN X 4 IN POLYMER GRADE RING	MHGR30X4	1	190
B	60 IN X 3 FT ECC POLYMER CONE W/ 30" ACCESS	MHC6030	1	2,200
C	60 IN X 6 FT POLYMER MH SECTION	MHS6072	1	2,900
D	36 IN TEE ADAPTOR 60 IN	56003-TL	1	
Totals (precast, lbs.)			4	5,290



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207 Heritage Court
Sulphur Springs, TX 75482

1708 Turkey Creek Road
Plant City, FL 33566

702-824-9702
www.armorock.com

APPROVED BY: _____

Structure: Tee 756009378
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County

Based on:
Take Off By: 7/17/2025 PMcRae

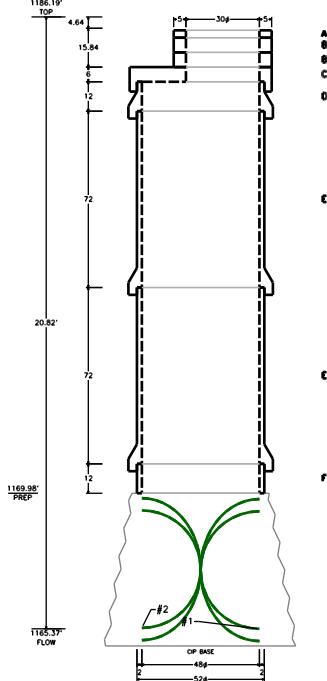
Contractor: All Bidders
Agency:
Engineer:

Design Build Height		Stack Build Height		Ship Date:	
Rim	+ 1,186.19'	Ring	+ 0.39'	Ring:	30" City of Chandler CAP R&C By Others
Outlet Invert	- 1,165.37'	Adjustment	+ 1.32'	Channel:	BASE POURED IN FIELD BY CONTRACTOR
Design Height =	20.82'	Eccentric Slab	+ 0.50'	Reduction:	
To Highest Pipe -	0.03'	Risers	+ 14.00'	Notes:	Confirm structure design
Pipe Inner Dia. -	4.00'	Above Base =	16.21'		
Pipe Wall -	0.42'	Contractor Base +	4.61'		
Pour Above -	0.17'	Design Height =	20.82'		
Above Base =	16.21'				

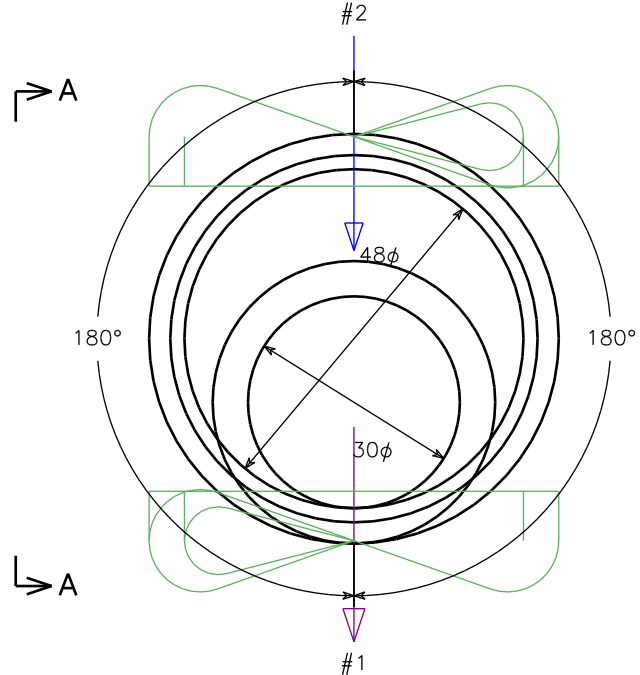
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.37'	48" RCP B	In 48 IN CIP Base	N/A	0°	31"	-5.64"	48 RCP(B)
#2	1165.40'	48" RCP B	In 48 IN CIP Base	N/A	180°	31.36"	-6"	48 RCP(B)

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	4	
A	30 IN X 3 IN POLYMER GRADE RING	MHGR30X3	1	140
B	30 IN X 6 IN POLYMER GRADE RING	MHGR30X6	2	560
C	48 IN ECC POLYMER LID FLAT W/ 30" ACCESS	MHL4830	1	1,200
D	48 IN X 1 FT POLYMER MH SECTION	MHS4812	1	730
E	48 IN X 6 FT POLYMER MH SECTION	MHS4872	2	4,800
F	48 IN X 1 FT POLYMER MH SECTION NO BELL	MHS4812NB	1	320
Totals (precast, lbs.)			8	7,750



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207 Heritage Court
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1708 Turkey Creek Road
Plant City, FL 33566

702-824-9702
www.armorock.com

APPROVED BY: _____

Structure: Tee 756011281
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County

Contractor: All Bidders
Agency:
Engineer:

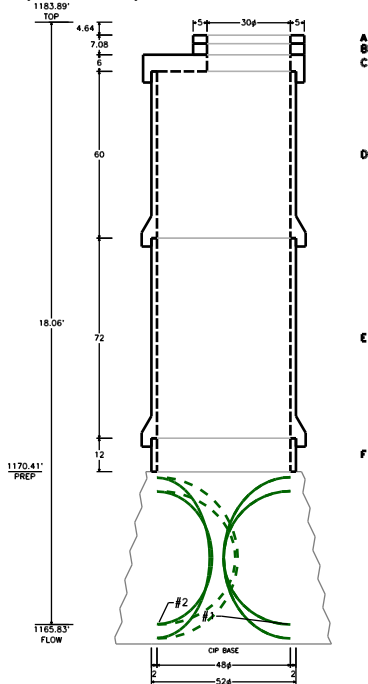
Based on:
Take Off By: 7/17/2025 PMcRae

Design Build Height		Stack Build Height		Ship Date:	
Rim	+ 1,183.89'	Ring	+ 0.39'	Ring:	30" City of Chandler CAP R&C By Others
Outlet Invert	- 1,165.83'	Adjustment	+ 0.59'	Channel:	BASE POURED IN FIELD BY CONTRACTOR
Design Height	= 18.06'	Eccentric Slab	+ 0.50'	Reduction:	
Pipe Inner Dia.	- 4.00'	Risers	+ 12.00'	Notes:	Confirm structure design
Pipe Wall	- 0.42'	Above Base	= 13.48'		
Pour Above	- 0.17'	Contractor Base	+ 4.58'		
Above Base	= 13.48'	Design Height	= 18.06'		

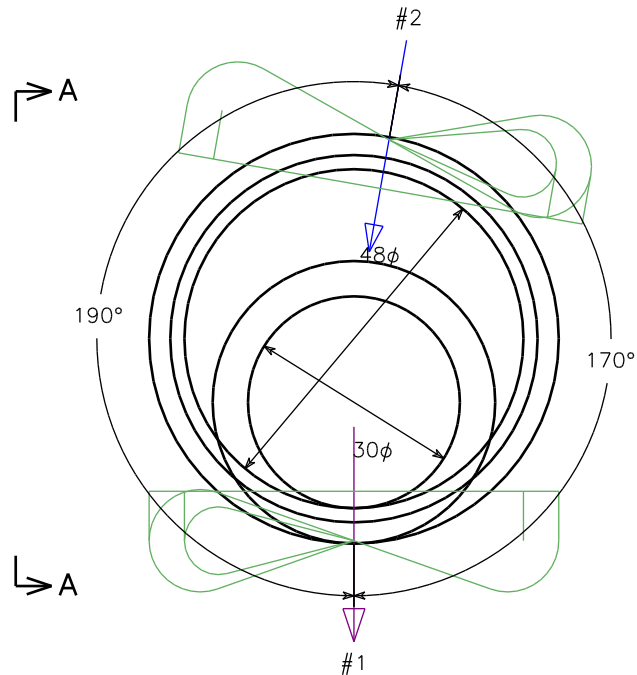
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.83'	48" RCP B	In 48 IN CIP Base	N/A	0°	31"	-6"	48 RCP(B)
#2	1165.83'	48" RCP B	In 48 IN CIP Base	N/A	190°	31"	-6"	48 RCP(B)

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	3	
A	30 IN X 3 IN POLYMER GRADE RING	MHGR30X3	1	140
B	30 IN X 4 IN POLYMER GRADE RING	MHGR30X4	1	190
C	48 IN ECC POLYMER LID FLAT W/ 30" ACCESS	MHL4830	1	1,200
D	48 IN X 5 FT POLYMER MH SECTION	MHS4860	1	2,000
E	48 IN X 6 FT POLYMER MH SECTION	MHS4872	1	2,400
F	48 IN X 1 FT POLYMER MH SECTION NO BELL	MHS4812NB	1	320
Totals (precast, lbs.)			6	6,250



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Henderson, NV 89012

207 Heritage Court
Sulphur Springs, TX 75482

1708 Turkey Creek Road
Plant City, FL 33566

702-824-9702
www.armorock.com

APPROVED BY: _____

Structure: Tee 756013317
Job Number: AZ2402
Job Name: Price Road 66" Rehab (Insert Material)
Job Location: Chandler, AZ
Maricopa County

Contractor: All Bidders
Agency:
Engineer:

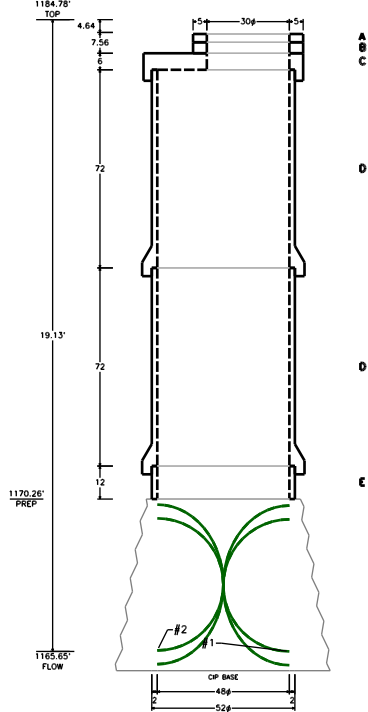
Based on:
Take Off By: 7/17/2025 PMcRae

Design Build Height		Stack Build Height		Ship Date:	
Rim	+ 1,184.78'	Ring	+ 0.39'	Ring:	30" City of Chandler CAP R&C By Others
Outlet Invert	- 1,165.65'	Adjustment	+ 0.63'	Channel:	BASE POURED IN FIELD BY CONTRACTOR
Design Height =	19.13'	Eccentric Slab	+ 0.50'	Reduction:	
To Highest Pipe -	0.03'	Risers	+ 13.00'	Notes:	Confirm structure design
Pipe Inner Dia. -	4.00'	Above Base =	14.52'		
Pipe Wall -	0.42'	Contractor Base +	4.61'		
Pour Above -	0.17'	Design Height =	19.13'		
Above Base =	14.52'				

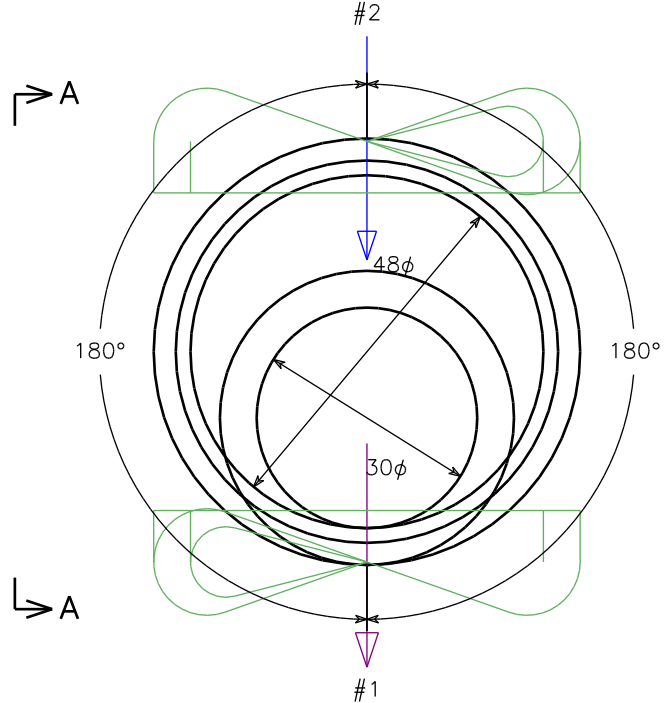
Openings

ID	Invert	Pipe Size	Connector	Hole Size	Angle	To CL	Top to Top	Hole Former
#1	1165.65'	48" RCP B	In 48 IN CIP Base	N/A	0°	31"	-5.64"	48 RCP(B)
#2	1165.68'	48" RCP B	In 48 IN CIP Base	N/A	180°	31.36"	-6"	48 RCP(B)

Elevation (View A-A)



Plan View



BOM For Stack

ID	Description	Part Number	Qty.	Weight
	1.25 IN X 14 FT JOINT MASTIC ROLL	MASJ	3	
A	30 IN X 3 IN POLYMER GRADE RING	MHGR30X3	1	140
B	30 IN X 4 IN POLYMER GRADE RING	MHGR30X4	1	190
C	48 IN ECC POLYMER LID FLAT W/ 30" ACCESS	MHL4830	1	1,200
D	48 IN X 6 FT POLYMER MH SECTION	MHS4872	2	4,800
E	48 IN X 1 FT POLYMER MH SECTION NO BELL	MHS4812NB	1	320
Totals (precast, lbs.)			6	6,650



SONCO
CONSTRUCTION

4927 Toronto Ave

Fontana CA 92336

(909) 223-1280 / (909) 260-4749

license #911666 / dir #1000024656

PROPOSAL

To:	Travis Monk	Proposal #	1800-11-02
Company:	B&F Contracting	Proposal Date:	07/18/25
		Project Name	Chandler 66
Phone:	602-326-9282	Project Address:	Chandler AZ
Email	Tmonk@bfcontracting.com		

UNITS	DESCRIPTION	UNIT PRICE	LABOR and MATERIAL
	Furnish and install ArmorLok PVC Liner, to new Junction structure.		
	Junction Structure B		
	Install Armorlok Liner to wall, Deck and around all Pipe ends. Weld all seam and spark test. Approx: 3810 sqft		\$ 102,870.00
	Install Armorlok Shieldlok system to all grout fill. Weld all seams and sparktest. Approx:1200 sqft		\$ 90,000.00
			\$ 192,870.00

***Included in this Proposal**

- (1) All labor and equipment necessary. (2) ArmorLok Certified PVC welding Technicians.
- (3) Unlimited access to work area. (4) Typical ventilation (5) Top man and confined space restrictions
- (6) must be clean and dry

*** Excluded from this Proposal**

- (1)Traffic control. (2) Inspection (3) Exterior water proofing. (4) Sewer bypass and/or plugs.
- (5) Damage caused to liner due to operations of other trades. (6) Spray coatings.
- (7) Permits, Bonds, Liquidated Damages. (8) Dewatering

Sonco Construction, Inc.

Prime Contractor

By: Richard Chico

By:

Date:

Date:

PAYMENT CONDITIONS

BILLING TURNED IN BY THE 25th OF EACH MONTH WILL BE PAID BY THE 10th DAY OF THE FOLLOWING MONTH

SIGNATURE OF THIS DOCUMENT IS TO BE CONSIDERED ACCEPTANCE OF THESE TERMS AND AN AUTHORIZATION TO PROCEED.

THIS PROPOSAL MAY BE SUBJECT TO CHANGE OR CANCELLATION IF NOT ACCEPTED WITHIN 30 DAYS.



Quote Number: **Q10018482-2**
Created On: 7/17/2025
Quote Expires On: 8/16/2025

Project: **COC- CMAR 66" Sewer**
Project Address: S Price Rd & 202 South
Chandler, AZ 85224
Estimated Start Date: 9/1/2025
Project End Date: 4/1/2026

Account: **B & F Contracting Inc**
Account Number: 40032
Contact: Wes Johnston
E-mail: wesley.johnston@bfcontracting.com
Phone: 480-489-9893

Customer Service:
Sales Office Address: PO Box 741805 Kilgore Companies
Los Angeles, CA 90074-1805

READY-MIX (SELECTED AS LOWEST)

MATERIAL PRICING

Item Code	Description	Quantity	Unit Price
PHCLSM	1/2 SACK ABC SLURRY (COP) <i>Escalator of \$2.00 effective 10/1/2025 recurring every 12 months</i>	5,400 CY	\$97.00
13004	MAG-A 3000 PSI <i>Escalator of \$4.00 effective 10/1/2025 recurring every 12 months</i>	60 CY	\$136.00
14004	MAG-AA 4000 PSI, 1" ROCK <i>Escalator of \$4.00 effective 10/1/2025 recurring every 12 months</i>	30 CY	\$142.00
50020	2000 PSI GROUT W/ASH, ASTM 476 <i>Escalator of \$4.00 effective 10/1/2025 recurring every 12 months</i>	210 CY	\$151.00

SUNDRIES AND SURCHARGES

Item	Comments	Price	Unit
Fuel Surcharge RMX		\$35.00	EA
WEEKEND TRUCK TIME		\$50.00	HRS
ENVIRONMENTAL CHARGE		\$25.00	EA
Load Size: 0.00 > 6.00		\$200.00	CY
Additional Products & Services			
Hot Water		\$2.50	PCY
Recover per 1/2 hour		\$4.00	OZ
Micro Fiber		\$7.50	PCY
Color Wash Out		\$25.00	EA
Nitrogen Cooling		\$9.50	GAL
Non-Chloride Accerator (per 1%)		\$0.00	GAL
Macro Fiber (Structural)		\$7.00	PCY
Color Seal		\$0.00	PCY
Chilled Water		\$2.50	PCY

Respectfully,

Arizona Materials

Sales Rep: Randy Schmolke
Phone: 602-342-6595
Email: rschmolk@summit-materials.com

Customer Acceptance

Signed by:

Date:

☐ Taxable ☐ Tax Exempt (Attach Certificate)

QUOTE NUMBER: 155376
SALES REP: Villegas, Raul
(480) 298-0157
Raul.Villegas@MartinMarietta.com

PROJECT NAME 66"- REDUNDANT SEWER PH II		JOB / P. O. NUMBER 83734 /		QUOTE DATE 7/17/2025 2:12:25 PM	
PROJECT LOCATION PRICE RD AND LOOP 202 CHANDLER, AZ				BID DATE March 24, 2025	
CUSTOMER B & F CONTRACTING INC CONTACT TRAVIS MONK TMONK@BFCONTRACTING.COM STREET ADDRESS 11011 N 23RD AVE CITY, STATE, ZIP PHOENIX, AZ 85029-4828 CUSTOMER NO. 992678				PROJECT START DATE July 14, 2025	
				QUOTE EXPIRATION DATE March 31, 2026	
				See Price Escalators	

PRODUCT CODE	MATERIAL DESCRIPTION	UOM	USAGE	SLUMP	QTY	PRICE
PHCLSM	1/2 SACK ABC SLURRY (PHCLSM1/2)	CY	SLURRY	7-11	5400	\$104.00
2303141	MAG A, 1"	CY	SITE WORK	3-5	60	\$140.00
2403141	MAG AA, 1"	CY	SITE WORK	3-5	22	\$145.00
422033A	2000 PSI-F, 30% 3/8" GROUT (ASTM C-476)	CY	GROUT	8-11	210	\$149.00

ADDITIONAL COMMENTS *PRICES DO NOT INCLUDE TAX*

ESCALATION NOTES:
 CUBIC YARD PRICE INCREASE: 1/1/2026 \$8.00

Add: **Add Miscellaneous Extra Charges:**

\$10.00 Per Load Transportation Surcharge - Add to Above Pricing \$1.00 Per Cubic Yard Environmental Fee - Add to Above Pricing \$1.00 Per Cubic Yard Tempered Water Service Fee \$10.00 Per Cubic Yard Saturday Premium Charge	\$8.00 Per Cubic Yard per each 1% Non Chloride Accelerator (available in 0.25% increments) \$3.00 Per Cubic Yard per Each 30 Minute Increment of Hydration Stabilizer (Retarder) \$7.00 Per Cubic Yard Mono/Micro Fiber (1LB per Cubic Yard) \$4.00 Per Pound of Color per Sack of Cement per Cubic Yard \$50.00 Color Washout Fee per Load \$250.00 Per Load Short Load Delivery Fee (Orders of 6 CYs or Less) \$2.00 Per Minute of Wait Time per Load After 45 Minutes (on job standby) \$5.00 Per Gallon Nitro (3 gallons is equivalent to 20 LBs. of ice) Call for Availability \$0.75 Per Pound Ice \$8.00 Per Cubic Yard Cancellation Fee within 24 Hours of Scheduled Start Time (\$200 minimum fee) \$100.00 Per Load Multi-Stop Delivery Fee (per stop)
---	---

Above pricing is valid for 30 days from quote date, written quote acceptance is required to hold pricing beyond 30 days.
 Prices are FOB job site unless noted otherwise and do not include taxes. Sales Tax owed unless exempt documentation is received before the first pour.
 See item 7. TERMS: NET 30 DAYS. If not paid when due, Applicant agrees to pay interest charges at the rate of 1.5% per month or the maximum lawful rate, whichever is lower, and Seller's reasonable costs of collection, including reasonable attorneys' fees.

Signature: _____ Quote Date: 7/17/2025 Accepted by: _____

Please Note

All concrete as bid is per 90 minute max delivery and 95 degree max concrete temperature unless noted otherwise. All Concrete as Bid contains Type F Ash and normal weight aggregate unless specifically noted. All concrete pricing above is valid from Monday thru Friday during normal business hours unless noted otherwise. Aggregates are a naturally occurring material which can contain particles that degrade or stain concrete. It is not possible to completely remove all such particles; however, the occurrence is infrequent, and the bulk aggregate meets the Limits for Deleterious Substances and Physical Property Requirements of Coarse Aggregate for Concrete as outlined in ASTM C 33. Martin Marietta does not warranty against these infrequent occurrences.

Minimum amount of standard concrete for deliveries is 2 cubic yards, specialty concrete is 4 cubic yards. Buyer is responsible for safe access to work area and wash out containment area for all deliveries.



DESERT READY MIX

READY MIX CONCRETE SALES AND SERVICE

4011 PRESIDIO STREET

MESA, AZ 85215

OFFICE: 480-428-2280 FAX: 480-985-4248

Begin Date: February 19, 2025
Expiration Date: December 31, 2025

Quote # : 25967
MAP/GRID

Customer: **B & F CONTRACTING INC**
Customer Code: BFCO001
Address: 11011 N 23RD AVE
PHOENIX, AZ 85029
Contact: NANCY A/P
Phone: 623-582-1170 O
Fax:
Project: **CITY OF CHANDLER CMAR 66-INCH REDUNDANT**
Job Address: PRICE ROAD & LOOP 202
Terms Description: **2% 10TH NET 30**

Item Code	Item Description	Est. Qty	Price
S500501	PHCLSM.5 (1/2 SK SLURRY COP)	5,400	\$108.00
SRP104	1/2SK MAG CLSM		\$108.00
TC	TEMPERATURE CONTROL		\$1.00
A203001	MAG A 3000 PSI W/ASH	60	\$140.00
A204001	MAG AA 4000 PSI W/ASH	22	\$148.00
G402061	2000 PSI GROUT	210	\$165.00
AG010003 DELIVERED	MAG SPEC ABC (NON-LIME)	15,000	\$20.00
AG020003 DELIVERED	CONCRETE SAND	200	\$25.00
AG040002 DELIVERED	3/8 REJECT BEDDING CHIPS ***IF AVAILABLE***	1,000	\$18.00
AG040001 DELIVERED	#8 (3/8 CONCRETE ROCK)		\$23.25

Ready Mix Additional Fees

Fee ID *	UOM	Price
E01	cy	3.25
FSC	LOAD	TBD

Ready Mix Minimum Load Fee

Less than or equal to	UOM	Price
2	Load	0
3.5	Load	0
5.5	Load	0

- Normal operating hours are Mon-Fri, Sat, Sun or Holidays are subject to surcharges.
- Color added at no expressed guarantee.
- Drivers do not accept cash or make change.
- Desert Ready Mix reserves the right to increase prices due to RAW Material cost increases such as Cement, Fly Ash, Aggregates or unexpected Fuel increases.
- All specialized aggregate sales are based on availability.
- Deliveries less than 6 Cubic yards are subject to our minimum load charges.
- This quote does not guarantee product or service availability
- All Aggregates and Concrete loads have an additional fuel surcharge.

Quoted By: JUSTIN DAWSON

* E – Environmental | S – Saturday Fee | F – Fuel Surcharge

Quote Number: Q25-100-Rev.02
Date of Quote: 7/16/2025
Project Name: **Chandler 66" Redundant Sewer CMAR**
Jobsite Location: Chandler, Arizona
Owner: City of Chandler
Bid Date: N/A
Type of Job: Gravity Sewer
Agent: N/A

Hobas Pipe USA, Inc.
 1413 E. Richey Road
 Houston, Texas 77073
 +1 (800) 856-7473



FIBERGLASS PIPE PRODUCTS SUPPLY-QUOTE

To assist you in preparing your bid for the above referenced project, we are pleased to offer you the following prices for your consideration:

FIBERGLASS PIPE								
QUANTITY	UNIT	SIZE (DN) in	CLASS (PN) PSI	STIFFNESS (SN) PSI	TYPE OF JOINT	PIPE LENGTH LF	UNIT PRICE USD \$	TOTAL USD \$
640	LF	66"	0 PSI	71 PSI	Flush Reline*	20'	\$ 383.00	\$ 245,120
960	LF	66"	25 PSI	46 PSI	Coupling*	20'	\$ 330.00	\$ 316,800
220	LF	48"	25 PSI	46 PSI	Coupling*	20'	\$ 186.00	\$ 340,800
42	EA	48"-66"	Freight Per Truck (Houston to Above Jobsite)**				\$ 2,800.00	\$ 117,600
* No Freight Included - See Separate Freight Rates Above								
** Freight Costs Shown Are Based on Rates at Bid Time, and on Quantities Shown. Buyer Will pay								
Actual Freight Invoice Amounts. Any Increase or Decrease in the freight is for Account of the Buyer.								
Hobas Package Total: \$								1,020,320

Remarks:

Line Item quantities are based on available information, if it differs use Unit Pricing

Additional Information:

- The prices quoted above are firm for acceptance within 30 days of latest quote date. Final prices are valid for 1 year after receipt of Order Confirmation.
- Applicable sales taxes are to be added to the net amount of the quoted prices on transactions which are not exempt. Buyer is responsible for all applicable taxes.
- Terms: Net 30 days from date of invoice, subject to credit approval. Payments to be made via ACH. Payment bonds required on all public jobs. Private jobs without payment bonds will be required to pay 50% upfront with balance due in 30 days or provide a "Standby Letter of Credit".
- Prior to agreeing to a delivery start date and delivery rate, HOBAS requires a signed quotation or acceptable purchase order, receipt of approved submittal, and payment bond information.
- Freight Terms: F.O.B. above referenced Jobsite full freight quantities, with unloading by others (Incoterms 2020).
- HOBAS manufactures its pipe products to the specifications provided by the engineer and/or project owner and warrants them to be in line with such specifications as set forth in the attached General Terms, unless stated herein or any accompanying documentation.
- HOBAS cannot be responsible for and makes no warranties or representations as to the selection of a suitable pipe design for any specific job and/or methods used in installation of the pipe products.
- To optimize shipments, pipe diameters 51" thru 60" ship on steel racks & must ship in multiples of 10 loads over a 10 day time period. Contractor must store & reload racks for return to yard for prices quoted to be valid. If contractor does not wish to do this, alternate, increase freight rates are available.
- The enclosed HOBAS General Terms are hereby incorporated and expressly apply to this quotation.

ACCEPTANCE

Subject to being awarded the above referenced project we hereby order the outlined pipe products.

COMPANY: _____

BY: _____

TITLE: _____

For information concerning this quote contact

Stephen Alston - Area Manager

Cell Phone (801) 888-2772

Quotation Approved by:

General Terms in the next pages





Fullerform Systems Inc.
24 East Pioneer St.
PHOENIX, AZ 85040
Phone (602) 268-5701



Quotation

EXPIRATION DATE	QUOTE NUMBER
03/30/2025	S100052097
Fullerform Systems Inc. 24 East Pioneer St. PHOENIX, AZ 85040 Phone (602) 268-5701	PAGE NO.
	1 of 1

QUOTE TO:

SHIP TO:

B&F CONTRACTING, INC
11011 N. 23RD AVENUE
PHOENIX, AZ 85029

B&F CONTRACTING, INC
11011 N. 23RD AVENUE
PHOENIX, AZ 85029

CUSTOMER NUMBER	CUSTOMER PO NUMBER	JOB NAME / RELEASE NUMBER	SALESPERSON		
129	City of Chandler Redundant Sew		BRYCE YNCERA		
WRITER		SHIP VIA	TERMS	SHIP DATE	FREIGHT ALLOWED
JJ PARKINSON			1% disc 20 days, Net 30	02/28/2025	No
ORDER QTY	DESCRIPTION			UNIT PRICE	EXT PRICE
	Project: City of Chandler Redundant Sewer Spec: City of Chandler Bid Date: 7/18/2025 Pricing Valid: 30 Days 48" Heavy Wall Sewer Pipe -----				
224ea	48" PS115 SDR26 PVC Non Return / Non Cancellable			344.681/ea	77208.52
2ea	3DT-GREEN / 3DT-SEWER 3" X 1000' ROLL OF DETECTABLE SEWER TAPE GREEN			40.500/ea	81.00
2ea	14UF GREEN WIRE X 500			76.667/ea	153.33
*ALL PRODUCTS WILL BE PRICED IN EFFECT AT TIME OF SHIPMENT. *DUE TO SUPPLY CHAIN PRICE INCREASES.				Subtotal	77442.85
				S&H Charges	0.00
				Estimated Tax	0.00
				Amount Due	77442.85



TRENCH SAFETY

PROPOSAL

To: B&F Contracting

Date: 7/17/2025

Project: 101/202 66 Inch Redundant Sewer Rehab

Project Location: Chandler, AZ

Plans: United Rentals (North America), Inc. ("United Rentals" or "Company") proposes to furnish equipment ("Equipment") and engineering for a shoring system (the "System") as follows.

Shoring System:

System Type: 4-Sided Slide Rail Pit Application. **(Engineering review required)**

System Dimensions: 48' wide x 48' long x 32' deep.

Phasing Required: Yes, Removal of rolling strut carts after Tie Backs and Sacrificial are installed.

Sheeting Required: No.

Benching/Sloping Required: No.

Soil Description: Soil is dense sand over very dense sand over medium dense sand based on Bore B-1 by Ninyo & Moore dated 2-25-21.

Dewatering Required: Base of excavation on interior and exterior of system.

Surcharge Setback: HS20-44 Vehicular Traffic 20', Waterways 30', Structures 50'.

Additional Notes: The proposed system is based on the information available, adverse site or soil conditions may affect design and engineering.

Rental:

Daily Rate before taxes and freight	\$ 10,087.31
Weekly Rate before taxes and freight	\$ 20,174.62
Monthly Rate before taxes and freight	\$ 50,436.56
Total Delivery and Pickup of all Equipment (12 Trucks Each Way)	\$ 20,800.00
Rigging Box RPP	\$ 151.05

Sale Item :

Sacrificial Beams (2 at 50')	\$ 15,000.00
Beam Price to be quoted at time of installation due to market fluctuation	
Engineering Fee (see Note 6)	\$ 4,000.00
Shoring Advisor: (\$750 day estimated 4 days)	\$ 3,000.00

Below are optional equipment and/ or services available upon request:

Pertaining to Slide Rail Shoring systems only

Panel Puller (see Note 14)		\$975.00
Rental Protection Plan System (See Note 13)	15% of Rental	\$

Pertaining to Sheeting, Bracing and beam systems only

Vibratory Hammer Rental Weekly Rate (HPSI 80E or equivalent)	\$ XXXX	Initial_____
---	---------	--------------

Please select one of the following options:

Sheeting Service Fee (see Note 11)	10% of sheet rental up to 1 st month only	Initial_____
Rental Protection Plan (see Note 12)	15% of total rental contract for full duration	Initial_____



TRENCH SAFETY

PROPOSAL

To: B&F Contracting

Date: 7/17/2025

Project: 101/202 66 Inch Redundant Sewer Rehab

Project Location: 331 S Price Rd. Chandler, AZ 85224

Plans: United Rentals (North America), Inc. ("United Rentals" or "Company") proposes to furnish equipment ("Equipment") and engineering for a shoring system (the "System") as follows.

Shoring System:

System Type: 4-Sided Slide Rail Pit Application

System Dimensions: 20' wide x 20' long x 27' deep.

Phasing Required: No.

Sheeting Required: Yes, Lower 8' on (3) adjacent walls.

Benching/Sloping Required: No.

Soil Description: Soil is dense sand over very dense sand over medium dense sand based on Bore B-1 by Ninyo & Moore dated 2-25-21. Dewatering Required: Base of excavation on interior and exterior of system.

Surcharge Setback: HS20-44 Vehicular Traffic 20', Waterways 30', Structures 50'.

Additional Notes: The proposed system is based on the information available, adverse site or soil conditions may affect design and engineering.

Rental:

Daily Rate before taxes and freight	\$ 2,306.37
Weekly Rate before taxes and freight	\$ 4,612.75
Monthly Rate before taxes and freight	\$ 11,531.87
Total Delivery and Pickup of all Equipment (4 Trucks Each Way)	\$ 400.00
Rigging Box RPP	\$143.85

Sale Item :

Engineering Fee (see Note 6)	\$ 2,750.00
Shoring Advisor: (\$750 day estimated 2 days)	\$ 1,500.00

Below are optional equipment and/ or services available upon request:

Pertaining to Slide Rail Shoring systems only

Panel Puller (see Note 14) \$975.00

Rental Protection Plan System (See Note 13) 15% of Rental \$ XXXX

Pertaining to Sheeting, Bracing and beam systems only

Vibratory Hammer Rental Weekly Rate (HPSI 80E or equivalent) \$ XXXX Initial _____

Please select one of the following options:

Sheeting Service Fee (see Note 11)	10% of sheet rental up to 1 st month only	Initial _____
Rental Protection Plan (see Note 12)	15% of total rental contract for full duration	Initial _____

BRANCH DAB
503 W 3RD AVE
MESA AZ 85210-1246
602-357-7670
602-357-7671 FAX

250597626

Job Site

101/202 66" SEWER LINE REHAB
331 S PRICE RD
CHANDLER AZ 85224-4900

Office: 623-582-1170

B & F CONTRACTING
A1011 N 23RD AVE
PHOENIX AZ 85029-4828

Customer # : 220648
Quote Date : 07/17/25
Estimated Out : 09/30/25 09:00 AM
Estimated In : 10/28/25 09:00 AM
UR Job Loc : 331 S PRICE RD, CHAN
UR Job # : 895
Customer Job ID :
P.O. # : QUOTE
Ordered By : TRAVIS MONK
Written By : BRANDON SOWELL
Salesperson : BRANDON SOWELL

**This is not an invoice
Please do not pay from this document**

RENTAL ITEMS:

Qty	Equipment	Description	Minimum	Day	Week	4 Week	Estimated Amt.
2	9418471	TRENCH BOX 8' X 24' (6" FB)		80.14	320.54	961.63	1,923.26
1	9418566	TRENCH BOX 10' X 24' (6" FB)		101.51	406.02	1,218.07	1,218.07
1	9418530	TRENCH BOX 10' X 16' (6" FB)		76.93	307.72	923.17	923.17
2	9418435	TRENCH BOX 8' X 16' (6" FB)		64.11	265.44	769.31	1,538.62
6	940/5620	20' END LOADING SPREADER		63.00	252.40	757.00	4,542.00
18	940/5538	SPREADER BAR (KIT) 8" X 20'		21.37	106.85	320.54	5,769.72
360	940/5655	8" SPREADER BAR (LINEAR FEET)					
16	941/8103	STACKING PINS FOR STEEL BOXES		1.12	2.04	5.09	81.44
1	940/9560	LADDER HANGER FOR TRENCH SHIELD		2.49	9.97	19.92	19.92
1	6759415	TRENCH BRIDGE STEEL 8'		14.86	74.28	222.85	222.85
1	330/7120	LIFTING BRIDLE 4 LEG X 20'		18.57	74.28	222.85	222.85
6	944/9944	ROAD PLATE 8' X 16'		10.58	74.08	296.33	1,777.98
300	942/7280	SHEETING PZ27 PAIRED SHEETS (LINEAR FT)		1.24	2.17	4.96	1,488.00
		(10) 30' SHEETS					

Rental Subtotal: 19,727.88

SALES/MISCELLANEOUS ITEMS:

Qty	Item	Price	Unit of Measure	Extended Amt.
1	PROPERTY TAX SURCHARGE [DRSURAZ/MCI]	295.920	EACH	295.92
1	DELIVERY CHARGE	215.000	EACH	215.00
1	PICKUP CHARGE	215.000	EACH	215.00

Sales/Misc Subtotal: 725.92

Agreement Subtotal: 20,453.80

Tax: 1,697.68

Estimated Total: 22,151.48

COMMENTS/NOTES:

CONTACT: TRAVIS MONK
CELL#: 602-326-9282

This proposal may be withdrawn if not accepted within 30 days. The above referenced Rental Protection Plan, environmental, and tax charges are estimates and are subject to change.

NOTICE: This is not a rental agreement. The rental of equipment and any items listed above is subject to availability and subject to the terms and conditions of the Rental and Service Agreement, which are available at <https://www.unitedrentals.com/legal/rental-service-terms-US> and which are incorporated herein by reference. A COPY OF THE RENTAL AND SERVICE AGREEMENT TERMS ARE AVAILABLE IN PAPER FORM UPON REQUEST.

BRANCH DA8
503 W 3RD AVE
MESA AZ 85210-1246
602-357-7670
602-357-7671 FAX

250597793

Job Site

101/202 66" SEWER LINE REHAB
331 S PRICE RD
CHANDLER AZ 85224-4900

Office: 623-582-1170

B & F CONTRACTING
A1011 N 23RD AVE
PHOENIX AZ 85029-4828

Customer # : 220648
Quote Date : 07/17/25
Estimated Out : 09/30/25 09:00 AM
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UR Job Loc : 331 S PRICE RD, CHAN
UR Job # : 895
Customer Job ID :
P.O. # : QUOTE
Ordered By : TRAVIS MONK
Written By : BRANDON SOWELL
Salesperson : BRANDON SOWELL

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Please do not pay from this document**

RENTAL ITEMS:

Qty	Equipment	Description	Minimum	Day	Week	4 Week	Estimated Amt.
2	9418471	TRENCH BOX 8' X 24' (6" FB)		80.14	320.54	961.63	1,923.26
1	9418566	TRENCH BOX 10' X 24' (6" FB)		101.51	406.02	1,218.07	1,218.07
1	9418530	TRENCH BOX 10' X 16' (6" FB)		76.93	307.72	923.17	923.17
2	9418435	TRENCH BOX 8' X 16' (6" FB)		64.11	265.44	769.31	1,538.62
16	941/8103	STACKING PINS FOR STEEL BOXES		1.12	2.04	5.09	81.44
1	940/9560	LADDER HANGER FOR TRENCH SHIELD		2.49	9.97	19.92	19.92
1	6759415	TRENCH BRIDGE STEEL 8'		14.86	74.28	222.85	222.85
1	330/7120	LIFTING BRIDLE 4 LEG X 20'		18.57	74.28	222.85	222.85
6	944/9944	ROAD PLATE 8' X 16'		10.58	74.08	296.33	1,777.98
300	942/7280	SHEETING PZ27 PAIRED SHEETS (LINEAR FT) (10) 30' SHEETS		1.24	2.17	4.96	1,488.00
18	940/5530	SPREADER BAR (KIT) 8" X 16'		21.37	106.85	320.54	5,769.72
288	940/5655	8" SPREADER BAR (LINEAR FEET)					
6	940/5616	16' END LOADING SPREADER		50.40	201.92	605.60	3,633.60
						Rental Subtotal:	18,819.48

SALES/MISCELLANEOUS ITEMS:

Qty	Item	Price	Unit of Measure	Extended Amt.
1	PROPERTY TAX SURCHARGE [DRSURAZ/MCI]	282.294	EACH	282.29
1	DELIVERY CHARGE	215.000	EACH	215.00
1	PICKUP CHARGE	215.000	EACH	215.00
Sales/Misc Subtotal:				712.29
Agreement Subtotal:				19,531.77
Tax:				1,621.15
Estimated Total:				21,152.92

COMMENTS/NOTES:

CONTACT: TRAVIS MONK
CELL#: 602-326-9282

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NOTICE: This is not a rental agreement. The rental of equipment and any items listed above is subject to availability and subject to the terms and conditions of the Rental and Service Agreement, which are available at <https://www.unitedrentals.com/legal/rental-service-terms-US> and which are incorporated herein by reference. A COPY OF THE RENTAL AND SERVICE AGREEMENT TERMS ARE AVAILABLE IN PAPER FORM UPON REQUEST.



TRENCH SAFETY RENTALS

320 S. 19th Avenue, Phoenix AZ 85009

Phone: (602) 253-3311

Fax: (602) 253-3320

RENTAL QUOTE

Sales Rep: Kevin Koehler

Phone: (602) 509-2231

Email: kevin.koehler@sunstateequip.com

Date: 7/16/2025

Customer: B&F
Contact: Travis Monk
Phone: 602-326-9282
Email: tmonk@bfcontracting.com
Job Address: SR 101 & SR 202
Job/PO #: Chandler Redundant Sewer BORE PIT

	CATEGORY	DESCRIPTION	EACH DAILY	EACH WEEKLY	EACH MONTHLY
3	Steel Crossing Plates	8 X 20 END PROTECTION	\$ 11.93	\$ 83.51	\$ 334.04
1	Trench Box - Steel	6' x 16' x 4"	\$ 38.30	\$ 153.20	\$ 459.60
2	Trench Box - Steel	10' x 16' x 4"	\$ 61.42	\$ 245.68	\$ 737.04
1	Trench Box - Steel	6' x 24' x 6"	\$ 51.31	\$ 205.24	\$ 615.72
2	Trench Box - Steel	10' x 24' x 6"	\$ 82.37	\$ 329.48	\$ 988.44
18	Trench Box - Accessories	Corrugated Hanger Plates 10-6', 8-20"	\$ 6.95	\$ 27.80	\$ 83.40
12	Trench Box - Accessories	20' I-Beam Spreader	\$ 20.00	\$ 80.00	\$ 240.00
12	Trench Box - Accessories	20' Pipe Spreader	\$ 5.00	\$ 20.00	\$ 60.00
1	Trench Box - Accessories	Ladder Hanger	\$ 2.49	\$ 9.96	\$ 29.88
4	Trench Box - Accessories	Stacking Pin (Set of 4)	\$ 2.01	\$ 8.04	\$ 24.12
12	Trench Box - Accessories	Handrail	\$ 7.00	\$ 28.00	\$ 84.00
2	Slings - Chain	4 Way Bridle 20' Box	\$ 17.38	\$ 69.52	\$ 208.56
1	Trench Bridges	8'	\$ 15.18	\$ 60.72	\$ 182.16
			DAILY	WEEKLY	MONTHLY
SUBTOTAL - ESTIMATED RENTAL			\$ 742.55	\$ 3,077.57	\$ 9,483.24
TAX - 8.6%			\$ 63.86	\$ 264.67	\$ 815.56
TOTAL - ESTIMATED RENTAL			\$ 806.41	\$ 3,342.24	\$ 10,298.80

QTY	SERVICE ITEMS	COST	UNIT	TOTAL COST
5	Delivery - Phoenix Metropolitan Area	\$225.00	Each	\$ 1,125.00
5	Pickup - Phoenix Metropolitan Area	\$225.00	Each	\$ 1,125.00
4	Wide Load Charge Delivery & Pick up	\$50.00	Each	\$ 200.00
1	Site Specific Engineering (For excavations over 20ft depth if required)	\$1,500.00	Each	\$ 1,500.00
SUBTOTAL - ESTIMATED TRANSPORTATION/SERVICE ITEMS				\$ 3,950.00
TAX 8.6%				\$ 210.70
TOTAL - ESTIMATED TRANSPORTATION/SERVICE ITEMS				\$ 4,160.70

After Hours/Wide Load charge of \$150.00/per load if applicable/Freight is for Metro Phoenix unless noted.

Customers will be responsible for equipment assembly/disassembly.

Quoted By: Kevin Koehler

Customer Signature

QUOTE IS VALID FOR 30 DAYS, WE RESERVE THE RIGHT TO CHANGE/CANCEL TERMS WITHOUT WRITTEN NOTICE

THANK YOUR FOR THE OPPORTUNITY TO SERVE YOU!



TRENCH SAFETY RENTALS

320 S. 19th Avenue, Phoenix AZ 85009

Phone: (602) 253-3311

Fax: (602) 253-3320

RENTAL QUOTE

Sales Rep: Kevin Koehler

Phone: (602) 509-2231

Email: kevin.koehler@sunstateequip.com

Date: 7/16/2025

Customer: B&F

Contact: Travis Monk

Phone: 602-326-9282

Email: tmonk@bfcontracting.com

Job Address: SR 101 & SR 202

Job/PO #: Chandler Redundant Sewer Junction Structure B

	CATEGORY	DESCRIPTION	EACH DAILY	EACH WEEKLY	EACH MONTHLY
28	Trench Box - Accessories	Handrail	\$ 7.00	\$ 28.00	\$ 84.00
1	Trench Bridges	8'	\$ 15.18	\$ 60.72	\$ 182.16
			DAILY	WEEKLY	MONTHLY
SUBTOTAL - ESTIMATED RENTAL			\$ 211.18	\$ 844.72	\$ 2,534.16
TAX - 8.6%			\$ 18.16	\$ 72.65	\$ 217.94
TOTAL - ESTIMATED RENTAL			\$ 229.34	\$ 917.37	\$ 2,752.10

After Hours/Wide Load charge of \$150.00/per load if applicable/Freight is for Metro Phoenix unless noted.

Customers will be responsible for equipment assembly/disassembly.

Quoted By: Kevin Koehler

Customer Signature

Date: 7/16/2025

Title

QUOTE IS VALID FOR 30 DAYS, WE RESERVE THE RIGHT TO CHANGE/CANCEL TERMS WITHOUT WRITTEN NOTICE

THANK YOUR FOR THE OPPORTUNITY TO SERVE YOU!



Slide Rail Rental Quote

Job Name: Chandler Redundant Sewer Junction Structure C
Job Location: SR 101 & SR 202

Quantity	Service/Item Description	Unit Price	Unit	Total
5	Delivery	\$ 800.00	EACH	\$4,000.00
5	Pickup	\$ 800.00	EACH	\$4,000.00
SERVICE/ITEM TOTAL			\$8,000.00	

Customer Signature _____ Date _____



TRENCH SAFETY RENTALS

320 S. 19th Avenue, Phoenix AZ 85009

Phone: (602) 253-3311

Fax: (602) 253-3320

RENTAL QUOTE

Sales Rep: Kevin Koehler

Phone: (602) 509-2231

Email: kevin.koehler@sunstateequip.com

Date: 7/16/2025

Customer: B&F

Contact: Travis Monk

Phone: 602-326-9282

Email: tmonk@bfcontracting.com

Job Address: SR 101 & SR 202

Job/PO #: Chandler Redundant Sewer Junction Structure C

	CATEGORY	DESCRIPTION	EACH DAILY	EACH WEEKLY	EACH MONTHLY
20	Trench Box - Accessories	Handrail	\$ 7.00	\$ 28.00	\$ 84.00
1	Trench Bridges	8'	\$ 15.18	\$ 60.72	\$ 182.16
			DAILY	WEEKLY	MONTHLY
SUBTOTAL - ESTIMATED RENTAL			\$ 155.18	\$ 620.72	\$ 1,862.16
TAX - 8.6%			\$ 13.35	\$ 53.38	\$ 160.15
TOTAL - ESTIMATED RENTAL			\$ 168.53	\$ 674.10	\$ 2,022.31

After Hours/Wide Load charge of \$150.00/per load if applicable/Freight is for Metro Phoenix unless noted.

Customers will be responsible for equipment assembly/disassembly.

Quoted By: Kevin Koehler

Date: 7/16/2025

Customer Signature

Title



Slide Rail Rental Quote

Job Name: Chandler Redundant Sewer Junction Structure E
Job Location: SR 101 & SR 202

Quantity	Service/Item Description	Unit Price	Unit	Total
5	Delivery	\$ 800.00	EACH	\$4,000.00
5	Pickup	\$ 800.00	EACH	\$4,000.00
SERVICE/ITEM TOTAL			\$8,000.00	

Customer Signature _____ Date _____



TRENCH SAFETY RENTALS

320 S. 19th Avenue, Phoenix AZ 85009

Phone: (602) 253-3311

Fax: (602) 253-3320

RENTAL QUOTE

Sales Rep: Kevin Koehler

Phone: (602) 509-2231

Email: kevin.koehler@sunstateequip.com

Date: 7/16/2025

Customer: B&F

Contact: Travis Monk

Phone: 602-326-9282

Email: tmonk@bfcontracting.com

Job Address: SR 101 & SR 202

Job/PO #: Chandler Redundant Sewer Junction Structure E

	CATEGORY	DESCRIPTION	EACH DAILY	EACH WEEKLY	EACH MONTHLY
20	Trench Box - Accessories	Handrail	\$ 7.00	\$ 28.00	\$ 84.00
1	Trench Bridges	8'	\$ 15.18	\$ 60.72	\$ 182.16
			DAILY	WEEKLY	MONTHLY
SUBTOTAL - ESTIMATED RENTAL			\$ 155.18	\$ 620.72	\$ 1,862.16
TAX - 8.6%			\$ 13.35	\$ 53.38	\$ 160.15
TOTAL - ESTIMATED RENTAL			\$ 168.53	\$ 674.10	\$ 2,022.31

After Hours/Wide Load charge of \$150.00/per load if applicable/Freight is for Metro Phoenix unless noted.

Customers will be responsible for equipment assembly/disassembly.

Quoted By: Kevin Koehler

Customer Signature

Date: 7/16/2025

Title

QUOTE IS VALID FOR 30 DAYS, WE RESERVE THE RIGHT TO CHANGE/CANCEL TERMS WITHOUT WRITTEN NOTICE

THANK YOUR FOR THE OPPORTUNITY TO SERVE YOU!



TRENCH SAFETY RENTALS

320 S. 19th Avenue, Phoenix AZ 85009

Phone: (602) 253-3311

Fax: (602) 253-3320

RENTAL QUOTE

Sales Rep: Kevin Koehler

Phone: (602) 509-2231

Email: kevin.koehler@sunstateequip.com

Date: 7/16/2025

Customer: B&F

Contact: Travis Monk

Phone: 602-326-9282

Email: tmonk@bfcontracting.com

Job Address: SR 101 & SR 202

Job/PO #: Chandler Redundant Sewer RECEIVER PIT

	CATEGORY	DESCRIPTION	EACH DAILY	EACH WEEKLY	EACH MONTHLY
2	Steel Crossing Plates	8 X 20 END PROTECTION	\$ 11.93	\$ 83.51	\$ 334.04
1	Trench Box - Steel	6' x 16' x 4"	\$ 38.30	\$ 153.20	\$ 459.60
2	Trench Box - Steel	10' x 16' x 4"	\$ 61.42	\$ 245.68	\$ 737.04
1	Trench Box - Steel	6' x 24' x 6"	\$ 51.31	\$ 205.24	\$ 615.72
2	Trench Box - Steel	10' x 24' x 6"	\$ 82.37	\$ 329.48	\$ 988.44
16	Trench Box - Accessories	Corrugated Hanger Plates 9-6', 8-20"	\$ 6.95	\$ 27.80	\$ 83.40
12	Trench Box - Accessories	16' I-Beam Spreader	\$ 18.00	\$ 72.00	\$ 216.00
12	Trench Box - Accessories	20' Pipe Spreader	\$ 5.00	\$ 20.00	\$ 60.00
1	Trench Box - Accessories	Ladder Hanger	\$ 2.49	\$ 9.96	\$ 29.88
4	Trench Box - Accessories	Stacking Pin (Set of 4)	\$ 2.01	\$ 8.04	\$ 24.12
12	Trench Box - Accessories	Handrail	\$ 7.00	\$ 28.00	\$ 84.00
2	Slings - Chain	4 Way Bridle 20' Box	\$ 17.38	\$ 69.52	\$ 208.56
1	Trench Bridges	8'	\$ 15.18	\$ 60.72	\$ 182.16
			DAILY	WEEKLY	MONTHLY
SUBTOTAL - ESTIMATED RENTAL			\$ 716.72	\$ 2,938.46	\$ 8,982.40
TAX - 8.6%			\$ 61.64	\$ 252.71	\$ 772.49
TOTAL - ESTIMATED RENTAL			\$ 778.36	\$ 3,191.17	\$ 9,754.89

QTY	SERVICE ITEMS	COST	UNIT	TOTAL COST
5	Delivery - Phoenix Metropolitan Area	\$225.00	Each	\$ 1,125.00
5	Pickup - Phoenix Metropolitan Area	\$225.00	Each	\$ 1,125.00
4	Wide Load Charge Delivery & Pick up	\$50.00	Each	\$ 200.00
1	Site Specific Engineering (For excavations over 20ft depth if required)	\$1,500.00	Each	\$ 1,500.00
SUBTOTAL - ESTIMATED TRANSPORTATION/SERVICE ITEMS				\$ 3,950.00
TAX 8.6%				\$ 210.70
TOTAL - ESTIMATED TRANSPORTATION/SERVICE ITEMS				\$ 4,160.70

After Hours/Wide Load charge of \$150.00/per load if applicable/Freight is for Metro Phoenix unless noted.

Customers will be responsible for equipment assembly/disassembly.

Quoted By: Kevin Koehler

Customer Signature

QUOTE IS VALID FOR 30 DAYS, WE RESERVE THE RIGHT TO CHANGE/CANCEL TERMS WITHOUT WRITTEN NOTICE

THANK YOUR FOR THE OPPORTUNITY TO SERVE YOU!



Slide Rail Rental Quote

Job Name: Chandler Redundent Sewer Junction Structure B
Job Location: SR 101 & SR 202 San Tan

Quantity	Service/Item Description	Unit Price	Unit	Total
10	Delivery	\$ 800.00	EACH	\$8,000.00
10	Pickup	\$ 800.00	EACH	\$8,000.00
2	47'6" Waster Beam	\$ 5,700.00	EACH	\$11,400.00
SERVICE/ITEM TOTAL			\$27,400.00	

Customer Signature _____ Date _____



Quote # 2502R

Date: 7/18/2025
Quoted By: Timothy Plane
Phone: 602-370-7532

Customer: B&F Contracting Contact: Travis Monk
Phone: 602-326-9282
Email: tmonk@bfcontracting.com

Project Name: COC 66" Redundant CMAR - Junction Structure B

Location: NEC of San Tan 202 & Price Rd, Chandler

Approx Duration: 7 Months 901D

Rental Items Description	Qty.	Per Unit Cost			
			Daily	Weekly	Monthly
Slide Rail System 33'D x 47'L x 47' W	7				55,814.00
Clearspan					
Note: Contractor to supply 2"x4"					
material to complete guard rail fencing					
10' Ped Bridge	7				479.91
Ladder Hanger	7				47.99
Note: Contractor to supply ladder					
EXTENDED TOTALS			0.00	0.00	394,393.30

Delivery/ Pickup	Qty.	Cost	Ext.
Round Trip Freight (estimated at 7 truck loads each way)	14	2,500.00	35,000.00
Deadman	2	3,600.00	7,200.00
Site Specific Engineering	1	6,300.00	6,300.00
Site Consultation (est. at 4 days install included rigging kit)	1	4,500.00	4,500.00
EXTENDED TOTALS		16,900.00	48,000.00

TOTALS			442,393.30
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Fuel surcharge is not included and will be added to all items

Sales tax is not included and will be added to all items

Phoenix (602)605-4150 | Southern Arizona (833)384-1176 | Northern Arizona (833)384-1176



Quote # 2503R

Date: 7/18/2025
Quoted By: Timothy Plane
Phone: 602-370-7532

Customer: B&F Contracting Contact: Travis Monk
Phone: 602-326-9282
Email: tmonk@bfcontracting.com
Project Name: COC 66" Redundant CMAR - Junction Structure C
Location: NEC of San Tan 202 & Price Rd, Chandler
Approx Duration: 3 Months 901D

Rental Items Description	Qty.	Per Unit Cost			
			Daily	Weekly	Monthly
Slide Rail System 28'D x 20'L x 20'W	3				17,020.00
Clearspan					
10' Ped Bridge	3				479.91
Lagger Hanger	3				47.99
Note: Contractor to supply ladder					
Note: Contractor to supply 2"x4"					
material to complete guard rail fencing					
EXTENDED TOTALS			0.00	0.00	52,643.70

Delivery/ Pickup	Qty.	Cost	Ext.
Round Trip Freight (estimated at 4 truck loads)	8	2,500.00	20,000.00
Deadman	0	3,600.00	0.00
Site Specific Engineering	1	3,500.00	3,500.00
Site Consultation (est. at 3 days install includes rigging kit)	1	3,375.00	3,375.00
EXTENDED TOTALS		12,975.00	26,875.00

TOTALS			79,518.70
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Fuel surcharge is not included and will be added to all items

Sales tax is not included and will be added to all items

Phoenix (602)605-4150 | Southern Arizona (833)384-1176 | Northern Arizona (833)384-1176



Quote # 2504R

Date: 7/18/2025
Quoted By: Timothy Plane
Phone: 602-370-7532

Customer: B&F Contracting Contact: Travis Monk
Phone: 602-326-9282
Email: tmonk@bfcontracting.com

Project Name: COC 66" Redundant CMAR - Bore Pit

Location: NEC of San Tan 202 & Price Rd, Chandler

Approx Duration: 6 Months 901D

Rental Items Description	Qty.	Per Unit Cost			
			Daily	Weekly	Monthly
Slide Rail System 26'D x 40'L x 20' W	6				30,240.00
Clearspan					
Note: Contractor to supply 2"x4"					
material to complete guard rail fencing					
8x20 Steel Plate	36				873.17
Hanging End Panels (26')	60				221.87
10' Ped Bridge	6				479.91
Ladder Hanger	6				47.99
Note: Contractor to supply ladder					
EXTENDED TOTALS			0.00	0.00	229,353.72

Delivery/ Pickup	Qty.	Cost	Ext.
Round Trip Freight (estimated at 5 truck loads each way)	10	2,500.00	25,000.00
Deadman	1	2,100.00	2,100.00
Site Specific Engineering	1	4,250.00	4,250.00
Site Consultation (estimated at 4 days install)	1	4,500.00	4,500.00
EXTENDED TOTALS		13,350.00	35,850.00

TOTALS			265,203.72
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Fuel surcharge is not included and will be added to all items

Sales tax is not included and will be added to all items

Phoenix (602)605-4150 | Southern Arizona (833)384-1176 | Northern Arizona (833)384-1176



Quote # 2505R

Date: 7/18/2025
Quoted By: Timothy Plane
Phone: 602-370-7532

Customer: B&F Contracting Contact: Travis Monk
Phone: 602-326-9282
Email: tmonk@bfcontracting.com

Project Name: COC 66" Redundant CMAR - Receiving Pit

Location: NEC of San Tan 202 & Price Rd, Chandler

Approx Duration: 6 Months 901D

Rental Items Description	Qty.	Per Unit Cost			
			Daily	Weekly	Monthly
Slide Rail System 25'D x 40'L x 16' W	6				25,270.00
Clearspan					
Note: Contractor to supply 2"x4"					
material to complete guard rail fencing					
8x20 Steel Plate	36				873.17
Hanging End Panels (26')	60				221.87
10' Ped Bridge	6				479.91
Ladder Hanger	6				47.99
Note: Contractor to supply ladder					
EXTENDED TOTALS			0.00	0.00	199,533.72

Delivery/ Pickup	Qty.	Cost	Ext.
Round Trip Freight (estimated at 4 truck loads each way)	8	2,500.00	20,000.00
Deadman	1	2,100.00	2,100.00
Site Specific Engineering	1	4,250.00	4,250.00
Site Consultation (estimated at 4 days install)	1	4,500.00	4,500.00
EXTENDED TOTALS		13,350.00	30,850.00

TOTALS			230,383.72
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Fuel surcharge is not included and will be added to all items

Sales tax is not included and will be added to all items

Phoenix (602)605-4150 | Southern Arizona (833)384-1176 | Northern Arizona (833)384-1176



Quote # 2512R

Date: 7/18/2025
 Quoted By: Timothy Plane
 Phone: 602-370-7532

Customer: B&F Contracting Contact: Travis Monk

Phone: 602-326-9282

Email: tmonk@bfcontracting.com

Project Name: COC 66" Redundant CMAR - Junction Structure E

Location: NEC of San Tan 202 & Price Rd, Chandler

Approx Duration: 3 Months 901D

Rental Items Description	Qty.	Per Unit Cost			
			Daily	Weekly	Monthly
Slide Rail System 27'D x 20'L x 20'W	3				17,020.00
Clearspan					
Note: Contractor to supply 2"x4"					
material to complete guard rail fencing					
10' Ped Bridge	3				479.91
Ladder Hanger	3				47.99
Note: Contractor to supply ladder					
EXTENDED TOTALS			0.00	0.00	52,643.70

Delivery/ Pickup	Qty.	Cost	Ext.
Round Trip Freight (estimated at 4 truck loads each way)	8	2,500.00	20,000.00
Deadman	0	3,600.00	0.00
Site Specific Engineering	1	3,500.00	3,500.00
Site Consultation (est. at 4 days install including rigging kit)	1	3,375.00	3,375.00
EXTENDED TOTALS		12,975.00	26,875.00

TOTALS			79,518.70
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Fuel surcharge is not included and will be added to all items

Sales tax is not included and will be added to all items

Phoenix (602)605-4150 | Southern Arizona (833)384-1176 | Northern Arizona (833)384-1176

Fiber Glass Systems | NOV

Fiber Glass Systems, L.P.
 5985 South 6th Way
 Ridgefield, WA 98642
 Phone: (360) 887-3580
 Fax: (360) 887-3577
 Web: www.Plasti-Fab.com

Quote

Account Manager:
 Mike A Garcia
 Regional Sales Manager - West
 564-237-0342
 Mike.Garcia@nov.com

Prepared by:
 Damon Quint

B&F Contracting

Date: 07/17/2025

Attn: Travis Monk

Project: City of Chandler Junction Structures B & C – AZ

Quotation Number: 250123101 Rev2

Qty	Description	Price	Total
	<u>Junction Structure B</u> Pipe Size: 66" ID Structural Opening: 74" ID Plasti-Fab Stop Log / Filler Log Assembly 66" x 84" (3) Guide Frames (3) Sets of Logs Includes:		\$270,034.00
3	T-316 stainless steel surface mount guide frame 5'-6" wide x 23'-0" high w/ embedded bottom channel and T-316 anchor bolts.		
9	Composite FRP infusion molded <u>stop log</u> 5'-6" wide x 2'-0" high x 3.5" thick with T-316 lifting pins, EPDM base seals and vertical seals mounted on the face. (Approximately 301 lbs per log)		
3	Composite FRP infusion molded <u>stop log</u> 5'-6" wide x 1'-0" high x 3.5" thick with T-316 lifting pins, EPDM base seals and vertical seals mounted on the face. (Approximately 161 lbs per log)		
3	Composite FRP infusion molded <u>filler log</u> 5'-6" wide x 6'-0" high x 3.5" thick with flow through opening and T-316 lifting pins. (Approximately 350 lbs per log)		
1	T-316 stainless steel lifting beam for above logs		
3	T-316 stainless steel storage rack wall mounted 66" x 84"		

	<u>Junction Structure C</u> Pipe Size: 66" ID Structural Opening: 74" ID Plasti-Fab Stop Log / Filler Log Assembly 66" x 84" (2) Guide Frames (2) Sets of Logs Includes:		\$186,689.00
2	T-316 stainless steel surface mount guide frame 5'-6" wide x 19'-0" high w/ embedded bottom channel and T-316 anchor bolts.		
6	Composite FRP infusion molded <u>stop log</u> 5'-6" wide x 2'-0" high x 3.5" thick with T-316 lifting pins, EPDM base seals and vertical seals mounted on the face. (Approximately 301 lbs per log)		
2	Composite FRP infusion molded <u>stop log</u> 5'-6" wide x 1'-0" high x 3.5" thick with T-316 lifting pins, EPDM base seals and vertical seals mounted on the face. (Approximately 161 lbs per log)		
2	Composite FRP infusion molded <u>filler log</u> 5'-6" wide x 6'-0" high x 3.5" thick with flow through opening and T-316 lifting pins. (Approximately 350 lbs per log)		
1	T-316 stainless steel lifting beam for above logs		
2	T-316 stainless steel storage rack wall mounted 66" x 84"		
	<u>Clarifications:</u> 1) Davit crane, hoist cable/rope, lifting bridle, eye hook and access hatch by others. 2) We are quoting guide frame and storage rack material as T-316 stainless steel in lieu of specified FRP. 3) Fiber Glass Systems provides a 1-year warranty against failure due to any defects in material and workmanship. 4) All testing and field start-up by others.		
Subtotal			\$456,723.00
Estimated Freight + 15% - EXW Seller's Premises Incoterms 2020			\$15,525.00

Terms: Unless otherwise specified, Fiber Glass Systems standard terms are NET 30, contingent upon credit approval.

Exclusions:

- Installation, grouting and concrete.
- Fasteners, anchor hardware and adhesive not expressly stated above.
- Appurtenances, fittings, supports, and materials not expressly stated above.
- Controls and operators not expressly stated above.
- Spare parts not expressly stated above.
- NSF-61 compliant components not expressly stated above.
- Site visitation for any reason, including start-up and validation.
- Shop testing and field testing.

O&M Manuals: When requested, 1 electronic copy only will be provided. Hard copies not included.